STORMWATER ANALYSIS AND DRAINAGE REPORT

CAPE VIEW WAY BOURNE, MASSACHUSETTS

Prepared for:

PRESERVATION OF AFFORDABLE HOUSING

2 OLIVER STREET, SUITE 500

BOSTON, MA 02109

AND

HOUSING ASSISTANCE CORPORATION

460 WEST MAIN STREET

HYANNIS, MA 02601

Prepared by: Horsley Witten Group, Inc.

March 5, 2021

Revised: December 2021

Horsley Witten Group Sustainable Environmental Solutions 90 Route 6A • Unit 1 • Sandwich, MA 02563 508-833-6600 • horsleywitten.com



TABLE OF CONTENTS

Page

STORMWATER CHECKLIST

1.0	STORMWATER AND DRAINAGE NARRATIVE	2
1.1.	Existing Conditions	2
1.2.	Proposed Conditions	4
2.0	DRAINAGE DESIGN METHODOLOGY AND ANALYSIS	6
3.0	COMPLIANCE WITH MADEP STORMWATER STANDARDS	9
4.0	CONSTRUCTION ACTIVITIES AND GENERAL CONSTRUCTION SEQUENCE	12
5.0	POLLUTANT CONTROLS DURING CONSTRUCTION	13
5.1.	Structural Practices	13
5.2.	Stabilization Practices	15
5.3.	Other Types of Controls	15
6.0	STORMWATER OPERATION AND MAINTENANCE PLAN	18
7.0	REFERENCES	18

FIGURES

Figure 1:	USGS Locus
Figure 2:	Soils Map
Figure 3:	FEMA Flood Zones
Figure 4:	Site Constraints

APPENDICES

- Appendix A: Site Soil Evaluations
- Appendix B: Drainage Area Maps
- Appendix C: GSI Sizing Calculations
- Appendix D: HydroCAD® Modeling
- Appendix E: TSS and Recharge Calculations
- Appendix F: Stormwater Operation and Maintenance Plan

1.0 STORMWATER AND DRAINAGE NARRATIVE

This Stormwater Management Report provides a summary of the proposed stormwater management for the Cape View Way housing development project (Project). The purpose of this report is to describe the pre- and post-development site conditions and the practices to be used for reducing stormwater runoff and pollutants during and after construction. The proposed project has been developed to incorporate a series of green stormwater infrastructure (GSI) practices into the overall site and landscape design. The design includes practices such as tree trenches, vegetated bioretention systems, and underground recharge areas to manage the onsite runoff.

Due to the proximity of the proposed Project to existing wetlands, adherence to the Massachusetts Stormwater Standards (MASWS) (revised in January 2008) is required. The proposed site design conforms to the Standards by providing stormwater runoff treatment or the first 1-inch runoff from proposed impervious areas contributing to site runoff. On-site attenuation and infiltration is provided to match or reduce peak runoff conditions for the 10 and 100-year storm events. The proposed stormwater controls will be maintained during as part of the development regular landscape maintenance as well as during the construction.

1.1. Existing Conditions

The project site is located at Cape View Way off of Meetinghouse Lane in the Town of Bourne, Massachusetts (Latitude: 41°46'57.8"N, Longitude: 70°32'17.2"W) (**Figure 1**) and includes the following parcels:

Map/Parcel	Street Address	Lot Size (acres)
Map 7, Parcel 23	0 Cape View Way	0.263
Map 7, Parcel 86	6 Cape View Way	0.478
Map 7, Parcel 87	8 Cape View Way	0.475
Map 7, Parcel 88	10 Cape View Way	0.491
Map 7, Parcel 89	12 Cape View Way	0.528
Map 7, Parcel 90	11 Cape View Way	0.542 ¹
Map 6, Parcel 38	0 Homestead Road Extension	0.159

Table 1. Project Site Parcel Information

¹ This is the value in the Bourne Assessors Database. HW's research indicates a slightly larger size lot.

The site is bordered by Cherry Hill Court to the east and Meetinghouse Lane and commercial properties to the south. The site abuts residential properties located on Homestead Road, Homestead Road Extension, and Andrew Road to the west. The site is near the Bourne Fire Department and United States Postal Service, located in an area near transit and amenities.

The site is currently undeveloped and is characterized by dense vegetation and invasive species. The existing Cape View Way road has a paved surface that extends approximately 145 feet from Meetinghouse Lane. The road then continues as a dirt road. There are two abandoned hydrants along the road.

There is a wetland on the western portion of the site, which was confirmed during a field survey conducted by Horsley Witten Group, Inc. in May 2019. This wetland was delineated and flagged in accordance with methods developed by MassDEP, the Massachusetts Wetlands Protection Act regulations, and the Bourne *Wetlands Protection* Regulations.

The existing drainage area is 7.96 acres (346,654 square feet) and is comprised of the following land cover:

	Area	Area	
Coverage	(ft2)	(acres)	%
Roadway	5,583	0.13	2%
Sidewalks	0	0.00	0%
Roof	9,642	0.22	3%
Gravel	1,705	0.04	0%
Pervious Pavers	0	0.00	0%
Pond/SW	0	0.00	0%
Forest (Type A)	297,534	6.63	86%
Grass (Type A)	32,190	0.74	9%
Subcatchment total	346,654	7.96	100%

Table 2: Existing Land Coverage

The site is divided into three sub catchments, DA1, DA2, and DA3, which discharges via overland flow to the wetlands Study Point 1 (SP1), Study Point 2 (SP2), and Study Point 3 (SP3).

DA1 is approximately 3.64 acres, includes the north and western portion of the site and is comprised of woodlands, grass, roofs and roadway from abutting properties. The area gently slopes from the northwest to a wetland on the western portion of the site, SP1.

DA2 is approximately 1.83 acres, includes the central part of the site and is comprised of woodlands and grass. The area gently slopes from the northeast and the ends in on the southwestern border of the site. Runoff appears to flow overland to SP2.

DA3 is approximately 2.48 acres and located in the eastern part of the property and is comprised of woodlands, roadway, gravel, and grass. The area gently slopes from the northwest to the south towards Meetinghouse Lane, SP3.

1.1.1. Soils

According to the "Soil Survey of Barnstable County, Massachusetts" (Fletcher, 1993) soils underlying the Site are classified as Carver Loamy Coarse Sand (Figure 2). This soil group is classified as hydrologic soil group A and described as "very deep, gently sloping, excessively drained soil generally is in broad areas on outwash plains but is also in areas of sandy glacial lake deposits."

Nine site soil evaluation test pits were performed in October 2019 to complete a Phase 2 environmental assessment and assess the subsurface conditions to determine its suitability for the construction of wastewater and stormwater management practices. Infiltrometer testing was conducted at one test pit. The test pits were conducted by a Massachusetts Licensed Soil Evaluator. The soils were determined to be a sandy permeable soil. No standing water was observed in any of the test pits. A memo describing the soil test pits is included in Appendix A.

1.2. Proposed Conditions

The Applicant proposes to construct the following:

- 42 dwelling units located in one building
- Approximately 800 linear feet of paved access road and paved parking areas for 73 spaces (46,508 sf)
- ADA accessible sidewalks (4,553 sf)
- Interior landscaped areas, open spaces, and lighting.

1.2.1. Stormwater Management

The proposed stormwater management includes a GSI approach to capture, treat, infiltrate, and detain runoff, when applicable and to the maximum extent practicable, by using the following BMPs.

Bioretention Areas (BIO)

A bioretention area (also referred to as a "rain garden" or a "biofilter") is a stormwater management practice to manage and treat stormwater runoff using a conditioned planting soil bed or "filter" media and plants to filter runoff captured in a shallow depression. The method combines physical filtering and adsorption with bio-geochemical processes to remove pollutants. The system consists of an inflow component, a pretreatment element, an overflow structure, an underdrain, a shallow ponding area (3 to 6 inches deep), a well-drained planting soil bed, and plants.

Tree Trench (TT)

A tree trench is a tree pit with underground infiltration trenches. The tree trench uses a stone reservoir and planting soils (within the tree pit). The systems are designed to be off-line; meaning the that the water quality treatment volume is diverted into the trenches and an overflow pipe is provided to convey larger storms to the drainage pipe network. The system consists of a perforated inflow pipe, pretreatment via deep sump catchbasins, an overflow pipe out of the catchbasins, the stone (and soil for tree trenches) storage reservoir, and street trees (for the tree trenches).

Underground Recharge Chambers (URC)

Underground recharge chambers and basins capture, and store stormwater collected from surrounding impervious areas. Riser pipes, curb cuts, and/or drainage structures direct surface

stormwater to subsurface interconnected storage units. When site conditions are appropriate, stored water is released directly into the ground mimicking pre-development conditions. Use of stormwater recharge chambers allows stored water to infiltrate and recharge groundwater.

Dry Well (Recharge Basin (RB))

A subsurface stormwater facility that is designed to collect and temporarily store runoff before infiltration into the subsoil. Use of stormwater recharge basins allows stored water to infiltrate and recharge groundwater.

Sediment Forebays

Sediment forebays are also provided at the bioretention areas for pretreatment of the surface water runoff from the proposed pavement and concrete surfaces to allow for sediment to settle from the incoming stormwater runoff prior to conveyance to the bioretention and infiltration basin areas. The forebays are designed to provide less volume than required by the Massachusetts Stormwater Standards. This is due to limited space and to reduce unsightly large quantities of sediment accumulation over time. The sediment forebays are designed to be smaller and require more frequent cleaning. See the Stormwater Operation and Maintenance Plan.

Deep Sump Catch Basins (CB)

Deep sump catch basins equipped with a hooded outlet are also provided to remove trash, debris, and coarse sediment from stormwater runoff.

1.2.2. Drainage Area

The proposed site development includes a low impact stormwater management approach, private septic, public water, natural gas and other associated utilities. The total proposed development is comprised of the following land cover:

	Area	Area	
Coverage	(ft2)	(acres)	%
Roadway	46,508	1.07	13%
Sidewalks	4,553	0.10	1%
Roof	28,678	0.66	8%
Gravel	0	0	0%
Pervious Pavers	1,265	0.03	0%
Pond/SW	2,145	0.05	1%
Forest (Type A)	185,226	4.25	53%
Grass (Type A)	78,279	1.80	23%
Subcatchment total	346,654	7.96	100%

Table 3: Proposed Land Coverage

The proposed site drainage is divided into nine subcatchments: DA1, DA2a, DA2b, DA2c, DA3a, DA3b, DA3c, DA3d, and R1. The nine drainage areas drain to same three study points

(SP1, SP2, and SP3) as outlined in the existing conditions above. Pre and Post Drainage maps can be found in **Appendix B**.

DA1 is approximately 3.64 acres, located in the north and western portion of the site and is comprised of woodlands, grass, roofs and roadway from abutting properties and runoff will continue to drain via overland flow to the wetland (SP1). This drainage area remains unchanged from existing conditions.

DA2a is approximately 0.51 acres, located in the central portion of the property, and includes behind the building pervious pavers, grass, and woodlands. The area drains north to south and runoff will be captured in a recharge basin.

DA2b is approximately 0.33 acres, located in the central portion site and driveway turnaround area. The area includes sidewalks, roadway, and landscaped areas. The area slopes from north to south. Runoff is captured at a curb inlet/sediment forebay at BIO-2 for treatment and discharged to underground chambers (URC-3) for infiltration.

DA2c is approximately 0.74 acres, located along the side of the building and pervious portion in front of the building. The area includes grass and woodlands. The area slopes north to south and drains overland to SP2.

DA3a is approximately 1.11 acres, located along the northern boundary of the site and consists of the upper parking lot. The area includes driveway, parking areas, sidewalks, woodlands, and landscaped areas. The area slopes from north to south. Runoff is captured at a curb inlet/sediment forebay at BIO-1 for treatment and discharged to underground chambers (URC-1) for infiltration.

DA3b is approximately 0.39 acres, located on the southwestern boundary of the site. The area includes the lower parking lot, roadway, sidewalks, woodlands, and landscaped areas. Runoff is captured in a deep sump catch baingan direct to a tree trench (TT1) for treatment and underground chambers (URC-1) for infiltration.

DA3c is approximately 0.67 acres, located on the southern boundary of the site. The area includes roadway, sidewalks, woodlands, and landscaped areas. Runoff is directed to localized recharge basins for infiltration.

DA3d is approximately 0.11 acres, located in the southern part of the site. The area includes parking lot, roadway, sidewalks, and landscaped areas. Runoff is directed to a tree trench (TT2) for treatment and recharge basins for infiltration.

R1E and R1W are approximately 0.31 and 0.21 acres, respectively, and it is the proposed building's roof. Runoff is directed via roof leaders to underground chambers (URC-2 and URC-3) for infiltration.

2.0 DRAINAGE DESIGN METHODOLOGY AND ANALYSIS

The drainage design was completed by performing the following series of tasks:

- Site soil evaluations (9 test pits) (Appendix A)
- Delineation of drainage areas and sub catchments (Appendix B)
- Sizing the bioretention areas, tree trenches and underground recharge chambers (Appendix C)
- Modeling the proposed drainage network with HydroCAD® software (Appendix D)
- TSS and Recharge calculations (Appendix E)

Nine soil test pits were excavated; six test pits were for the purposes of the Phase 2 environmental assessment and three test pits were for assess subsoil conditions for wastewater and stormwater practices. No standing water was found in any of the test pits. Redoximorphic features (mottling) were found in two test pits: TP-E & TP-F. In TP-E, two lenses of mottling were observed: one in the sandy loam just above the silt loam layer, and one within the sandy loam layer just below the silt loam layer. Based on the soil evaluator's judgement, both sets of mottles are due to the interface between the loamy sand and silt loam layers and is indicative of the restriction of infiltration of surface water caused by this textural change and that this is a sign of a perched water table in this area of the site. Based upon our field observations and the topography in this area, it appears that the wetland on the west side of the site is the result of runoff that most likely settles in the area and becomes "trapped" in the subsurface, unable to infiltrate and creating the perched water table. The soil test pit data are included in **Appendix A**.

The site is located approximately half a mile from the Cape Cod Canal, a navigable ocean channel connecting Cape Cod Bay with Buzzards Bay. The lowest test pit (TP-F) observed was at ground surface elevation 63.5 and was excavated to a depth of 10 feet. No redoximorphic features, seepage, or restrictive layers were encountered in this excavation. Comparable regional groundwater monitoring wells less than a mile from the ocean indicate a high groundwater elevation of approximately 5 to 10. The recommended design Estimated Seasonal High Ground Water elevation is 10.0 feet.

