

CHANGE ORDERS

the execution of such Plans.

2.1 All changes and deviations in the Plans, including cost, credit or debt, must be set forth in a Change Order agreed upon and signed by the OWNERS and Designs by SPB (hereinafter called "Change Order"). A Change Order concerning any portion of the Plan must be in advance of the performance of that specific portion of the work and at the OWNERS expense, if any, shall be paid at the time the Change Order is signed by all parties.

professionals, oWNER undrstands and assumes all risk regarding

2.2 OWNERS understand that additional expenses may be incurred in excess of the amount of the estimated original cost due to hidden or unknown contingencies, changes, permits, or the like that may occurduring the process, preparation and/or performance of construction. In the event that such hidden, unknown contingencies or changes shall arise reqiuring revised Plans or design changes, Designs by SPB and OWNERS shall execute a Change Order with respect to the same in advance of the performance of work by Designs by SPB.

REFER TO 2015 IRC & 9TH EDITION MASSACHUSETTS CODE

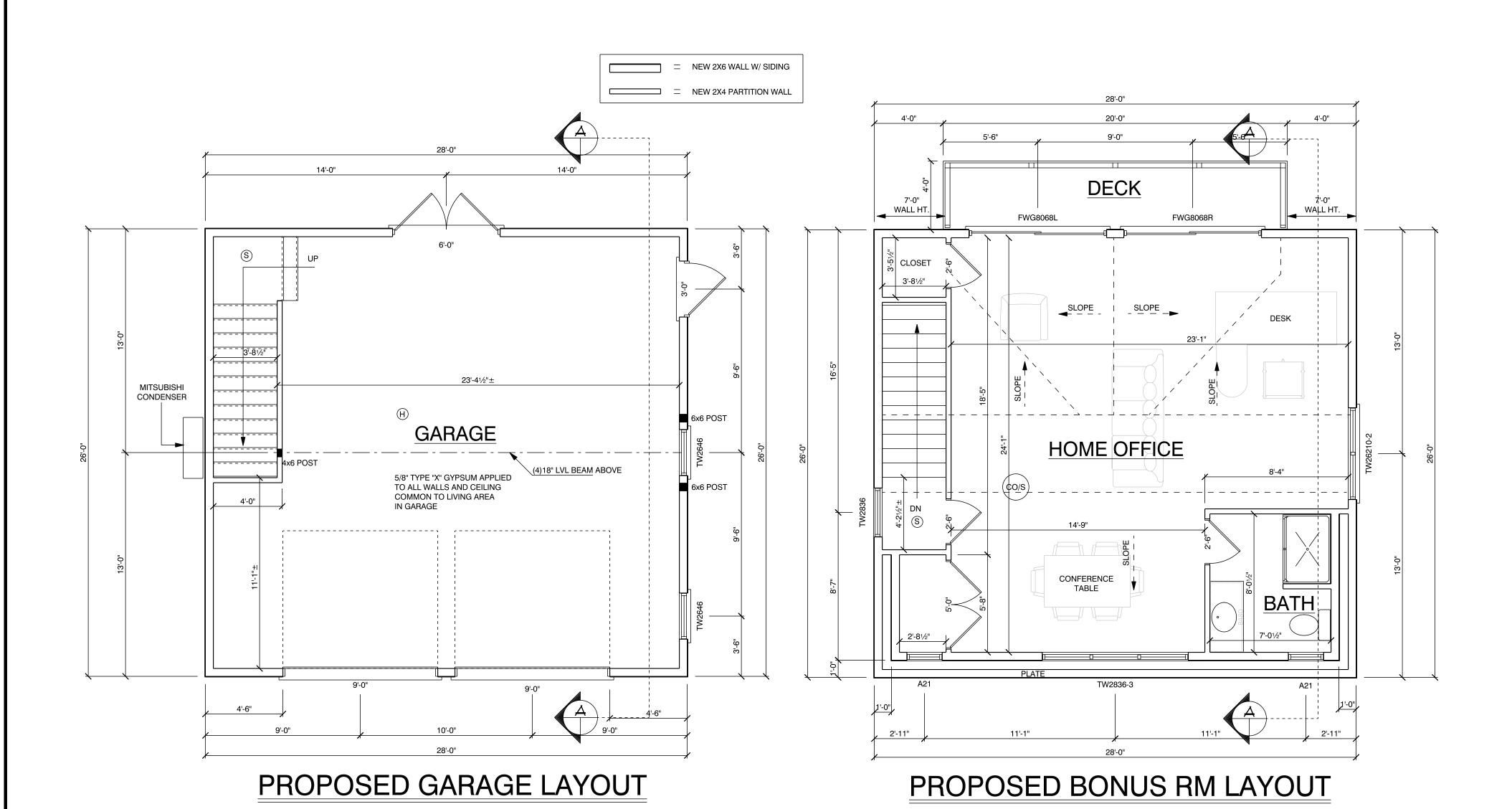
LEFT ELEVATION

NOTE: ALL EXISTING AND PROPOSED DIMENSIONS TO BE FIELD VERIFIED.

RIGHT ELEVATION

FRONT ELEVATION

NOTE: ALL TYPE/ SIZE OF SIDING,TRIM,& EXTERIOR DETAILS TO BE VERIFIED BY HOMEOWNER/CONTRACTOR





REAR ELEVATION

CURREN RESIDENCE

RESIDENTIAL/COMMERCIAL

GARAGE DESIGN

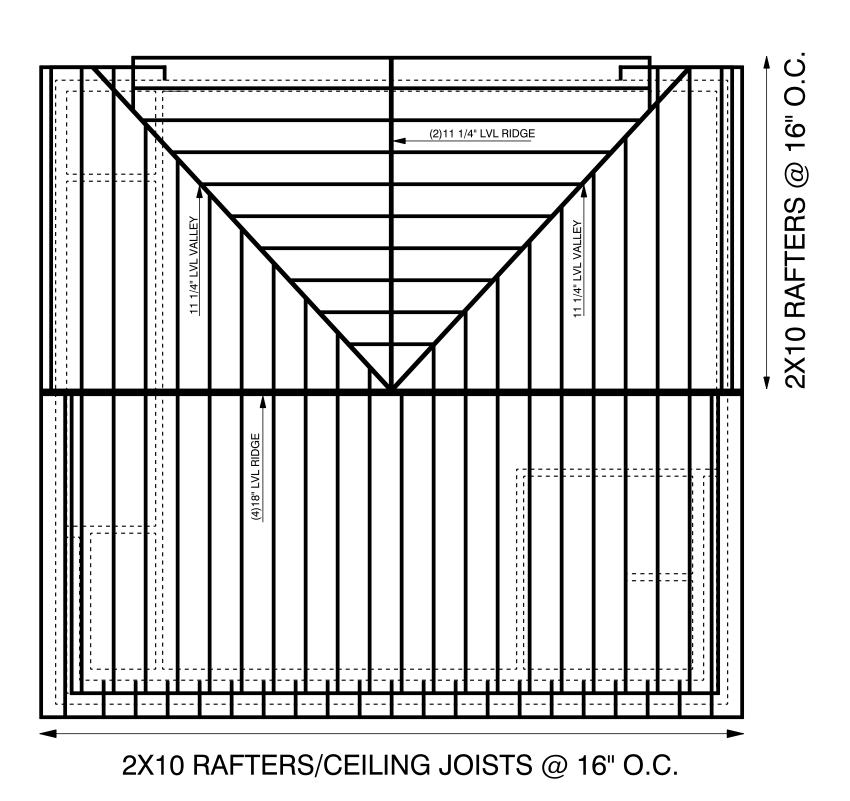
104 ROCKY POINT

BOURNE, MA. (508)495-2881

REVISIONS:

SCALE: 1/4"=1'-0" UNLESS NOTED

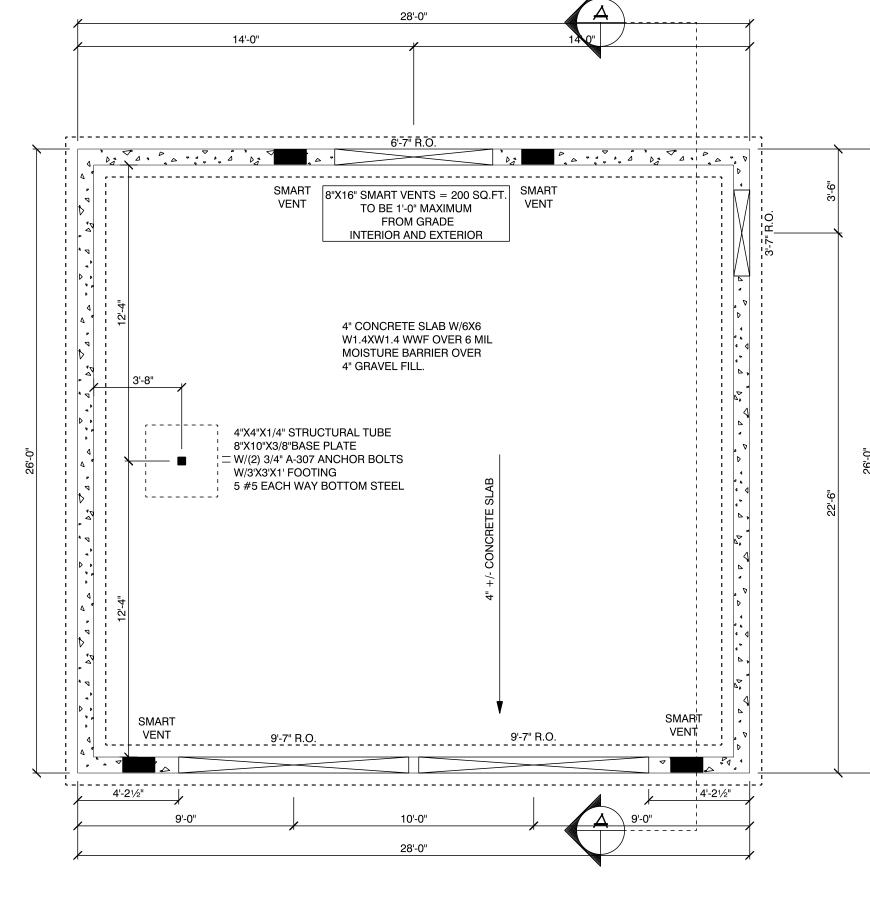
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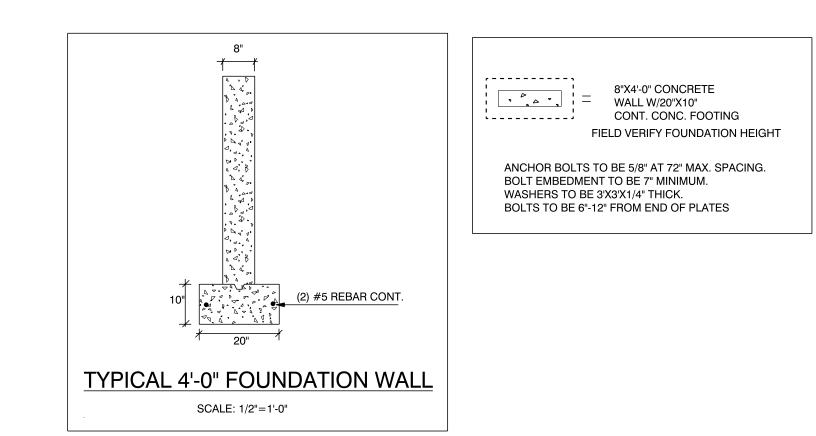
ROOF FRAMING PLAN

2X8 P.T. DECK JOISTS @ 16" O.C. 2X10 FLOOR JOISTS @ 16" O.C.

DOUBLE JOISTS @ FRONT SHED DORMER SECOND FLOOR FRAMING PLAN



PROPOSED FOUNDATION PLAN



DESIGN LOADS

- 1. The various portions of the structure are designed to carry the following live loads in addition to specific machinery and equipment loads: Roof 21 psf based on a ground snow of 30 psf
- Corridors
- 2. Lateral loads: Wind Load: 15 psf based on wind velocity of 120 mph. Seismic Design Load: $S\sim sMS\sim S=0.34$, $S\sim sDS\sim S=0.08$, Site Class C, $C\sim ss\sim S=0.04$
- EXCAVATIONS, FOUNDATIONS AND BACKFILL
- 1. All footings shall be carried down to 4" into the undisturbed layer having a minimum bearing capacity of 2 ton per square foot, or shall bear on compacted fill as described in note 10 below.
- 2. Typical footing excavations will be inspected by the Architect before the footings are cast in order to confirm that the foundation material is adequate to sustain the design bearing pressure.
- 3. All exterior construction shall be carried down a minimum of 4 feet below finished exterior grade, or bear on ledge, unless otherwise shown on

CONCRETE

- 1. All concrete shall conform to the Building Code Requirements for Reinforced Concrete (ACI 318), the Standard Specification for Structural Concrete in Buildings (ACI 301) and local building codes. All concrete work shall be as specified and recommended by ACI field reference manual SP-
- 2. All concrete shall have a minimum compressive strength of 3000 psi at
- 3. Provide 6% air-entrainment for all concrete. (Optional addition) exposed to earth or weather.
- 4. All reinforcing bars shall be deformed bars conforming to requirements of ASTM Specification A615, Grade 60.