A double-ring infiltrometer test was performed at TP-F which resulted in an infiltration rate of 7.0 inches/hour. Based on the infiltration test data results, existing subsoils, and deep depth to groundwater, this site is ideal for stormwater infiltration, confirming prior assumptions and expectations for the proposed site and drainage design other than the area of TP-E.

Soil logs are provided in **Appendix A** and the test pits locations are located on the Grading and Drainage Plan.

The Stormwater Management System has been designed to accomplish the following major objectives:

• To capture and treat, at a minimum, the "first flush" (first one-inch of stormwater runoff) from the impervious surfaces to maintain or improve water quality conditions when compared to existing conditions.

- To provide groundwater recharge to the greatest extent practicable in conformance with the Massachusetts Department of Environmental Protection groundwater recharge criteria.
- To minimize runoff from the post-developed conditions at the study point located along the periphery of the site.

These objectives are met through the use of the following stormwater management measures:

- Bioretention systems and tree trenches sized to treat the first one-inch of stormwater runoff for water quality treatment of runoff from the driveway, walkways, and parking areas. The systems are equipped with overflows to convey runoff from larger storm events into proposed underground recharge chambers. (Appendix C).
- Underground recharge chambers and recharge basins sized to retain and infiltrate onsite runoff.

The proposed Stormwater Management System was designed to accommodate predevelopment site hydrologic conditions as well reduce stormwater pollution from the proposed site conditions. Stormwater runoff quantity was evaluated for the 2-year, 10-year, 25-year, and 100-year Type III, 24-hour storm events for both pre-development and post-development conditions. Per the town's subdivision provisions, the Stormwater Management System was designed to detain runoff from the 25-year event. Pre-development and post-development conditions were modeled using HydroCAD software, which combines USDA Soil Conservation Service hydrology and hydraulic techniques (commonly known as SCS TR-55 and TR-20) to generate hydrographs (See **Appendix B** for both "Pre-developed" and "Post-developed" Drainage Area Maps). The model was run for the 100-year events to ensure peak flows would be managed. The rainfall amounts used for calculating runoff for the storm events were obtained from the Cornell University Extreme Precipitation Events for the area (**Appendix D**). A summary table of pre- and post-development runoff peak flow rates and volumes is provided in Table 3.

Stormwater runoff quality was evaluated to ensure that pollutant export from the project site was minimized to the maximum extent practicable. The stormwater management system for the site was designed in accordance with the MASWS and the applicable criteria within the Town's Subdivision Regulations and Zoning Bylaw. The proposed stormwater management system detains runoff from the 25-year event.

Table 4: Peak Flow and Volume Comparison

DECICN	PRE-DEVEL	OPMENT	POST-DEVE	LOPMENT	PERCENT REDUCTION			
STORM	PEAK FLOW (CFS)	EAK FLOW VOLUME (CFS) (AF)		VOLUME (AF)	PEAK FLOW	VOLUME		
2 YR	0.00	0.000	0.00	0.000	0%	0%		
10 YR	0.07	0.045	0.07	0.045	0%	0%		
25 YR	0.53	0.129	0.53	0.129	0%	0%		

STUDY POINT 1

STUDY POINT 2

DESIGN	PRE-DEVELO	OPMENT	POST-DEVE	LOPMENT	PERCENT REDUCTION		
STORM	PEAK FLOW (CFS)	VOLUME (AF)	PEAK FLOW (CFS)	VOLUME (AF)	PEAK FLOW	VOLUME	
2 YR	0.00	0.000	0.00	0.000	0%	0%	
10 YR	0.00	0.001	0.00	0.003	0%	-200%	
25 YR	0.03	0.018	0.02	0.014	33%	22%	

STUDY POINT 3

DECION	PRE-DEVEL	OPMENT	POST-DEVE	LOPMENT	PERCENT REDUCTION		
STORM	PEAK FLOW (CFS)	VOLUME (AF)	PEAK FLOW (CFS)	VOLUME (AF)	PEAK FLOW	VOLUME	
2 YR	0.00	0.000	0.00	0.000	0%	0%	
10 YR	0.00	0.002	0.00	0.000	0%	100%	
25 YR	0.04	0.024	0.00	0.000	100%	100%	

3.0 COMPLIANCE WITH MADEP STORMWATER STANDARDS

The Massachusetts Stormwater Standards were revised in February 2008 to include ten stormwater management standards, established jointly by the DEP and the Office of Coastal Zone Management and published in the 2008 update of the Stormwater Management Handbook. Projects that are within the jurisdiction of the Wetlands Protection Act Regulations, 310 CMR 10.00 are subjected to these Stormwater Management Standards. For this project, adherence to the Handbook is required as the project is within the jurisdiction of the Wetlands Protection Act. Therefore, the stormwater management system was designed in accordance with the MASWS.

The following is a list of Stormwater Management Standards and accompanying documentation describing compliance of the proposed retrofit project with each Standard:

1. No new stormwater conveyances (e.g. outfalls) may discharge untreated stormwater directly to or cause erosion in wetlands or waters of the Commonwealth.

No new untreated stormwater will discharge to wetland areas.

2. Stormwater management systems shall be designed so that post-development peak discharge rates do not exceed pre-development peak discharge rates.

Discharge rates for pre and post-development were calculated using HydroCAD® 2010, and SCS-TR20 based stormwater modeling computer program (**Appendix D**). Post-development peak discharge rates are less than pre-development rates for the 2-, 10-, and 25- storms. A summary table of these precipitation events is provided in Table 3. Rainfall values from the NRCC Extreme Precipitation for New England database were utilized for this analysis.

3. Loss of annual recharge to groundwater shall be eliminated or minimized through the use of infiltration measures including environmentally sensitive site design, low impact development techniques, stormwater best management practices, and good operation and maintenance. At a minimum, the annual recharge from the post-development site shall approximate the annual recharge from pre-development conditions based on soil type. This Standard is met when the stormwater management system is designed to infiltrate the required recharge volume as determined in accordance with the Massachusetts Stormwater Handbook.

Under the proposed design, the stormwater runoff is being directed to recharge basins and underground recharge chambers. The intent is to recharge groundwater to the maximum extent practicable as required by Standard 3. Since the site is characterized with a high infiltration rate (greater than 2.4 in/hr.), at least 44% of the total suspended solids must be removed prior to discharge to the infiltration structure. The required TSS pretreatment will be done through deep sump catch basins, bioretention practices, and tree trenches. TSS and recharge calculations are provided in **Appendix E**.

4. Stormwater management systems shall be designed to remove 80% of the average annual post-construction load of Total Suspended Solids (TSS). This Standard is met when:

- Suitable practices for source control and pollution prevention are identified in a long-term pollution prevention plan, and thereafter are implemented and maintained;
- Structural stormwater best management practices are sized to capture the required water quality volume determined in accordance with the Massachusetts Stormwater Handbook; and
- Pretreatment is provided in accordance with the Massachusetts Stormwater Handbook.

The stormwater management practices are sized to capture the required water quality volume (**Appendix C**).

The stormwater management pretreatment and treatment systems for the sites have been selected and sized for the most removal of the average annual load of TSS possible. The following removal rates were taken from MA Stormwater Handbook:

Sediment Forebay or Deep Sump Hooded Catch Basin: Tree Trench (Treebox filter) Bioretention (with sediment forebay):

Recommended design rate: 25% Recommended design rate: 80% Recommended design rate: 90%

TSS calculations are provided in **Appendix E.** Source controls and pollution prevention will be controlled by the methods outlined in **Sections 5.0 and 7.0**. The proposed Operation and Maintenance Plan was developed to ensure that the stormwater system continues to function as it was designed into the future (**Appendix F**).

5. For land uses with higher potential pollutant loads, source control and pollution prevention shall be implemented in accordance with the Massachusetts Stormwater Handbook to eliminate or reduce the discharge of stormwater runoff from such land uses to the maximum extent practicable. If through source control and/or pollution prevention all land uses with higher potential pollutant loads cannot be completely protected from exposure to rain, snow, snow melt, and stormwater runoff, the proponent shall use the specific structural stormwater BMPs determined by the Department to be suitable for such uses as provided in the Massachusetts Stormwater Handbook. Stormwater discharges from land uses with higher potential pollutant loads shall also comply with the requirements of the Massachusetts Clean Waters Act, M.G.L. c. 21, §§ 26-53 and the regulations promulgated thereunder at 314 CMR 3.00, 314 CMR 4.00 and 314 CMR 5.00.

The site is not considered a LUHPPL; thus, this standard is not applicable.

6. Stormwater discharges within the Zone II or Interim Wellhead Protection Area of a public water supply, and stormwater discharges near or to any other critical area, require the use of the specific source control and pollution prevention measures and the specific structural stormwater best management practices determined by the Department to be suitable for managing discharges to such areas, as provided in the Massachusetts Stormwater Handbook. A discharge is near a critical area if there is a strong likelihood of a significant impact occurring to said area, taking into account site-specific factors. Stormwater discharges to Outstanding Resource Waters and Special Resource Waters shall be removed and set back from the receiving water or wetland and receive the highest and best practical method of treatment. A "storm water discharge" as defined in 314 CMR 3.04(2)(a)1 or (b) to an Outstanding Resource Water or Special Resource Water shall comply with 314 CMR 3.00 and 314 CMR 4.00. Stormwater discharges to a Zone I or Zone A are prohibited unless essential to the operation of a public water supply.

The project site is not located within a Zone II or Interim Wellhead Protection Area. The stormwater discharges are near to a wetland on the western portion of the site.

The project proposes to use stormwater pretreatment, treatment, and infiltration BMPs identified in Standard 6. Sediment forebays and deep sump catch basins are approved pretreatment BMPs, filtering bioretention areas (included tree trenches) are approved treatment BMPs with 44% TSS reduction prior to treatment occurring, and dry wells and subsurface structures are approved infiltration BMPs.

7. A redevelopment project is required to meet the following Stormwater Management Standards only to the maximum extent practicable: Standard 2, Standard 3, and the pretreatment and structural best management practice requirements of Standards 4, 5, and 6. Existing stormwater discharges shall comply with Standard 1 only to the maximum extent practicable. A redevelopment project shall also comply with all other requirements of the Stormwater Management Standards and improve existing conditions.

This project is not a redevelopment project, therefore, Standard 7 does not apply.

8. A plan to control construction-related impacts including erosion, sedimentation and other pollutant sources during construction and land disturbance activities (construction period erosion, sedimentation, and pollution prevention plan) shall be developed and implemented.

An Erosion and Sediment Control Plan is included in construction documents, and a Pollutant Prevention Plan is included in this Report. Silt fence and/or silt socks are proposed at the limit of work; silt socks are proposed along the downgradient edges of the area of disturbance. Disturbed areas will be stabilized with seeding and/or erosion control blankets, if necessary, as soon as possible to minimize erosion and sedimentation. Additional pollutant controls during construction are described in **Section 5.0** and on the plans. A Stormwater Pollution Plan (SWPPP) is required as part of the NPDES Construction General Permit and will be submitted prior to construction.

The contractor will be required to establish erosion controls prior to beginning any other projectrelated work. The Erosion and Sediment Control Plan will also establish the limit of work, beyond which the contractor will not be allowed to perform any project work. It is the contractor's responsibility to monitor and correct erosion control practices throughout the duration of the project. Erosion control measures will not be removed until the project reaches completion as directed by the project engineer or landscape architect.

9. A long-term operation and maintenance plan shall be developed and implemented to ensure that stormwater management systems function as designed.

The long-term stormwater operation and maintenance plan for each stormwater best management practice is discussed in **Section 6.0** and provided with this report in **Appendix F**.

10. All illicit discharges to the stormwater management system are prohibited.

An Illicit Discharge Statement will be provided in the SWPPP prior to construction.

4.0 CONSTRUCTION ACTIVITIES AND GENERAL CONSTRUCTION SEQUENCE

Construction activities will involve site preparation and earthwork necessary for construction of the proposed project. These activities primarily include the following:

- Erosion control installation
- Clearing and grubbing of existing vegetation within the proposed limits of work

- Excavation stockpiling, and hauling of excavated foundation, topsoil and subsoils
- Rough grading of all disturbed areas
- Construction of new housing unit
- Construction of stormwater management system
- Installation of utilities
- Paving
- Finish grading, final site stabilization and landscaping

Erosion and sediment control (ESC) measures will be installed per the construction plans and specifications prior to commencement of any soil disturbing activities. ESC measures will remain in place until final site stabilization is complete. Topsoil will be separated from the remaining soil and stockpiled on-site for use during site finish grading. The stockpiled topsoil will be protected to prevent erosion and sedimentation.

5.0 POLLUTANT CONTROLS DURING CONSTRUCTION

Controls will be used to reduce erosion during the construction period. Perimeter controls and sediment settling devices will be installed during construction to minimize sediment movement in stormwater and to protect the adjacent properties and buffers on the property.

5.1. Structural Practices

The following are the structural practices that will be implemented as part of the construction activity.

- <u>Silt Fence & Sediment Silt Sock Barrier</u> will be installed prior to commencement of construction. This type of practice creates erosion control barriers to intercept sediment in diffuse runoff. The Town will be informed upon installation so that they may inspect these barriers prior to construction. Portions of the erosion control barriers will be replaced and/or repaired as necessary to prevent erosion. Barriers will be installed parallel to land slope at the perimeter of the work site. In addition, silt fence barriers will be installed around the bioretention areas during construction.
- <u>Silt Sacks (or approved equivalent)</u> will be installed at identified existing catch basins and structure following construction of the proposed catch basins to prevent sedimentation during the any additional construction. The Silt Sack will be replaced and disposed of off-site if damage is observed.
- <u>Bioretention Area(s)</u> will be graded to within one foot of design elevations until site is fully stabilized to capture sediment during construction. Heavy equipment will not be allowed to operate on the surface location where the systems are planned because soil compaction would adversely impact their long-term performance. Silt fence will be utilized around the perimeter of the bioretention systems during construction. Light

earth-moving equipment will be used for excavation and construction of the systems. All excavated materials from the area will be removed and disposed of in an approved location. All bioretention areas will be inspected at least once every seven calendar days and immediately after storm events by the Site Superintendent.

• <u>Slope Stabilization</u> will be installed immediately upon obtaining final grades as shown on the project site plans. Areas that fail to stabilize will be re-graded to final grade and stabilized as necessary. Amount of land disturbed will be minimized to reduce potential for erosion and sedimentation. Stabilization measures shall be initiated within 14 days following the end of construction at each portion of the site and as soon as practicable.

The entire stormwater management system including overflow spillway and sediment forebay will be inspected upon completion of construction. Sediment will be removed from all elements of the stormwater management system. All control measures must be installed and maintained in accordance with manufacturer's specifications, good engineering practices, and in accordance with this Plan (every seven calendar days and after storm events). If inspections show that a control has failed or been installed incorrectly, the Operator must replace or modify it within 24 hours.

Structural controls will be regularly inspected to ensure proper performance. The following operation and maintenance provisions will be provided:

- Silt fences will be inspected for depth of sediment, tears, to determine if the fabric is securely attached to the fence posts, and to determine if the fence posts are firmly in the ground. Silt fence will be replaced when necessary.
- Silt Socks shall be inspected for depth of sediment and any breaches will promptly be repaired or replaced when necessary.
- Sediment shall be removed where accumulation reaches one-third the above ground height of any barrier.
- Once each workday structural control measures receiving flows from areas that have not been stabilized shall be inspected.
- Remedial action shall be taken in areas where temporary and permanent seeding is deemed inefficient through weekly inspections to establish a stabilized surface.
- All BMP's will be cleared of accumulated foreign debris, including leaves and lawn cuttings.
- All BMP's will be inspected for slope integrity and erosion.
- All control measures will be inspected at least once every 7-calendar days and within 24 hours after storm events of 0.5 inches or more.

• All measures will be maintained in good working order, if a repair is necessary, it will be initiated within 24 hours of discovery.

5.2. Stabilization Practices

The amount of land disturbed during construction will be minimized to reduce the potential for erosion and sedimentation. Prompt surface stabilization will be practiced to control erosion in areas where disturbances cannot be avoided during construction. Stabilization measures shall be initiated within 14 days following the end of construction at each portion of the site. Exceptions to this requirement are allowable when snow cover prevents the initiation of stabilization within 14 days, in which case such measures shall be undertaken as soon as possible.

Stabilization measures that may be used during construction are described below:

- <u>Temporary Seeding</u> Temporary seeding of disturbed surfaces with fast-growing grasses (annual rye) to provide greater resistance to stormwater runoff and/or wind erosion for areas where construction has temporarily ceased.
- <u>Permanent Seeding</u> Permanent seeding of surfaces with vegetation, including but not limited to grass, trees, bushes, and shrubs, to stabilize the soil. Establishing a permanent and sustainable ground cover at a site stabilizes the soil while reducing the sediment content in runoff.
- <u>Permanent Planting</u> the contractor shall install and adequately establish all planting as required at the completion of the project.
- <u>Mulching/Hydro mulching</u> hydro mulch will be placed on the soil surface to cover and hold in place disturbed soils.

Temporary seeding or other soil stabilization measures will be provided where construction activities have ceased at the site. Topsoil stockpiles will be temporarily seeded or covered to prevent erosion and will be surrounded with silt fence. When the site's final grade has been established, permanent vegetation will be planted on the disturbed areas. The vegetation will consist of grass, shrubs, bushes, and trees.

5.3. Other Types of Controls

Additional controls/practices will be undertaken to reduce pollution in stormwater runoff flows which include, but are not limited to, control of off-site mud tracking from construction site, dust suppression, proper sanitary waste disposal, earthwork procedures timed and conducted in manners aimed to minimize erosion and sedimentation, snow removal plans, proper management of waste materials, proper management of hazardous waste, proper material stockpiling, and spill prevention and control measures.

• <u>Dust Suppression</u> – Water sprays shall be used to control dust during extended dry periods during construction.

- <u>Sanitary Wastes</u> All sanitary wastes will be collected from the portable units by a licensed sanitary waste management contractor (as required by local regulations).
- <u>Earthwork</u> The exposure of disturbed surfaces to stormwater and potential stormwater erosion will be minimized by well organized earthwork procedures. Stabilization procedures shall be undertaken in accordance with this report. Grubbing during wet seasons will be avoided if feasible.
- <u>Snow Removal Plan</u> Plowed snow collected from the parking areas will be deposited onto free draining, pervious surfaces, away from the site's drainage conveyance structures to maximize infiltration. Snowmelt runoff that is not infiltrated will be directed to the site's stormwater management system. Snow is not to be plowed or piled onto the stormwater management facility or wetlands.
- <u>Waste Materials</u> Dumpsters rented from a licensed solid waste management company will be used to store solid waste and debris that cannot be recycled, reused or salvaged. The dumpsters will meet all local and state solid waste management regulations. Dumpsters will be covered when refuse is not being directly deposited or withdrawn from them. Potentially hazardous wastes will be separated from normal wastes, including segregation of storage areas and proper labeling of containers. Removal of all waste from the site will be performed by licensed contractors in accordance with applicable regulatory requirements and disposed of at either local or regional approved facilities. Waste materials will not be buried on-site. All site personnel will be instructed regarding the correct procedures for waste disposal. Notices stating these procedures will be posted at the site. Solvents and flushing materials used during construction and preoperational cleaning will be provided, handled, managed, and removed by the contractor for appropriate off-site disposal.
- <u>Hazardous Waste Materials</u> Any disposal of hazardous materials will be completed using the required paperwork. Copies will be provided to the Engineer and to the Town.
- <u>Spill Prevention and Control Measures</u> To minimize the risk of spills or other accidental exposure of materials and substances to stormwater runoff, the following material management practices will be used throughout the project:
 - $_{\odot}$ An effort will be made to store only enough products required to do the job.
 - All materials stored on-site will be stored in a neat, orderly manner in their appropriate containers and, if possible, under a roof or other enclosure.
 - Products will be kept in their original containers with the original manufacturer's label.
 - Substances will not be mixed with one another unless recommended by the manufacturer.
 - Whenever possible, the maximum amount of a product will be used before disposing of the container.
 - Manufacturers' recommendations for proper use and disposal will be followed.

• The site superintendent will conduct daily inspections to ensure proper use and disposal of materials.

To reduce the risk associated with hazardous materials used on the site, the following practices will be used:

- Products will be kept in original containers unless they are not resealable.
- Original labels and material safety data sheets will be retained and kept on-site; they contain important product information.
- If surplus product must be disposed of, manufacturers' or local and state recommended methods for proper disposal will be followed.
- <u>Materials List</u> Materials or substances listed below are expected to be present on-site during construction:

- Concrete	- Fertilizers
- Asphalt	- Petroleum Based Products
- Paints (enamel and latex)	- Cleaning Solvents
- Metal Studs	- Wood
- Concrete	- Tar
- Sealants	- Adhesives

The following product-specific practices will be followed on-site:

- <u>Petroleum Products</u> All on-site vehicles will be monitored for leaks and receive preventative maintenance to reduce the chance of leakage. Petroleum products will be stored in tightly sealed containers which area clearly labeled. Any asphalt substances used on-site will be applied according to the manufacturers' recommendations.
- <u>Fertilizers</u> Fertilizers used will be applied only in the minimum amounts recommended by the manufacturer. Once applied, fertilizer will be worked into the soil to limit exposure to stormwater. Products will be stored in a covered shed. The contents of any partially used bags of fertilizer will be transferred to a sealable plastic bin to avoid spills.
- <u>Paints</u> All containers will be tightly sealed and stored indoors when not required for use. Excess paint will not be discharged to the storm sewer system but will be properly disposed of according to the manufacturers' instructions or state and local regulations.
- <u>Concrete Trucks</u> Concrete trucks will not be allowed to wash out or discharge surplus concrete or drum wash water on the site.

In addition to the good housekeeping and material management practices discussed in the previous sections of this plan, the following practices will be followed for spill prevention and cleanup:

- Manufacturers' recommended methods for spill cleanup will be clearly posted, and site personnel will be made aware of the procedures and location of the information and cleanup supplies.
- Materials and equipment necessary for spill cleanup will be kept in the material storage area on-site. Equipment and materials will include, but not be limited to, brooms, dustpans, mops, rags, gloves, goggles, speedi-dry, sand, sawdust, and plastic and metal trash containers specifically for this purpose.
- All spills will be cleaned up immediately after discovery. Spills large enough to reach the storm water system will be reported to the National Response Center at 1-800-424-8802.
- The spill area will be kept well ventilated and personnel will wear appropriate protective clothing to prevent injury from contact with a hazardous substance.
- Spills of toxic or hazardous material will be reported to the appropriate state or local government agency, regardless of the size.
- The site superintendent responsible for the day-to-day site operations will be the spill prevention and clean-up coordinator. He will designate at least three other site personnel who will receive spill prevention and cleanup training. These individuals will each become responsible for a particular phase of prevention and cleanup. The names of responsible spill personnel will be posted in the material storage area and in the onsite office trailer.

6.0 STORMWATER OPERATION AND MAINTENANCE PLAN

All stormwater management measures and controls identified in this report shall be operated and maintained appropriately during the construction phase of the project and during regular operation of the site in the post-construction period as required on the construction drawings and the separate Stormwater Management Maintenance Plan (**Appendix F**).

7.0 REFERENCES

- 1. MADEP (Massachusetts Department of Environmental Protection). 2008. Massachusetts Stormwater Standards Manual.
- Northeast Regional Climate Center and Natural Resources Conservation Service. 2010-2018. Extreme Precipitation for New York and New England. Version 1.12. <u>http://precip.eas.cornell.edu/</u>

FIGURES





Document Path: H:\Projects\2019\19038 Cape View Way\19038A Survey-Environmental Services Cape View Way\GlS\Maps\Soils.mxd





Figure 3





Site Soil Evaluations



MEMORANDUM

TO:	Meena Jacobs – Senior Project Manager – Real Estate Development Preservation of Affordable Housing, Inc.
FROM:	Joe Henderson, P.E.
DATE:	October 28, 2019
RE:	Cape View Way, Bourne MA Site Soil Evaluation
CC:	Brian Kuchar, RLA, P.E.

Horsley Witten Group, Inc. (HW) has conducted a total of 14 soil test pits at Cape View Way to complete a Phase 2 assessment and assess the subsurface conditions and determine its suitability for the construction of wastewater and stormwater management practices. The test pits were spread throughout the site and locations are shown on the attached plan. The results of the nine test pits for stormwater and wastewater soil test pit data are attached. For more information on the Phase 2 assessment, please see the Limited Investigation Subsurface Report dated November 22, 2019.

The soil map units according to the USDA Natural Resources Conservation Service for this location are "fine sandy loam" and "loamy coarse sand." The majority of test pits consisted of varying depths of loamy sand overlaying fine sand with a gravelly lens. The top layers of soil were generally friable and the underlying layers were more firm in place but generally friable in hand. The outlier test pit (TP-E) was closest to the existing wetland (approx. 100 ft away) and consisted of much tighter soils, including a silt loam layer from 66-116" with gleyed soils containing decomposed organic matter underlain by sandy soil.

No standing water was observed in any of the test pits; TP-E encountered seepage at 116" that was held above the restrictive silt loam layer but was determined to be not indicative of the groundwater table.

Redoximorphic features (mottling) were found in two test pits: TP-E & TP-F. In TP-E, two lenses of mottling were observed: one in the sandy loam just above the silt loam layer, and one within the sandy loam layer just below the silt loam layer. In our opinion, both sets of mottles are due to the interface between the loamy sand and silt loam layers and is indicative of the restriction of infiltration of surface water caused by this textural change. A gap between the two sets of mottles was observed where no redoximorphic features could be seen. In TP-F, a similar two lenses of mottling were observed, both of which we believe are a result of textural changes. Based on the soil textures, it is our opinion that the redox in these pits and the seepage in TP-E are not indication of a high water table in this area.

The site is located approximately half a mile from the Cape Cod Canal, a navigable ocean channel connecting Cape Cod Bay with Buzzards Bay. The lowest test pit (TP-F) observed was at ground surface elevation 63.5 and was excavated to a depth of 10 feet with no indication of a seasonal high water table. Comparable regional groundwater monitoring wells less than a mile from the ocean indicate a high groundwater elevation of approximately 5 to 10. The recommended design Estimated Seasonal High Ground Water (ESHGW) elevation is el. 10.0 ft.

Bourne Health Agent Terri Guarino observed percolation tests at TP-6 and TP-B on October 24, 2019. At TP-6, the percolation test was performed from 60" to 75" and the soil was unable to maintain a water height of 12". The full 24 gallons were applied to TP-6, giving the percolation test a default rate of 5 minutes/inch. At TP-B, the percolation test was performed from 44" to 59", resulting in a rate of 3.3 minutes/inch. The Health Agent also observed TP-A, TP-C and TP-D and concluded that the material was of similar nature and did not require further percolation tests. The design percolation rate for sizing wastewater leaching facilities is 5 minutes/inch.

In addition, HW performed a double-ring infiltrometer test at TP-F in the fine loamy sand layer, which resulted in an infiltration rate of 7.0 inches/hour. To be conservative, half of the average observed rate will be used in the design of infiltrating stormwater practices (e.g., 3.5in/hr). Although not tested with the infiltrometer, the underlying sandy layer would likely have an infiltration rate of 8.27 in/hr based on the percolation testing results.

Based upon the results of on-site soil evaluations, the underlying sandy soils onsite are acceptable for wastewater and stormwater infiltration. Stormwater practices located near TP-E and TP-F should be located in the underlying sandy layers below the restrictive soil layers (approx., El. 55 – 57). The extent of restrictive soil layers should be confirmed during construction if necessary.