STRUCTURAL STEEL

1. Steel plates and angles shall conform to ASTM A36. Steel wide flange beams (and all other shapes not specifically addressed) shall conform to ASTM A992 (Fy= 50 ksi). Structural tubes shall conform to ASTM A500, Grade B (Fy= 46 ksi). Pipe shall conform to ASTM A53, Grade B or ASTM

WOOD FRAMING

- 1. All wood construction shall conform to the requirements of the National Design Specification for Wood Construction by the National Forest Products Association and to local building codes.
- 2. All wood for framing shall be kiln-dried with a maximum moisture content of 19% at the time of dressing.
- 3. All wood members 2x4 and larger shall have the following minimum structural properties:
- Fb =875 psi Fv = 75 psi
- Fc = 875 psi
- $E = 1.4x10^6 psi$
- 4. All wood stud bearing walls, exterior walls, and walls greater than 10 feet in height shall be 2x6 at 16" o/c. unless otherwise indicated. In all bearing walls provide a single bottom shoe, double top plate and solid wall bridging at a maximum vertical spacing of 48 inches.
- 5. All wall studs shall be located directly under floor joists. Provide double studs on each side of all openings, and additional jack studs to support header beams. Form corners with a minimum of three studs spiked
- 6. Continuity in framing shall be provided at all bearing walls in order to transfer the loads to the foundation or other framing. Full depth blocking shall be used in the floor framing under wood posts, bearing walls, and built-up studs to provide full bearing through framing. Double joists shall be used under all partitions running parallel to joist span.
- 7. All wood headers at bearing walls shall be a minimum of 3-2x8 unless otherwise shown on plans. Wood members used for headers or built-up beams shall not have checks or splits longer than the wide face width.

- 8. All member to member connections shall be made with joist or beam hangers, and sheet metal post bases and caps as appropriate. Joist hangers, framing clips and other hardware shall be manufactured by Simpson company or approved equal.
- 9. Refer to the Massachusetts State Building Code Nailing Schedule for nailing and bolting not otherwise specified on the drawings.
- 10. Do not notch the tops or bottoms of joists in the middle third of the span. Notches in the end thirds shall not exceed 1/6 of the joist depth. Holes drilled in joists shall not exceed 1/3 of the joist depth and shall not be located within 2" of the top or bottom of the joist.
- 11. Preservative pressure treat wood with Chromated copper arsenite, to a retention of 0.75 lb/cu.ft. for wood exposed to earth or weather, and 0.35 lb/cu.ft. for wood in contact with concrete, masonry or roofing.

FRAMING NOTES

BLOCKING & CONNECTIONS SHALL BE PROVIDED AT PANEL EDGES PERPENDICULAR TO FLOOR FRAMING MEMBERS IN THE FIRST TWO TRUSS OR JOIST SPACES AND SHALL BE SPACED AT A MAXIMUM 4 FEETON CENTER. NAILING REQUIREMENTS ARE: BLOCKING TO JOIST--2-8d FOR COMMON NAILS & AT EACH END.