Test Ho	le Number	:		TP-A	24-Oct-19	Time	1:30 PM		60F, sunny		
				Dat	le	Time			weather		
1. Lo	cation										
	Ground Eleva	ation at Surface c	f Hole	60.8							
	Location (Ide	entify on Plan)	Northeast	t corner of site	e, closest to	Cherry Hill bu	iildings				
2. La	and Use: Wo	oodland				No			0-3		
	(e.ç	g. woodland, agricultu	ral field, vaca	ant lot, etc.)		Surface Sto	nes		Slope (%)		
	mi	x of brush and tre	es	Landfor	m		Position on	landscape (at	tach sheet)		
	· · · · · · · · · · · · · · · · · · ·		L .	Ducing			Dessible				
3. DI	stances from:	Open water Boo	feet	Draina	age way	feet	Possible	vvet Area	feet	_	
		Property Line		Drinking \	Water Well		Other				
			feet			feet				_	
4 Pa	rent Material:					Unsuitable M	laterials Pr	esent: Ye	s No x]	
lf Y	es: Disturb	bed Soil	Fill Materi	ial	Impervious	Layer(s)	We	eathered/F	ractured Rock	Bedrock	
5 Gr	oundwater Ob	oserved: Yes	No	X							
lf \	es: Depth	Weeping from P	it		Depth	Standing Wa	ter in Hole				
Га	timated Dapth	to High Cround	votori			J			Observed		
E9	umated Depti		valer.	feet		elevation			elevatio	on	
Τe	est Pit Num	ber:			TP-A						
Depth (In.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redo	oximorphic Fe (mottles)	eatures	Soil Texture (USDA)	Coarse F % by	ragments Volume	Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
0-10	A	10 YR 4/2	-	-	-	SL	<5	-			
10-38	Bw	10YR 4/6	-	-	-	SL	-	-			
38-50	C1	10YR 6/3	-	-	-	FS	-	-			
50-68	C2	10YR 6/3	-	-	-	G-FS	50	10			

Test Ho	ole Number	r:		TP-B	24-Oct-19	Time	2:00 PM		60F, sunny		
1. Lo	ocation			20.							
	Ground Elev	ation at Surface c	f Hole	63.1			-				
	Location (Ide	entify on Plan)	North edg	e of the site,	near Cherry	Hill property	line (west o	of A, north	of C)		
2. La	and Use: W	oodland	ral field vaca	ant lot etc.)		No Surface Sto	nes		0-3 Slope (%)		
	mi	x of brush and tre	es								
	Ve	getation		Landfor	m		Position on	landscape (at	tach sheet)		
3. Di	stances from:	Open Water Boo	ly feet	Draina	age Way	feet	Possible	Wet Area	feet	-	
		Property Line	feet	Drinking	Water Well	feet	Other				
4 Pa	rent Material:					Unsuitable N	laterials Pr	esent: Ye	s No 🗴]	
lf Y	es: Disturt	bed Soil	Fill Materi	al	Impervious	Layer(s)	We	eathered/Fr	ractured Rock	Bedrock	
5 Gr	oundwater Ot	oserved: Yes	No	X							
lf Y	/es: Depth	Weeping from P	it		Depth	Standing Wa	ter in Hole				
Es	timated Depth	n to High Groundv	vater:						Observed		
				feet		elevation			elevatio	n	
Τe	est Pit Num	ber:			TP-B						
Depth (In.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redo	oximorphic Fe (mottles)	eatures	Soil Texture (USDA)	Coarse F % by `	ragments Volume	Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
0-6	А	10 YR 4/2	-	-	-	SL	<5	-			
6-36	Bw	10YR 5/8	-	-	-	SL	-	-			
36-66	C1	10YR 6/4	-	-	-	MS	-	-			
66-84	C2	10YR 6/4	-	-	-	G-FS	30	10			

Test Ho	ole Number			TP-C	24-Oct-19	Time	3:00 PM		60F, sunny		
1. Lo	cation			Dui		Time			Would		
	Ground Eleva	ation at Surface c	of Hole	62.9							
	Location (Ide	ntify on Plan)	South side	e of TP B (tov	vard Meetin	ahouse Rd)	-				
2 1						No			0.2		
2. Li	2. Land Use: Woodland (e.g. woodla		ral field, vaca	ant lot, etc.)		Surface Sto	nes		Slope (%)		
	mi	x of brush and tre	es								
Vegetation				Landfor	m		Position on	landscape (at	tach sheet)		
3. Di	stances from:	Open Water Boo	ly feet	Draina	age Way	feet	Possible	Wet Area	feet	_	
		Property Line	feet	Drinking	Water Well	feet	Other				
4 Pa	rent Material:					Unsuitable M	laterials Pr	resent: Ye	s No 🗴		
lf \	es: Disturb	oed Soil	Fill Materi	ial	Impervious	Layer(s)	We	eathered/Fi	ractured Rock	Bedrock	
5 Gr	oundwater Ob	served: Yes	No	X							
lf \	es: Depth	Weeping from P	it		Depth	Standing Wa	ter in Hole				
F۹	timated Denth	to High Ground	water:			C C			Observed		
20				feet		elevation			elevati	on	
Τe	est Pit Numl	ber:			TP-C						
Depth (In.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redo	oximorphic Fe (mottles)	eatures	Soil Texture (USDA)	Coarse F % by `	ragments Volume	Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
0-5	А	10 YR 4/2	-	-	-	SL	<5	-			
5-45	Bw	10YR 5/8	-	-	-	SL	-	-			
45-68	C1	10YR 6/4	-	-	-	MS	-	-			
68-80	C2	10YR 6/4	-	-	-	G-FS	30	10			

Test Hole Number:			TP-D 24-Oct-19 Date			4:00 PM		60F, sunny Weather					
1. Lo	cation												
	Ground Elev	ation at Surface o	of Hole	61									
	Location (Ide	entify on Plan)	South side	e of TP A (far	ther from C	herry Hill)							
2. La	and Use: W	oodland	ural field waar	ant lot oto)		No Surfago Sta	200		0-3				
	(e.g. woodand, agricultural field, v			ant lot, etc.)		Surface Sid	nes		Slope (%)				
	Ve	getation	,00	Landfor	m		Position on landscape (attach sheet)						
3. Di	stances from:	Open Water Boo	dy feet	Draina	age Way	feet	Possible Wet Area						
		Property Line	feet	Drinking	Water Well	feet	Other						
4 Pa	rent Material:					Unsuitable N	laterials Pr	esent: Ye	s No	x			
lf \	es: Disturt	bed Soil	Fill Materi	ial	Impervious	Layer(s)	We	eathered/Fi	ractured Rock	Bedrock			
5 Gr	oundwater Ot	oserved: Yes	No No	X									
lf \	es: Depth	Weeping from P	it		Depth	Standing Wa	ter in Hole						
Fo	timated Dopth	to High Cround	water:			5			Observed				
E3	umateu Depti		waler.	feet		elevation			eleva	ation			
Τe	est Pit Num	ber:			TP-D								
Depth (In.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redo	oximorphic Fe (mottles)	c Features Soil es) Texture (USDA		Coarse F % by `	ragments Volume	Soil Structure	Soil Consistence (Moist)	Other		
			Depth	Color	Percent		Gravel	Cobbles & Stones					
0-8	А	10 YR 4/2	-	-	-	SL	<5	-					
8-40	Bw	10YR 5/8	-	-	-	SL	-	-					
40-62	C1	10YR 6/4	-	-	-	MS	-	-					
62-74	C2	10YR 6/4	-	-	-	G-FS	30	10					

Test Ho	le Number	:		TP-E	25-Oct-19	Time	9:00 AM		60F, sunny		-			
	_			Dat	e	Time			weather					
1. Lo	cation													
	Ground Eleva	ation at Surface o	of Hole	66.2			_							
	Location (Ide	entify on Plan)	Between	proposed buil	ding footprii	nt and wetlan	d (Northwe	st side of b	uilding)					
2. La	and Use: Wo	oodland				No			0-3					
	(e.g. woodland, agricultural field, vac			ant lot, etc.)		Surface Sto	ones		Slope (%)		-			
	Su	mac trees, lower	brush	<u> </u>						x Bedrock vation				
Vegetation				Landfor	m		Position on							
3. Dis	stances from:	Open Water Boo	dy	Draina	age Way	feet	Possible	Wet Area	feet					
		Droporty Lino		Drinking \	Mator Mall		Othor							
			feet			feet								
4 Pa	rent Material:					Unsuitable M	laterials Pr	resent: Ye	s No	x				
lf V	as: Disturk		Fill Mater		Impervious	s Laver(s)		eathered/Fi		Bedrock				
	CO. Diotoric				mpervioue			cullered/11		Dearbork				
5 Gr	oundwater Ob	oserved: Yes												
lf Y	'es: Depth	Weeping from P	i <u>t</u>		Depth	Standing Wa	ater in Hole							
Es	timated Depth	n to High Ground	water:						Observed					
				feet		elevation			eleva	ition				
Te	est Pit Num	ber:			TP-E									
Depth (In.)	Soil Soil Matrix: Horizon/ Color-Mois		Red	oximorphic Fe (mottles)	eatures	Soil Texture (USDA)	Coarse F % by	ragments Volume	Soil Structure	Soil Consistence (Moist)	Other			
			Depth	Color	Percent		Gravel	Cobbles						
								& Stones						
0-4	A	10YR 3/2	-	-	-	VFSL	-	-						
4-14	Bw	10YR 5/2	-	-	-	VFSL	-	-						
14-66	C1	10YR 5/2	-	-	-	VFLS	-	-						
66-116	C2	GLEY 1 2.5/N, GLEY 5/10Y	66	2.5 YR 4/6	100	SiL	-	-			clays, black			

Test Hole Number:			TP-F 25-Oct-19		Time	11:00 AM		60F, sunny			
1. Lo	ocation			Dat	0						
	Ground Eleva	ation at Surface o	of Hole	63.8							
	Location (Ide	entify on Plan)	In propos	ed circle in fro	ont of buildir	na	-				
			<u></u>			No			0.2		
2. Li		(e.g. woodland, agricultural field, v				Surface Sto	nes		Slope (%)		
	Su	mac trees, lower	brush								
	Veç	getation		Landfor	m		Position on	landscape (at	tach sheet)		
3. Di	stances from:	Open Water Bod	ly <u>feet</u>	Draina	age Way	feet	Possible	Wet Area	feet	_	
		Property Line	feet	Drinking \	Water Well	feet	Other				
4 Pa	rent Material:					Unsuitable M	laterials Pr	resent: Ye	s No 🗴		
lf N	es: Disturb	bed Soil	Fill Mater	ial	Impervious	s Layer(s)	We	eathered/Fi	ractured Rock	Bedrock	
5 Gr	oundwater Ob	oserved: Yes	No	X							
lf \	/es: Depth	Weeping from Pi	it		Depth	Standing Wa	ter in Hole	1			
Fs	timated Depth	n to High Groundy	vater:			-			Observed		
				feet		elevation			elevati	on	
Τe	est Pit Numl	ber:			TP-F						
Depth (In.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Rede	oximorphic Fe (mottles)	s) Soil Coar (USDA)		Coarse F % by	ragments Volume	Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
0-6	A	10YR 3/2	-	-	-	FLS	-	-			
6-18	Bw	10YR 5/2	-	-	-	FLS	-	-			
18-60	C1	10YR 5/2	48	10 YR 6/8	-	FLS	-	-			
60-78	C2	GLEY 1 2.5/N, GLEY 5/10Y	66	2.5 YR 4/6	-	SiL	-	-			

Test Hole Number:		-	TP-6 24-Oct-19 Date		Time	10:00 AM		60F, sunny Weather					
1. Lo	cation												
	Ground Elev	ation at Surface c	of Hole	61.4			-						
	Location (Ide	entify on Plan)	Just south	nwest of Cher	ry Hill buildi	ngs							
2. La	and Use: W	oodland	ral field vaca	ant lot etc.)		No Surface Sto	ines						
	mix of brush and trees												
	Ve	getation		Landfor	m		Position on	landscape (at	tach sheet)				
3. Di	stances from	: Open Water Boo	ly feet	Draina	age Way	feet	Possible Wet Area						
		Property Line	feet	Drinking	Water Well	feet	Other						
4 Pa	rent Material:					Unsuitable N	laterials Pr	resent: Ye	s No	x			
lf Y	es: Distur	bed Soil	Fill Materi	al	Impervious	s Layer(s)	W	eathered/F	ractured Rock	Bedrock			
5 Gr	oundwater O	oserved: Yes	No No	Х									
lf Y	es: Deptr	Weeping from P	it		Depth	Standing Wa	ter in Hole)					
Es	timated Dept	h to High Ground	water:	feet		elevation			Observed eleva	tion			
Τe	est Pit Num	ber:			TP-6								
Depth (In.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redo	oximorphic Fe (mottles)	eatures	Soil Texture (USDA)	Coarse F % by	ragments Volume	Soil Structure	Soil Consistence (Moist)	Other		
			Depth	Color	Percent		Gravel	Cobbles & Stones					
0-12	A	10 YR 4/2	-	-	-	FSL	-	-					
12-36	Bw	10YR 4/6	-	-	-	SL	-	-					
36-48	C1	10YR 6/3	-	-	-	G-FS	30	5					
48-120	C2	10YR 6/3	-	-	-	MS	-	-					

Test Hole Number:			7	TP-7 23-Oct-19			10:30 AM		50F, cloudy and rainy		
				Dat	e	Time			Weather		
1. Lo	cation										
	Ground Eleva	ation at Surface c	f Hole	59.6							
	Location (Ide	entify on Plan)	North side	e of entrance	road, farthe	r into site thar	TP-8				
2. La	and Use: Wo	oodland		at late at a)		No		0-3			
	(e.ç	g. woodiand, agricultu	rai neid, vaca	ant lot, etc.)		Surface Sto	nes		Slope (%)		
	mi Veç	x of brush and tre getation	es	Landfor	m		Position on	andscape (at	tach sheet)		
3. Di	stances from:	Open Water Boo	ly feet	Draina	age Way	feet	Possible \	Vet Area	feet	-	
		Property Line	feet	Drinking \	Water Well	feet	Other				
4 Pa	rent Material:					Unsuitable M	laterials Pr	esent: Ye	s No x]	
lf Y	es: Disturb	oed Soil	Fill Materi	al	Impervious	Layer(s)	We	eathered/Fi	ractured Rock	Bedrock	
5 Gr	oundwater Ob	served: Yes	No No	X							
lf Y	es: Depth	Weeping from P	it		Depth	Standing Wa	ter in Hole				
۲e	timated Denth	to High Groundy	vator:		·	0			Observed		
LJ			valor.	feet		elevation			elevati	วท	
Τe	est Pit Numl	ber:			TP-7						
Depth (In.)	Soil Horizon/ Layer	Soil Matrix: Color-Moist (Munsell)	Redo	oximorphic Fe (mottles)	atures	Soil Texture (USDA)	Coarse F % by \	ragments √olume	Soil Structure	Soil Consistence (Moist)	Other
			Depth	Color	Percent		Gravel	Cobbles & Stones			
0-22	НТМ	10YR 3/4	-	-	-	FSL					
2-24	Ab	10YR 4/6	-	-	-	FSL					
24-32	Bw	10YR 6/6	-	-	-	FSL					
32-38	C1	10YR 6/3	-	-	-	G-FS					

Test Hole Number:			7	TP-8	23-Oct-19		9:30 AM		50F, cloudy and rainy		
				Dat	ē	Time			Weather		
1. Lo	cation										
	Ground Eleva	ation at Surface c	of Hole	58.7							
	Location (Ide	ntify on Plan)	North side	e of entrance	road, near p	oost office and	fire statio	n			
2. Land Use: Woodland					No 0-3						
	(e.g	J. woodland, agricultu	ral field, vaca	ant lot, etc.)		Surface Sto	nes		Slope (%)		
	miz	x of brush and tre	es, knotwe	eed Landfor	m		Position on	landscane (at	tach sheet)		
	• • • • • • • • • • • • • • • • • • •		L .	Ducies							
3. Distances from: Open Water Body				Draina	feet Possible Wet Area						
		Property Line		Drinking \	Water Well		Other				
			feet			feet				-	
4 Pa	rent Material:					Unsuitable M	aterials Pr	esent: Ye	s No x	J	
lf Y	es: Disturb	ed Soil	Fill Materi	al	Impervious	Layer(s)	We	eathered/Fr	actured Rock	Bedrock	
5 Gr	oundwater Ob	served: Yes	No	X							
lf \	es: Depth	Weeping from P	i <u>t</u>		Depth	Standing Wa	ter in Hole				
Es	timated Depth	to High Groundy	vater:						Observed		
-	· · · · · ·	3		feet					elevatio	n	
Τe	est Pit Num	per:			TP-8						
Depth (In.)	Soil Horizon/	Soil Matrix: Color-Moist	Redo	oximorphic Fe (mottles)	atures	Soil Texture	Coarse F % by `	ragments Volume	Soil Structure	Soil Consistence	Other
	Layer	(Munsell)	Death	0.1	During	(USDA)	0	Cobbles		(Moist)	
			Depth	Color	Percent		Gravel	& Stones			
0-6	А	10YR 3/4	-	-	-	FSL	5	-	GRANULAR		
6-21	Bw	10YR 4/6	-	-	-	FSL	-	-	MASSIVE		
21-45	C1	10YR 6/6	-	-	-	G-FS	50	10	MASSIVE		
45-84	C2	10YR 6/3	-	-	-	MS	-	-	MASSIVE		


United States Department of Agriculture

Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Barnstable County, Massachusetts



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
How Soil Surveys Are Made	5
Soil Map	8
Soil Map	9
Legend	10
Map Unit Legend	11
Map Unit Descriptions	11
Barnstable County, Massachusetts	13
245B—Hinckley loamy sand, 3 to 8 percent slopes	13
254B—Merrimac fine sandy loam, 3 to 8 percent slopes	14
259B—Carver loamy coarse sand, 3 to 8 percent slopes	16
430B—Barnstable sandy loam, 3 to 8 percent slopes	18
431C—Barnstable sandy loam, 8 to 15 percent slopes, very stony	19
435B—Plymouth loamy coarse sand, 3 to 8 percent slopes	20
435C—Plymouth loamy coarse sand, 8 to 15 percent slopes	22
435D—Plymouth loamy coarse sand, 15 to 35 percent slopes	23
References	25

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



	MAP LE	EGEND		MAP INFORMATION
Area of Interest	(AOI)	300	Spoil Area	The soil surveys that comprise your AOI were mapped at
Area	a of Interest (AOI)	۵	Stony Spot	1.23,000.
Soils	Man Linit Polygons	0	Very Stony Spot	Warning: Soil Map may not be valid at this scale.
Soil	Map Unit Linco	\$	Wet Spot	
Soli	Map Unit Drints	\triangle	Other	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil
		-	Special Line Features	line placement. The maps do not show the small areas of
Special Point Blow	vout	Water Feat	tures	contrasting soils that could have been shown at a more detailed scale.
🖾 Borr	row Pit	\sim	Streams and Canals	
	v Spot	Transporta	ation	Please rely on the bar scale on each map sheet for map
		+++	Rails	measurements.
		~	Interstate Highways	Source of Map: Natural Resources Conservation Service
Gia		~	US Routes	Web Soil Survey URL:
Gra	velly Spot	\sim	Major Roads	Coordinate System. Web Mercator (LF 30.3037)
🙄 Lan	dfill	\approx	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator
A Lava	a Flow	Backgrour	nd	projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the
🚲 Mar	sh or swamp	March 1	Aerial Photography	Albers equal-area conic projection, should be used if more
🙊 Mine	e or Quarry			accurate calculations of distance of area are required.
Miso	cellaneous Water			This product is generated from the USDA-NRCS certified data as
O Pere	ennial Water			of the version date(s) listed below.
v Roc	k Outcrop			Soil Survey Area: Barnstable County, Massachusetts
🕂 Sali	ne Spot			Survey Area Data: Version 17, Jun 9, 2020
san San	idy Spot			Soil map units are labeled (as space allows) for map scales
🕳 Sev	erely Eroded Spot			1:50,000 or larger.
👌 Sink	khole			Date(s) aerial images were photographed:
🔈 Slid	e or Slip			17, 2018
<i>∰</i> Sod	lic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
245B	Hinckley loamy sand, 3 to 8 percent slopes	1.8	3.5%
254B	Merrimac fine sandy loam, 3 to 8 percent slopes	5.0	9.9%
259B	Carver loamy coarse sand, 3 to 8 percent slopes	14.8	29.4%
430B	Barnstable sandy loam, 3 to 8 percent slopes	4.6	9.2%
431C	Barnstable sandy loam, 8 to 15 percent slopes, very stony	14.7	29.1%
435B	Plymouth loamy coarse sand, 3 to 8 percent slopes	0.0	0.0%
435C	Plymouth loamy coarse sand, 8 to 15 percent slopes	6.4	12.7%
435D	Plymouth loamy coarse sand, 15 to 35 percent slopes	3.2	6.3%
Totals for Area of Interest	·	50.4	100.0%

Map Unit Legend

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas

are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Barnstable County, Massachusetts

245B—Hinckley loamy sand, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2svm8 Elevation: 0 to 1,430 feet Mean annual precipitation: 36 to 53 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 250 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Hinckley and similar soils: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Hinckley

Setting

Landform: Outwash terraces, eskers, moraines, outwash plains, kames, outwash deltas, kame terraces

Landform position (two-dimensional): Summit, shoulder, backslope, footslope

Landform position (three-dimensional): Nose slope, side slope, base slope, crest, riser, tread

Down-slope shape: Linear, convex, concave

Across-slope shape: Convex, linear, concave

Parent material: Sandy and gravelly glaciofluvial deposits derived from gneiss and/or granite and/or schist

Typical profile

Oe - 0 to 1 inches: moderately decomposed plant material

A - 1 to 8 inches: loamy sand

Bw1 - 8 to 11 inches: gravelly loamy sand

Bw2 - 11 to 16 inches: gravelly loamy sand

BC - 16 to 19 inches: very gravelly loamy sand

C - 19 to 65 inches: very gravelly sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water capacity: Very low (about 3.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A *Ecological site:* F144AY022MA - Dry Outwash *Hydric soil rating:* No

Minor Components

Windsor

Percent of map unit: 8 percent

Landform: Kame terraces, outwash terraces, eskers, moraines, outwash plains, kames, outwash deltas

Landform position (two-dimensional): Summit, shoulder, backslope, footslope Landform position (three-dimensional): Nose slope, side slope, base slope, crest, tread, riser

Down-slope shape: Linear, convex, concave

Across-slope shape: Convex, linear, concave

Hydric soil rating: No

Sudbury

Percent of map unit: 5 percent
 Landform: Outwash terraces, moraines, outwash plains, outwash deltas, kame terraces
 Landform position (two-dimensional): Backslope, footslope
 Landform position (three-dimensional): Side slope, base slope, head slope, tread
 Down-slope shape: Concave, linear
 Across-slope shape: Linear, concave

Hydric soil rating: No

Agawam

Percent of map unit: 2 percent

Landform: Eskers, moraines, outwash plains, kames, outwash deltas, kame terraces, outwash terraces

Landform position (two-dimensional): Summit, shoulder, backslope, footslope Landform position (three-dimensional): Nose slope, side slope, base slope, crest, tread. riser

Down-slope shape: Linear, convex, concave *Across-slope shape:* Convex, linear, concave *Hydric soil rating:* No

254B—Merrimac fine sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2tyqs Elevation: 0 to 1,290 feet Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: All areas are prime farmland

Map Unit Composition

Merrimac and similar soils: 85 percent *Minor components:* 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Merrimac

Setting

Landform: Outwash terraces, eskers, moraines, outwash plains, kames Landform position (two-dimensional): Backslope, footslope, summit, shoulder Landform position (three-dimensional): Side slope, crest, tread, riser Down-slope shape: Convex Across-slope shape: Convex

Parent material: Loamy glaciofluvial deposits derived from granite, schist, and gneiss over sandy and gravelly glaciofluvial deposits derived from granite, schist, and gneiss

Typical profile

Ap - 0 to 10 inches: fine sandy loam Bw1 - 10 to 22 inches: fine sandy loam Bw2 - 22 to 26 inches: stratified gravel to gravelly loamy sand 2C - 26 to 65 inches: stratified gravel to very gravelly sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 99.90 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 2 percent
Maximum salinity: Nonsaline (0.0 to 1.4 mmhos/cm)
Sodium adsorption ratio, maximum: 1.0
Available water capacity: Low (about 4.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2s Hydrologic Soil Group: A Ecological site: F145XY008MA - Dry Outwash Hydric soil rating: No

Minor Components

Sudbury

Percent of map unit: 5 percent Landform: Terraces, deltas, outwash plains Landform position (two-dimensional): Footslope Landform position (three-dimensional): Tread, dip Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent Landform: Kames, eskers, deltas, outwash plains Landform position (two-dimensional): Summit, shoulder, backslope Landform position (three-dimensional): Head slope, crest, side slope, nose slope, rise Down-slope shape: Convex Across-slope shape: Convex, linear Hydric soil rating: No

Windsor

Percent of map unit: 3 percent Landform: Outwash plains, outwash terraces, deltas, dunes Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Tread, riser Down-slope shape: Linear, convex Across-slope shape: Linear, convex Hydric soil rating: No

Agawam

Percent of map unit: 2 percent Landform: Stream terraces, outwash plains, kames, outwash terraces, eskers, moraines Landform position (three-dimensional): Rise Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

259B—Carver loamy coarse sand, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2y07t Elevation: 0 to 240 feet Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Carver, loamy coarse sand, and similar soils: 80 percent *Minor components:* 20 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Carver, Loamy Coarse Sand

Setting

Landform: Moraines, outwash plains Landform position (two-dimensional): Summit, shoulder, backslope, footslope, toeslope Landform position (three-dimensional): Nose slope, side slope, crest, head slope, tread Down-slope shape: Convex, linear Across-slope shape: Linear Parent material: Sandy glaciofluvial deposits

Typical profile

Oi - 0 to 2 inches: slightly decomposed plant material *Oe - 2 to 3 inches:* moderately decomposed plant material *A - 3 to 7 inches:* loamy coarse sand *E - 7 to 10 inches:* coarse sand *Bw1 - 10 to 15 inches:* coarse sand *Bw2 - 15 to 28 inches:* coarse sand *BC - 28 to 32 inches:* coarse sand *C - 32 to 67 inches:* coarse sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Maximum salinity: Nonsaline (0.0 to 1.9 mmhos/cm)
Available water capacity: Low (about 4.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Ecological site: F149BY005MA - Dry Outwash Hydric soil rating: No

Minor Components

Deerfield

Percent of map unit: 10 percent Landform: Outwash terraces, outwash plains, outwash deltas, kame terraces Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: No

Hinckley

Percent of map unit: 5 percent

Landform: Outwash plains, kames, outwash deltas, kame terraces, outwash terraces, eskers, moraines

Landform position (two-dimensional): Shoulder, backslope, footslope, summit, toeslope

Landform position (three-dimensional): Crest, head slope, nose slope, side slope, riser, tread

Down-slope shape: Convex

Across-slope shape: Convex

Hydric soil rating: No

Merrimac

Percent of map unit: 3 percent Landform: Outwash deltas, kame terraces, outwash terraces Landform position (three-dimensional): Tread, riser *Down-slope shape:* Linear *Across-slope shape:* Linear *Hydric soil rating:* No

Mashpee

Percent of map unit: 2 percent Landform: Drainageways, terraces, depressions Landform position (three-dimensional): Tread Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

430B—Barnstable sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 98ps Elevation: 0 to 1,000 feet Mean annual precipitation: 40 to 50 inches Mean annual air temperature: 48 to 54 degrees F Frost-free period: 160 to 240 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Barnstable and similar soils: 75 percent *Minor components:* 25 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Barnstable

Setting

Landform: Ground moraines Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Side slope Down-slope shape: Convex Across-slope shape: Convex Parent material: Friable loamy ablation till over reworked sandy glaciofluvial deposits

Typical profile

H1 - 0 to 1 inches: sandy loam H2 - 1 to 23 inches: sandy loam H3 - 23 to 64 inches: coarse sand

Properties and qualities

Slope: 3 to 8 percent Depth to restrictive feature: More than 80 inches Drainage class: Well drained Runoff class: Low Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr) Depth to water table: More than 80 inches *Frequency of flooding:* None *Frequency of ponding:* None *Available water capacity:* Low (about 4.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2s Hydrologic Soil Group: A Ecological site: F149BY011MA - Well Drained Till Uplands Hydric soil rating: No

Minor Components

Plymouth

Percent of map unit: 8 percent Hydric soil rating: No

Nantucket

Percent of map unit: 7 percent Hydric soil rating: No

Merrimac

Percent of map unit: 5 percent Hydric soil rating: No

Carver

Percent of map unit: 5 percent Hydric soil rating: No

431C—Barnstable sandy loam, 8 to 15 percent slopes, very stony

Map Unit Setting

National map unit symbol: 98pw Elevation: 0 to 1,000 feet Mean annual precipitation: 40 to 50 inches Mean annual air temperature: 48 to 54 degrees F Frost-free period: 160 to 240 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Barnstable and similar soils: 70 percent *Minor components:* 30 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Barnstable

Setting

Landform: Ground moraines Landform position (two-dimensional): Backslope Landform position (three-dimensional): Side slope Down-slope shape: Linear Across-slope shape: Convex *Parent material:* Friable loamy ablation till over reworked sandy glaciofluvial deposits; loamy ablation till over reworked sandy outwash