FLOOR SHEATHING FASTENING NAILING REQUIREMENTS ARE: 3/4" T&G CDX PLYWOOD OR EQUAL

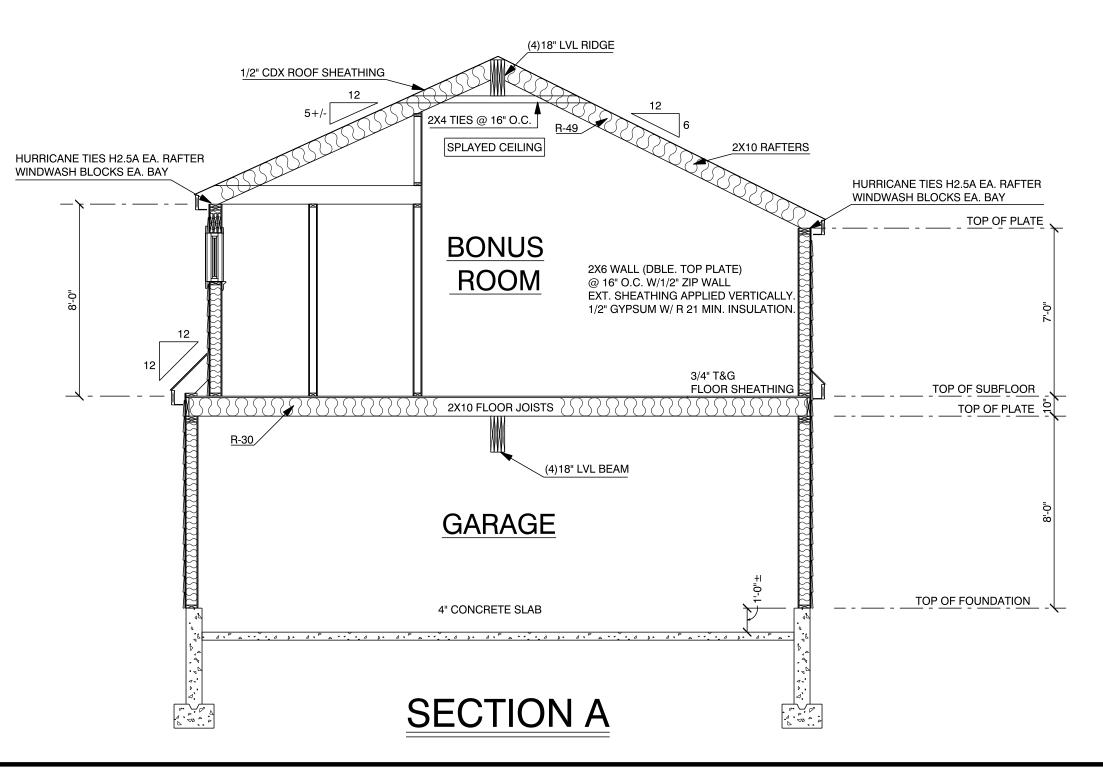
NAILING TO BE 8d FOR COMMON NAILS WITH SPACING AT 6" EDGE/12" FIELD. LOAD BEARING WALLS TO HAVE A MAXIMUM HEIGHT OF 10'-0" NON-LOAD BEARING WALLS TO HAVE A MAXIMUM HEIGHT OF 20'-0"

EXTERIOR WALLS

WALL SPACING TO BE 2X4 @ 16" O.C. WALL AT GARAGE DOORS TO 2X6 @ 16" O.C.

WOOD STUDS: LOAD BEARING WALLS TO HAVE A MAXIMUM HEIGHT OF 9'-9" NON-LOAD BEARING WALLS TO HAVE A MAXIMUM HEIGHT OF 9'-9" WALL SPACING TO BE 2X6 @ 16" O.C. WALL AT GARAGE DOORS TO 2X6 @ 16" O.C. STUDS IN GABLE END WALLS: ADJACENT TO CATHEDRAL CEILINGS SHALL BE CONTINUOUS FROM THE CEILING DIAPHRAM OR TO THE ROOF DIAPHRAM. DOUBLE TOP PLATE: SPLICE LENGTH = 4FT. MINIMUM WITH 14- 16d COMMON NAILS EACH SIDE OF SPLICE. WALL OPENINGS: HEADERS TO BE 2X10 WITH 3- FULL HEIGHT STUDS (UNLESS NOTED). EXTERIOR WALL SHEATHING: SHEATHING TYPE TO BE 1/2" NAILED 4" O.C. EDGES/12" O.C. IN FIELD. SHEATHING (FULL SHEETS)TO SPAN FROM RIM JOISTS/BOTTOM PLATE TO TOP PLATE.

ROOF OVERHANGS TO BE 1'-0" OR LESS. HURRICANE TIES TO BE SIMPSON H2.5A. RIDGE STRAP CONNECTION TO BE SIMPSON LSTA15 1/2" CDX PLYWOOD FASTENED WITH 8d COMMON NAILS @ 6" EDGE-6" FIELD. GABLE END WALL RAKE W/LOOKOUT BLOCKS TO BE 8d COMMOM NAILS BLOCKING TO BE PROVIDED IN FIRST TO RAFTERS/ ROOF TRUSSES @ 4'-0" O.C..



DESIGN

PLAN DATE: 7-20-2022 DRAWN BY: SPB

REVISIONS:

SCALE: 1/4"=1'-0" **UNLESS NOTED**