Typical profile

H1 - 0 to 1 inches: sandy loam H2 - 1 to 23 inches: sandy loam

H3 - 23 to 64 inches: coarse sand

Properties and qualities

Slope: 8 to 15 percent
Surface area covered with cobbles, stones or boulders: 2.0 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 4.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: A Ecological site: F149BY011MA - Well Drained Till Uplands Hydric soil rating: No

Minor Components

Nantucket

Percent of map unit: 10 percent Hydric soil rating: No

Plymouth

Percent of map unit: 10 percent *Hydric soil rating:* No

Carver

Percent of map unit: 10 percent *Hydric soil rating:* No

435B—Plymouth loamy coarse sand, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 98rs Elevation: 0 to 1,000 feet Mean annual precipitation: 35 to 50 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Plymouth and similar soils: 70 percent *Minor components:* 30 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Plymouth

Setting

Landform: Outwash plains Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Riser Down-slope shape: Convex Across-slope shape: Convex Parent material: Loose sandy glaciofluvial deposits and/or loose sandy ablation till; loose sandy ablation till and/or loose sandy glaciofluvial deposits; loose sandy ablation till and/or loose sandy glaciofluvial deposits

Typical profile

H1 - 0 to 3 inches: loamy coarse sand H2 - 3 to 29 inches: gravelly loamy coarse sand H3 - 29 to 64 inches: gravelly coarse sand

Properties and qualities

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): High to very high (6.00 to 20.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Very low (about 2.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A Ecological site: F149BY005MA - Dry Outwash Hydric soil rating: No

Minor Components

Hinckley

Percent of map unit: 8 percent Hydric soil rating: No

Carver

Percent of map unit: 8 percent Hydric soil rating: No

Barnstable

Percent of map unit: 6 percent Hydric soil rating: No

Nantucket

Percent of map unit: 6 percent

Hydric soil rating: No

Merrimac

Percent of map unit: 2 percent Hydric soil rating: No

435C—Plymouth loamy coarse sand, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 98rt Elevation: 0 to 1,000 feet Mean annual precipitation: 35 to 50 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Plymouth and similar soils: 65 percent *Minor components:* 35 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Plymouth

Setting

Landform: Ice-contact slopes Landform position (two-dimensional): Backslope Landform position (three-dimensional): Riser Down-slope shape: Linear Across-slope shape: Convex Parent material: Loose sandy glaciofluvial deposits and/or loose sandy ablation till; loose sandy ablation till and/or loose sandy glaciofluvial deposits

Typical profile

H1 - 0 to 3 inches: loamy coarse sand H2 - 3 to 29 inches: gravelly loamy coarse sand H3 - 29 to 64 inches: gravelly coarse sand

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): High to very high (6.00 to 20.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Very low (about 2.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4s Hydrologic Soil Group: A Ecological site: F149BY005MA - Dry Outwash Hydric soil rating: No

Minor Components

Carver

Percent of map unit: 15 percent *Hydric soil rating:* No

Hinckley

Percent of map unit: 8 percent Hydric soil rating: No

Barnstable

Percent of map unit: 6 percent Hydric soil rating: No

Nantucket

Percent of map unit: 6 percent Hydric soil rating: No

435D—Plymouth loamy coarse sand, 15 to 35 percent slopes

Map Unit Setting

National map unit symbol: 98rv Elevation: 0 to 1,000 feet Mean annual precipitation: 35 to 50 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 240 days Farmland classification: Not prime farmland

Map Unit Composition

Plymouth and similar soils: 65 percent *Minor components:* 35 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Plymouth

Setting

Landform: Ice-contact slopes Landform position (two-dimensional): Backslope Landform position (three-dimensional): Riser Down-slope shape: Linear Across-slope shape: Convex Parent material: Loose sandy glaciofluvial deposits and/or loose sandy ablation till; loose sandy glaciofluvial deposits and/or loose sandy ablation till

Typical profile

H1 - 0 to 3 inches: loamy coarse sand *H2 - 3 to 29 inches:* gravelly loamy coarse sand H3 - 29 to 64 inches: gravelly coarse sand

Properties and qualities

Slope: 15 to 35 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Excessively drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): High to very high (6.00 to 20.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Very low (about 2.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: A Ecological site: F149BY005MA - Dry Outwash Hydric soil rating: No

Minor Components

Carver

Percent of map unit: 15 percent Hydric soil rating: No

Hinckley

Percent of map unit: 10 percent *Hydric soil rating:* No

Barnstable

Percent of map unit: 5 percent Hydric soil rating: No

Nantucket

Percent of map unit: 5 percent Hydric soil rating: No

References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/national/soils/?cid=nrcs142p2_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/ home/?cid=nrcs142p2 053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/ detail/national/landuse/rangepasture/?cid=stelprdb1043084

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/? cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

APPENDIX B

Drainage Area Maps



	Revisions	Zhecked By: BRK Rav, Date By Appr. Description
	Horsley Witten Group, Inc. Sustainable Environmental Solutions 90 Route 6A Sandwich, MA 02663 508-833-3150 fax 508-833-3150 fax	Date: Design By: Drawn By: GLK
	BILLE FOR AFFORDABLE HOUSING CAPE VIEW WAY BOURNE, MASSACHUSETTS	PRE-CONSTRUCTION DRAINAGE MAP
	Propared For: POAH, INC. 2 OLIVEN STREET, SUITE 500 BOSTON, MA 02109 Phone (617) 261-9698 Fax:	
	Prepared B/: Horsky Writen Group, Inc. Poroute 6A Porovet (50) 333-6800 Dated: September 2019	
ES431CBARNSTABLE VERY STONY SANDY LOAM 8-15% SLOPES (HSG A)435CPLYMOUTH LOAMY COARSE SAND 8-15% SLOPES (HSG A)	Project Number: 19038 Sheet Number: 1 of 2	



Very and the second		Revisions	Rev. Date By Apr. Description
ES Marketing to the		, Inc.	wn By: Checked By: GLK BRRK
Provide the second state of the sec		Horsley Witten Group Sustainable Environmental So Sucus 6A Sandwich, MA 02563 508-833-6600 voice 508-833-3150 fax	Date: Design By: Dra 12/10/2021
PES All Phone def All Phone def A		BILLE FOR AFFORDABLE HOUSING CAPE VIEW WAY BOURNE, MASSACHUSETTS	PRATTIRE POST-CONSTRUCTION DRAINAGE MAP
Personal and the second		Propared For: POAH, INC. 2 OLIVER STREET, SUITE 500 BOSTON, MA 02109 Phone (617) 261-3698 Fax:	
431C BARNSTABLE VERY STONY SANDY LOAM 8-15% SLOPES (HSG A) Registration: 435C PLYMOUTH LOAMY COARSE SAND 8-15% SLOPES (HSG A) Project Number: 19038 Sheet Number: 2 of 2		Prepared By: Horsley Witten Group, Inc. 90 Route 8A Phone(508) 833-6600 Dated: September 2019	
8-15% SLOPES (HSG A) Project Number: 19038 Sheet Number: 2 of 2	431C BARNSTABLE VERY STONY SANDY LOAM 8-15% SLOPES (HSG A) PLYMOUTH LOAMY COARSE SAND	Registration:	
•	8-15% SLOPES (HSG A)	Project Number: 19038 Sheet Number: 2 of 2	

APPENDIX C

GSI Sizing Calculations



Project:	Cape View Way	Project No:	19038
Project Location:	Bourne, MA		
Calculated By:	GLK	Date :	12/7/2021
Checked By:	BRK	Date :	

Instructions: Enter values in cells only. All other cells are formulas or links and do not need to be edited. See cell comments for descriptions and formulas used.

Water Quality Volume (WQv)

Based upon 1-inch of rainfall times the contributing impervious area contributing impervious area

WQv (cf) = (1" rainfall/12) * Imp. Area (sf)

1 Inch Storm Type:

							WQv	WQv
		% Imp.	Draina	ge Area	Imp. A	rea	Required*	required
DA	Description	%	sf	ac	sf	ac	cf	af
3a	Upper parkin	40%	48,497	1.11	19,558	0.45	1,630	0.037
2b	Driveway loo	68%	14,564	0.33	9,894	0.23	825	0.019
	TOTALS		63061	1.11	29,452	0.45	2,454	0.056

Bioretention Sizing Calculations

Sizing Equations: Bioretention

Required Surface Area (sf) = (WQv) (df) / [(k) (hf + df) (tf)]

Where: df = Filter bed depth (ft) k = Coefficient of permeability of filter media (ft/day)

hf = Ave. height of water above filter bed (ft) *tf* = Design filter bed drain time (days)

BIORETENTION SIZING:

		WO			hmax- Height of	hf=avg		Surface	Surface	Sediment Forebay	0		Sediment	WQV
Bio Area	Drainage Area Name	WQv Required (af)	df (ft)	K (ft/day)	filter (in.)	of above (ft)	tf (days)	Area Required (sf)	Area Provided (sf)	Required 10% WQv (cf)	Forebay Depth (ft)	Forebay Area (sf)	Forebay Provided (cf)	Provided (af)
1	3a	0.037	2.00	1	9	0.375	1.67	822	819	163	0.50	108	132	0.037
2	2b	0.019	2.00	1	6	0.25	1.67	439	408	82	0.50	54	105	0.018
	TOTALS	0.056		Per	centage of T	reatment	t Provided	1261	1227 97%	245			237 97%	0.055

ENHANCED TREE TRENCH VOLUME CALCULATIONS

Project:	Cape View Way		Project No: 19038		
Project Location:	Bourne, MA				
			_		
Calculated By:	GK		Date :	1/27/2021	
Checked By:	BRK		Date :	3/5/2021	
Revised By:	GLK		Date :	12/10/2021	
<u>Storm Depth:</u>	1	Inch			

Instructions: Enter values in cells only. All other cells are formulas or links and do not need to be edited. See cell comments for descriptions and formulas used.

		PARAMETER			DESIGN						CALCULATION				
Tree Trench	# Location	Drainage Area (sf)	IC (sf)	WQv (cf)	Length (feet) "X"	Width (feet) "Y"	Area (sf)	Media Depth (feet)	% Voids (1.00)	Media Storage Volume (cf)	Soil Type	Infiltration Rate (in/hr)	WQv Storage Volume Provided (%)	WQv Volume Treated (includes infiltration) ¹	% WQV
											A -Sandy				
1	DA3b	17,134	10,502	875.17	103	6.2	638.6	2.17	0.33	457	Loam	8.27	52.3%	871.0	100%
											A -Sandy				
2	DA3d	4,883	4,686	390.50	30	5	150	2.50	0.33	124	Loam	8.27	31.7%	389.0	100%

¹ From HydroCAD

APPENDIX D

HydroCAD Modeling



Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	2YR	Type III 24-hr		Default	24.00	1	3.33	2
2	10YR	Type III 24-hr		Default	24.00	1	4.90	2
3	25YR	Type III 24-hr		Default	24.00	1	6.12	2
4	100YR	Type III 24-hr		Default	24.00	1	8.56	2
5	WQv	Type III 24-hr		Default	24.00	1	1.21	2

Rainfall Events Listing (selected events)

Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.739	39	>75% Grass cover, Good, HSG A (DA1, DA2, DA3)
0.039	96	Gravel surface, HSG A (DA3)
0.128	98	Paved parking, HSG A (DA1, DA3)
0.221	98	Roofs, HSG A (DA1)
6.830	30	Woods, Good, HSG A (DA1, DA2, DA3)
7.958	34	TOTAL AREA
Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
7.958	HSG A	DA1, DA2, DA3
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.000	Other	
7.958		TOTAL AREA

19038-PRE V2

Prepared by {enter	your company name here}
HydroCAD® 10.10-6a	s/n 01445 © 2020 HydroCAD Software Solutions LLC

HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
 0.739	0.000	0.000	0.000	0.000	0.739	>75% Grass cover, Good	DA1,
							DA2, DA3
0.039	0.000	0.000	0.000	0.000	0.039	Gravel surface	DA3
0.128	0.000	0.000	0.000	0.000	0.128	Paved parking	DA1, DA3
0.221	0.000	0.000	0.000	0.000	0.221	Roofs	DA1
6.830	0.000	0.000	0.000	0.000	6.830	Woods, Good	DA1,
							DA2, DA3
7.958	0.000	0.000	0.000	0.000	7.958	TOTAL AREA	

Ground Covers (all nodes)

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.00" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.00 cfs 0.000 af
Subcatchment DA2: DA2	Runoff Area=79,824 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=505' Tc=21.5 min CN=31 Runoff=0.00 cfs 0.000 af
Subcatchment DA3: DA3	Runoff Area=108,190 sf 0.23% Impervious Runoff Depth=0.00" Flow Length=537' Tc=26.5 min CN=31 Runoff=0.00 cfs 0.000 af
Pond SP1: SP1	Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af
Pond SP2: SP2	Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af
Pond SP3: SP3	Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af

Total Runoff Area = 7.958 acRunoff Volume = 0.000 afAverage Runoff Depth = 0.00"95.61% Pervious = 7.609 ac4.39% Impervious = 0.350 ac

Summary for Subcatchment DA1: DA1

Runoff = 0.00 cfs @ 24.04 hrs, Volume= 0.000 af, Depth= 0.00" Routed to Pond SP1 : SP1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

A	rea (sf)	CN E	Description		
1	18,297	30 V	Voods, Go	od, HSG A	
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A
	9,642	98 F	Roofs, HSG	βA	
	5,338	98 F	Paved park	ing, HSG A	
1	58,640	38 V	Veighted A	verage	
1	43,660	g	0.56% Per	vious Area	
	14,980	g	.44% Impe	ervious Area	a
			-		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.6	50	0.1100	0.08		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C
					Woodland Kv= 5.0 fps
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D
					Short Grass Pasture Kv= 7.0 fps
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1
					Woodland Kv= 5.0 fps
14.5	417	Total			

Subcatchment DA1: DA1



Summary for Subcatchment DA2: DA2

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP2 : SP2 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

	A	rea (sf)	CN D	Description		
		73,961	30 V	Voods, Go	od, HSG A	
		5,863	39 >	75% Gras	s cover, Go	bod, HSG A
		79,824	31 V	Veighted A	verage	
		79,824	1	00.00% Pe	ervious Are	а
	_					
	Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	13.6	50	0.0600	0.06		Sheet Flow, A TO B
						Woods: Dense underbrush n= 0.800 P2= 3.33"
	2.5	164	0.0490	1.11		Shallow Concentrated Flow, B to C
						Woodland Kv= 5.0 fps
	0.8	73	0.0480	1.53		Shallow Concentrated Flow, C to D
						Short Grass Pasture Kv= 7.0 fps
	4.6	218	0.0250	0.79		Shallow Concentrated Flow, D to SP2
						Woodland Kv= 5.0 fps
	21.5	505	Total			

Subcatchment DA2: DA2



Summary for Subcatchment DA3: DA3

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP3 : SP3 0.000 af, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

A	rea (sf)	CN [Description				
1	05,276	30 \	30 Woods, Good, HSG A				
	964	39 >	>75% Gras	s cover, Go	ood, HSG A		
	245	98 F	Paved park	ing, HSG A	N N N N N N N N N N N N N N N N N N N		
	1,705	96 (Gravel surfa	ace, HSG A	۱ <u> </u>		
1	08,190	31 \	Veighted A	verage			
1	07,945	ç	9.77% Per	vious Area			
	245	().23% Impe	ervious Are	а		
Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
17.9	50	0.0300	0.05		Sheet Flow, A TO B		
					Woods: Dense underbrush n= 0.800 P2= 3.33"		
8.0	362	0.0230	0.76		Shallow Concentrated Flow, B to C		
					Woodland Kv= 5.0 fps		
0.6	125	0.0340	3.74		Shallow Concentrated Flow, C to SP2		
					Paved Kv= 20.3 fps		
26.5	537	Total					

Subcatchment DA3: DA3



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Ar	ea =	3.642 ac,	9.44% Impervious,	Inflow Depth = 0.0	00" for 2YR event
Inflow	=	0.00 cfs @	24.04 hrs, Volume	= 0.000 af	
Primary	=	0.00 cfs @	24.04 hrs, Volume	= 0.000 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs





Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	\rea =	1.833 ac,	0.00% Impervious,	Inflow Depth = 0.0	00" for 2YR event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0.000 af	
Primary	/ =	0.00 cfs @	0.00 hrs, Volume=	: 0.000 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area =	=	2.484 ac,	0.23% Impervious,	Inflow Depth = 0.0	00" for 2YR event
Inflow	=	=	0.00 cfs @	0.00 hrs, Volume	= 0.000 af	
Primary	y =	=	0.00 cfs @	0.00 hrs, Volume	= 0.000 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP3: SP3

19038-PRE V2	T
Prepared by {enter your company name here}	
HvdroCAD® 10.10-6a s/n 01445 © 2020 HvdroCAD Software Solutions	LLC

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.15" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.07 cfs 0.045 af
Subcatchment DA2: DA2	Runoff Area=79,824 sf 0.00% Impervious Runoff Depth=0.01" Flow Length=505' Tc=21.5 min CN=31 Runoff=0.00 cfs 0.001 af
Subcatchment DA3: DA3	Runoff Area=108,190 sf 0.23% Impervious Runoff Depth=0.01" Flow Length=537' Tc=26.5 min CN=31 Runoff=0.00 cfs 0.002 af
Pond SP1: SP1	Inflow=0.07 cfs 0.045 af Primary=0.07 cfs 0.045 af
Pond SP2: SP2	Inflow=0.00 cfs 0.001 af Primary=0.00 cfs 0.001 af
Pond SP3: SP3	Inflow=0.00 cfs 0.002 af Primary=0.00 cfs 0.002 af

Total Runoff Area = 7.958 ac	Runoff Volume = 0.048 af	Average Runoff Depth = 0.07"
94	5.61% Pervious = 7.609 ac	4.39% Impervious = 0.350 ac

Summary for Subcatchment DA1: DA1

Runoff = 0.07 cfs @ 13.90 hrs, Volume= 0.045 af, Depth= 0.15" Routed to Pond SP1 : SP1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

ΑΑ	rea (sf)	CN E	Description		
1	18,297	30 V	Voods, Go	od, HSG A	
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A
	9,642	98 F	Roofs, HSG	βA	
	5,338	98 F	aved park	ing, HSG A	
1	58,640	38 V	Veighted A	verage	
1	43,660	g	0.56% Per	vious Area	
	14,980	g	.44% Impe	ervious Area	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.6	50	0.1100	0.08		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C
					Woodland Kv= 5.0 fps
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D
					Short Grass Pasture Kv= 7.0 fps
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1
					Woodland Kv= 5.0 fps
14.5	417	Total			



Subcatchment DA1: DA1

Summary for Subcatchment DA2: DA2

Runoff = 0.00 cfs @ 23.29 hrs, Volume= Routed to Pond SP2 : SP2 0.001 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

A	rea (sf)	<u>CN</u> D	escription		
	73,961	30 V	Voods, Go	od, HSG A	
	5,863	39 >	75% Gras	s cover, Go	ood, HSG A
	79,824	31 V	Veighted A	verage	
	79,824	1	00.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
13.6	50	0.0600	0.06		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
2.5	164	0.0490	1.11		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
0.8	73	0.0480	1.53		Shallow Concentrated Flow, C to D
					Short Grass Pasture Kv= 7.0 fps
4.6	218	0.0250	0.79		Shallow Concentrated Flow, D to SP2
					Woodland Kv= 5.0 fps

21.5 505 Total

Subcatchment DA2: DA2



Summary for Subcatchment DA3: DA3

Runoff = 0.00 cfs @ 23.35 hrs, Volume= 0.002 Routed to Pond SP3 : SP3

0.002 af, Depth= 0.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

A	rea (sf)		Description					
1	05,276	30 \	Noods, Go	od, HSG A				
	964	39 >	>75% Gras	s cover, Go	ood, HSG A			
	245	98 I	Paved park	ing, HSG A	N N N N N N N N N N N N N N N N N N N			
	1,705	96 (6 Gravel surface, HSG A					
1	08,190	31 \	Neighted A	verage				
1	07,945	ć	99.77% Pei	vious Area				
	245	().23% Impe	ervious Are	а			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
17.9	50	0.0300	0.05		Sheet Flow, A TO B			
					Woods: Dense underbrush n= 0.800 P2= 3.33"			
8.0	362	0.0230	0.76		Shallow Concentrated Flow, B to C			
					Woodland Kv= 5.0 fps			
0.6	125	0.0340	3.74		Shallow Concentrated Flow, C to SP2			
					Paved Kv= 20.3 fps			
26.5	537	Total						

Subcatchment DA3: DA3



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Ar	rea =	3.642 ac,	9.44% Impervious,	Inflow Depth = 0.7	15" for 10YR event
Inflow	=	0.07 cfs @	13.90 hrs, Volume	= 0.045 af	
Primary	=	0.07 cfs @	13.90 hrs, Volume	= 0.045 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow Ar	rea =	1.833 ac,	0.00% Impervious,	Inflow Depth = 0.0	01" for 10YR event
Inflow	=	0.00 cfs @	23.29 hrs, Volume	= 0.001 af	
Primary	=	0.00 cfs @	23.29 hrs, Volume	= 0.001 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs





Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	ea =	2.484 ac,	0.23% Impervious,	Inflow Depth = 0.0	01" for 10YR event
Inflow	=	0.00 cfs @	23.35 hrs, Volume	= 0.002 af	
Primary	=	0.00 cfs @	23.35 hrs, Volume	= 0.002 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP3: SP3

19038-PRE V2	Ty
Prepared by {enter your company name here}	
HvdroCAD® 10.10-6a s/n 01445 © 2020 HvdroCAD Software Solutions I	LLC

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.43" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.53 cfs 0.129 af
Subcatchment DA2: DA2	Runoff Area=79,824 sf 0.00% Impervious Runoff Depth=0.12" Flow Length=505' Tc=21.5 min CN=31 Runoff=0.03 cfs 0.018 af
Subcatchment DA3: DA3	Runoff Area=108,190 sf 0.23% Impervious Runoff Depth=0.12" Flow Length=537' Tc=26.5 min CN=31 Runoff=0.04 cfs 0.024 af
Pond SP1: SP1	Inflow=0.53 cfs 0.129 af Primary=0.53 cfs 0.129 af
Pond SP2: SP2	Inflow=0.03 cfs 0.018 af Primary=0.03 cfs 0.018 af
Pond SP3: SP3	Inflow=0.04 cfs 0.024 af Primary=0.04 cfs 0.024 af

Total Runoff Area = 7.958 acRunoff Volume = 0.171 afAverage Runoff Depth = 0.26"95.61% Pervious = 7.609 ac4.39% Impervious = 0.350 ac

Summary for Subcatchment DA1: DA1

Runoff = 0.53 cfs @ 12.49 hrs, Volume= 0.129 af, Depth= 0.43" Routed to Pond SP1 : SP1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

ΑΑ	rea (sf)	CN E	Description		
1	18,297	30 V	Voods, Go	od, HSG A	
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A
	9,642	98 F	Roofs, HSG	βA	
	5,338	98 F	aved park	ing, HSG A	
1	58,640	38 V	Veighted A	verage	
1	43,660	g	0.56% Per	vious Area	
	14,980	g	.44% Impe	ervious Area	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.6	50	0.1100	0.08		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C
					Woodland Kv= 5.0 fps
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D
					Short Grass Pasture Kv= 7.0 fps
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1
					Woodland Kv= 5.0 fps
14.5	417	Total			

Subcatchment DA1: DA1



Summary for Subcatchment DA2: DA2

Runoff = 0.03 cfs @ 15.22 hrs, Volume= Routed to Pond SP2 : SP2

0.018 af, Depth= 0.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

_	A	rea (sf)	CN D	Description		
		73,961	30 V	Voods, Go	od, HSG A	
_		5,863	39 >	75% Gras	s cover, Go	bod, HSG A
		79,824	31 V	Veighted A	verage	
		79,824	1	00.00% Pe	ervious Are	а
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	13.6	50	0.0600	0.06		Sheet Flow, A TO B
						Woods: Dense underbrush n= 0.800 P2= 3.33"
	2.5	164	0.0490	1.11		Shallow Concentrated Flow, B to C
						Woodland Kv= 5.0 fps
	0.8	73	0.0480	1.53		Shallow Concentrated Flow, C to D
						Short Grass Pasture Kv= 7.0 fps
	4.6	218	0.0250	0.79		Shallow Concentrated Flow, D to SP2
						Woodland Kv= 5.0 fps

21.5 505 Total

Subcatchment DA2: DA2



Summary for Subcatchment DA3: DA3

Runoff = 0.04 cfs @ 15.34 hrs, Volume= 0.024 af, Depth= 0.12" Routed to Pond SP3 : SP3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

A	rea (sf)	CN I	Description		
1	05,276	30	Noods, Go	od, HSG A	
	964	39 :	>75% Gras	s cover, Go	bod, HSG A
	245	98	Paved park	ing, HSG A	N Contraction of the second
	1,705	96	Gravel surfa	ace, HSG A	A
1	08,190	31	Neighted A	verage	
1	07,945	ę	99.77% Pei	vious Area	
	245	().23% Impe	ervious Are	a
_				_	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
17.9	50	0.0300	0.05		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
8.0	362	0.0230	0.76		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
0.6	125	0.0340	3.74		Shallow Concentrated Flow, C to SP2
					Paved Kv= 20.3 fps
26.5	537	Total			

Subcatchment DA3: DA3



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	3.642 ac,	9.44% Impervious,	Inflow Depth = 0.4	43" for 25YR event
Inflow	=	0.53 cfs @	12.49 hrs, Volume	= 0.129 af	
Primary	=	0.53 cfs @	12.49 hrs, Volume	= 0.129 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs





Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	ea =	1.833 ac,	0.00% Impervious,	Inflow Depth = 0 .	12" for 25YR event
Inflow	=	0.03 cfs @	15.22 hrs, Volume	= 0.018 af	
Primary	=	0.03 cfs @	15.22 hrs, Volume	= 0.018 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	rea =	2.484 ac,	0.23% Impervious,	Inflow Depth = 0.7	12" for 25YR event
Inflow	=	0.04 cfs @	15.34 hrs, Volume	= 0.024 af	
Primary	=	0.04 cfs @	15.34 hrs, Volume	= 0.024 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP3: SP3

19038-PRE V2	Type I
Prepared by {enter your company name here}	
HydroCAD® 10.10-6a s/n 01445 © 2020 HydroCAD Software Solutions	LLC

III 24-hr 100YR Rainfall=8.56" Printed 9/9/2021 Page 32

Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=1.30" Flow Length=417' Tc=14.5 min CN=38 Runoff=2.86 cfs 0.394 af
Subcatchment DA2: DA2	Runoff Area=79,824 sf 0.00% Impervious Runoff Depth=0.64" Flow Length=505' Tc=21.5 min CN=31 Runoff=0.38 cfs 0.098 af
Subcatchment DA3: DA3	Runoff Area=108,190 sf 0.23% Impervious Runoff Depth=0.64" Flow Length=537' Tc=26.5 min CN=31 Runoff=0.48 cfs 0.132 af
Pond SP1: SP1	Inflow=2.86 cfs 0.394 af Primary=2.86 cfs 0.394 af
Pond SP2: SP2	Inflow=0.38 cfs 0.098 af Primary=0.38 cfs 0.098 af
Pond SP3: SP3	Inflow=0.48 cfs 0.132 af Primary=0.48 cfs 0.132 af

Total Runoff Area = 7.958 acRunoff Volume = 0.624 afAverage Runoff Depth = 0.94"95.61% Pervious = 7.609 ac4.39% Impervious = 0.350 ac

Summary for Subcatchment DA1: DA1

Runoff = 2.86 cfs @ 12.26 hrs, Volume= 0.394 af, Depth= 1.30" Routed to Pond SP1 : SP1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	rea (sf)	CN E	Description		
1	18,297	30 V	Voods, Go	od, HSG A	
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A
	9,642	98 F	Roofs, HSG	βA	
	5,338	98 F	aved park	ing, HSG A	
1	58,640	38 V	Veighted A	verage	
1	43,660	g	0.56% Per	vious Area	
	14,980	g	.44% Impe	ervious Area	a
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.6	50	0.1100	0.08		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C
					Woodland Kv= 5.0 fps
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D
					Short Grass Pasture Kv= 7.0 fps
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1
					Woodland Kv= 5.0 fps
14.5	417	Total			

Subcatchment DA1: DA1



Summary for Subcatchment DA2: DA2

Runoff = 0.38 cfs @ 12.58 hrs, Volume= 0.098 Routed to Pond SP2 : SP2

0.098 af, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	rea (sf)	<u>CN</u> D	Description		
	73,961	30 V	Voods, Go	od, HSG A	
	5,863	39 >	75% Gras	s cover, Go	ood, HSG A
	79,824	31 V	Veighted A	verage	
	79,824	1	00.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
13.6	50	0.0600	0.06		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
2.5	164	0.0490	1.11		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
0.8	73	0.0480	1.53		Shallow Concentrated Flow, C to D
					Short Grass Pasture Kv= 7.0 fps
4.6	218	0.0250	0.79		Shallow Concentrated Flow, D to SP2
					Woodland Kv= 5.0 fps

21.5 505 Total

Subcatchment DA2: DA2



Summary for Subcatchment DA3: DA3

Runoff = 0.48 cfs @ 12.64 hrs, Volume= 0.132 af, Depth= 0.64" Routed to Pond SP3 : SP3

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	rea (sf)	CN I	Description		
1	05,276	30	Noods, Go	od, HSG A	
	964	39 :	>75% Gras	s cover, Go	bod, HSG A
	245	98 I	Paved park	ing, HSG A	N Contraction of the second
	1,705	96 (Gravel surfa	ace, HSG A	A
1	08,190	31 \	Neighted A	verage	
1	07,945	ę	99.77% Pei	rvious Area	
	245	().23% Impe	ervious Are	a
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
17.9	50	0.0300	0.05		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
8.0	362	0.0230	0.76		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
0.6	125	0.0340	3.74		Shallow Concentrated Flow, C to SP2
					Paved Kv= 20.3 fps
26.5	537	Total			

Subcatchment DA3: DA3

Hydrograph Runoff 0.48 cfs 0.5 Type III 24-hr 0.45 100YR Rainfall=8.56" 0.4 Runoff Area=108,190 sf 0.35 Runoff Volume=0.132 af (cfs) 0.3 Runoff Depth=0.64" Flow 0.25 Flow Length=537' Tc=26.5 min 0.2 CN=31 0.15 0.1 0.05 0 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 Time (hours)

Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow Are	ea =	3.642 ac,	9.44% Impervious,	Inflow Depth = 1.	30" for 100YR event
Inflow	=	2.86 cfs @	12.26 hrs, Volume	= 0.394 af	
Primary	=	2.86 cfs @	12.26 hrs, Volume	= 0.394 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow Area	a =	1.833 ac,	0.00% Impervious,	Inflow Depth = 0.6	64" for 100YR event
Inflow	=	0.38 cfs @	12.58 hrs, Volume	e 0.098 af	
Primary	=	0.38 cfs @	12.58 hrs, Volume	e= 0.098 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area =	2.484 ac,	0.23% Impervious,	Inflow Depth = 0.6	64" for 100YR event
Inflow	=	0.48 cfs @	12.64 hrs, Volume	= 0.132 af	
Primary	/ =	0.48 cfs @	12.64 hrs, Volume	= 0.132 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP3: SP3
Time span=0.00-36.00 hrs, dt=0.01 hrs, 3601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.00" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.00 cfs 0.000 af
Subcatchment DA2: DA2	Runoff Area=79,824 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=505' Tc=21.5 min CN=31 Runoff=0.00 cfs 0.000 af
Subcatchment DA3: DA3	Runoff Area=108,190 sf 0.23% Impervious Runoff Depth=0.00" Flow Length=537' Tc=26.5 min CN=31 Runoff=0.00 cfs 0.000 af
Pond SP1: SP1	Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af
Pond SP2: SP2	Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af
Pond SP3: SP3	Inflow=0.00 cfs 0.000 af Primary=0.00 cfs 0.000 af

Total Runoff Area = 7.958 ac	Runoff Volume = 0.000 af	Average Runoff Depth = 0.00"
9	5.61% Pervious = 7.609 ac	4.39% Impervious = 0.350 ac

Summary for Subcatchment DA1: DA1

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP1 : SP1 0.000 af, Depth= 0.00"

A	rea (sf)	CN E	Description		
1	18,297	30 V	Voods, Go	od, HSG A	
	25,363	39 >	>75% Gras	s cover, Go	ood, HSG A
	9,642	98 F	Roofs, HSG	β A	
	5,338	98 F	Paved park	ing, HSG A	
1	58,640	38 \	Veighted A	verage	
1	43,660	ç	0.56% Per	vious Area	
	14.980	ç	9.44% Impe	ervious Area	а
	.,				-
Tc	Lenath	Slope	Velocitv	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
10.6	50	0.1100	0.08		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C
					Woodland Kv= 5.0 fps
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D
					Short Grass Pasture Kv= 7.0 fps
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1
					Woodland Kv= 5.0 fps
14.5	417	Total			

Subcatchment DA1: DA1



Summary for Subcatchment DA2: DA2

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP2 : SP2 0.000 af, Depth= 0.00"

A	Area (sf)	CN D	Description		
	73,961	30 V	Voods, Go	od, HSG A	
	5,863	39 >	75% Gras	s cover, Go	ood, HSG A
	79,824	31 V	Veighted A	verage	
79,824 100.00%			00.00% Pe	ervious Are	а
_					
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
13.6	50	0.0600	0.06		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
2.5	164	0.0490	1.11		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
0.8	73	0.0480	1.53		Shallow Concentrated Flow, C to D
					Short Grass Pasture Kv= 7.0 fps
4.6	218	0.0250	0.79		Shallow Concentrated Flow, D to SP2
					Woodland Kv= 5.0 fps
21.5	505	Total			

Subcatchment DA2: DA2



Summary for Subcatchment DA3: DA3

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP3 : SP3 0.000 af, Depth= 0.00"

A	rea (sf)	CN [Description					
1	05,276	30 \	Voods, Go	od, HSG A				
	964	39 >	39 >75% Grass cover, Good, HSG A					
	245	98 F	Paved parking, HSG A					
	1,705	96 (Gravel surfa	ace, HSG A	۱ <u> </u>			
1	08,190	31 \	Veighted A	verage				
1	107,945 99.77% Pervious Area							
	245	().23% Impe	ervious Are	а			
Тс	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
17.9	50	0.0300	0.05		Sheet Flow, A TO B			
					Woods: Dense underbrush n= 0.800 P2= 3.33"			
8.0	362	0.0230	0.76		Shallow Concentrated Flow, B to C			
					Woodland Kv= 5.0 fps			
0.6	125	0.0340	3.74		Shallow Concentrated Flow, C to SP2			
					Paved Kv= 20.3 fps			
26.5	537	Total						

Subcatchment DA3: DA3



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow /	Area =	3.642 ac,	9.44% Impervious,	Inflow Depth = 0.0	00" for WQv event
Inflow	=	0.00 cfs @	0.00 hrs, Volume	= 0.000 af	
Primar	y =	0.00 cfs @	0.00 hrs, Volume	= 0.000 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area =	1.833 ac,	0.00% Impervious,	Inflow Depth = 0.0	00" for WQv event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	= 0.000 af	
Primary	/ =	0.00 cfs @	0.00 hrs, Volume=	= 0.000 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow /	Area =	2.484 ac,	0.23% Impervious	s, Inflow Depth =	0.0	0" for WC	Qv event
Inflow	=	0.00 cfs @	0.00 hrs, Volun	ne= 0.000	af		
Primary	y =	0.00 cfs @	0.00 hrs, Volun	ne= 0.000	af,	Atten= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-36.00 hrs, dt= 0.01 hrs



Pond SP3: SP3



Event#	Event	Storm Type	Curve	Mode	Duration	B/B	Depth	AMC
	Name				(hours)		(inches)	
1	2YR	Type III 24-hr		Default	24.00	1	3.33	2
2	10YR	Type III 24-hr		Default	24.00	1	4.90	2
3	25YR	Type III 24-hr		Default	24.00	1	6.12	2
4	100YR	Type III 24-hr		Default	24.00	1	8.56	2
5	WQv	Type III 24-hr		Default	24.00	1	1.21	2

Rainfall Events Listing (selected events)

Area Listing (all nodes)

Area	CN	Description
 (sq-ft)		(subcatchment-numbers)
78,279	39	>75% Grass cover, Good, HSG A (DA1, DA2a, DA2b, DA2c, DA3a, DA3b, DA3c,
		DA3d)
387	76	Gravel roads, HSG A (DA3b)
46,508	98	Paved parking, HSG A (DA1, DA2bi, DA3ai, DA3bi, DA3ci, DA3di)
878	40	Pervious Pavers (DA2a)
28,678	98	Roofs, HSG A (DA1, R1E, R1W)
1,223	98	Sidewalk, HSG A (DA3ai)
3,330	98	Sidewalks, HSG A (DA2bi, DA3bi, DA3ci)
2,082	98	Water Surface, HSG A (DA2bi, DA3ai)
185,226	30	Woods, Good, HSG A (DA1, DA2a, DA2c, DA3a, DA3b, DA3c)
346,591	48	TOTAL AREA

Soil Listing (all nodes)

Area	Soil	Subcatchment
(sq-ft)	Group	Numbers
345,713	HSG A	DA1, DA2a, DA2b, DA2bi, DA2c, DA3a, DA3ai, DA3b, DA3bi, DA3c, DA3ci,
		DA3d, DA3di, R1E, R1W
0	HSG B	
0	HSG C	
0	HSG D	
878	Other	DA2a
346,591		TOTAL AREA

19038-POST V4

Prepared by {enter your	company name here}	
HydroCAD® 10.10-7a s/n 0	445 © 2021 HydroCAD Software Solutions LLC	

Printed 12/12/2021 Page 5

HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Sub
(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	(sq-ft)	Cover	Nur
78,279	0	0	0	0	78,279	>75% Grass	
						cover, Good	
387	0	0	0	0	387	Gravel roads	
46,508	0	0	0	0	46,508	Paved parking	
0	0	0	0	878	878	Pervious Pavers	
28,678	0	0	0	0	28,678	Roofs	
1,223	0	0	0	0	1,223	Sidewalk	
3,330	0	0	0	0	3,330	Sidewalks	
2,082	0	0	0	0	2,082	Water Surface	
185,226	0	0	0	0	185,226	Woods, Good	
345,713	0	0	0	878	346,591	TOTAL AREA	

Ground Covers (all nodes)

19038-POST V4

Prepared by {enter	your company name here}	
HydroCAD® 10.10-7a	s/n 01445 © 2021 HydroCAD Software Solutions LLC	

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Width (inches)	Diam/Height (inches)	Inside-Fill (inches)
1	100	50.00	49.98	4.0	0.0050	0.013	0.0	12.0	0.0
2	200	52.70	52.68	4.0	0.0050	0.013	0.0	12.0	0.0
3	200	51.66	51.41	50.0	0.0050	0.013	0.0	8.0	0.0
4	BIO1	55.09	54.87	45.0	0.0049	0.013	0.0	12.0	0.0
5	BIO1	55.38	55.19	38.0	0.0050	0.010	0.0	4.0	0.0
6	BIO2	59.00	58.88	25.0	0.0048	0.013	0.0	12.0	0.0
7	BIO2	59.30	59.18	25.0	0.0048	0.010	0.0	4.0	0.0
8	DMH	54.30	54.26	9.0	0.0044	0.013	0.0	12.0	0.0
9	DMH	53.78	53.78	98.0	0.0000	0.013	0.0	12.0	0.0

Pipe Listing (all nodes)

Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.00" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.00 cfs 4 cf
Subcatchment DA2a: DA2a	Runoff Area=22,132 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=339' Tc=17.5 min CN=34 Runoff=0.00 cfs 0 cf
Subcatchment DA2b: DA2b pervious	Runoff Area=4,670 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=39 Runoff=0.00 cfs 1 cf
Subcatchment DA2bi: DA2b impervious	Runoff Area=9,894 sf 100.00% Impervious Runoff Depth=3.10" Tc=5.0 min CN=98 Runoff=0.76 cfs 2,553 cf
Subcatchment DA2c: DA2c	Runoff Area=32,448 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=255' Tc=21.2 min CN=34 Runoff=0.00 cfs 0 cf
Subcatchment DA3a: DA3a pervious	Runoff Area=28,939 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=34 Runoff=0.00 cfs 0 cf
Subcatchment DA3ai: DA3a impervious	Runoff Area=19,495 sf 100.00% Impervious Runoff Depth=3.10" Tc=5.0 min CN=98 Runoff=1.50 cfs 5,031 cf
Subcatchment DA3b: DA3b pervious	Runoff Area=6,632 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=36 Runoff=0.00 cfs 0 cf
Subcatchment DA3bi: DA3b impervious	Runoff Area=10,502 sf 100.00% Impervious Runoff Depth=3.10" Tc=5.0 min CN=98 Runoff=0.81 cfs 2,710 cf
Subcatchment DA3c: DA3c pervious	Runoff Area=26,092 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=33 Runoff=0.00 cfs 0 cf
Subcatchment DA3ci: DA3c impervious	Runoff Area=3,228 sf 100.00% Impervious Runoff Depth=3.10" Tc=5.0 min CN=98 Runoff=0.25 cfs 833 cf
Subcatchment DA3d: DA3d pervious	Runoff Area=197 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=39 Runoff=0.00 cfs 0 cf
Subcatchment DA3di: DA3d impervious	Runoff Area=4,686 sf 100.00% Impervious Runoff Depth=3.10" Tc=5.0 min CN=98 Runoff=0.36 cfs 1,209 cf
SubcatchmentR1E: EAST ROOF	Runoff Area=9,274 sf 100.00% Impervious Runoff Depth=3.10" Tc=5.0 min CN=98 Runoff=0.71 cfs 2,393 cf
Subcatchment R1W: WEST ROOF	Runoff Area=9,762 sf 100.00% Impervious Runoff Depth=3.10" Tc=5.0 min CN=98 Runoff=0.75 cfs 2,519 cf
Pond 100: CB 100 12.0" Ro	Peak Elev=50.32' Inflow=0.25 cfs 833 cf bund Culvert n=0.013 L=4.0' S=0.0050 '/' Outflow=0.25 cfs 833 cf

19038-POST V4

Prepared by {enter your company name here} HydroCAD® 10.10-7a s/n 01445 © 2021 HydroCAD Software Solutions LLC

Type III 24-hr 2YR Rainfall=3.33" Printed 12/12/2021

Page 8

Pond 200: CB 200	Peak Elev=52.08' Inflow=0.36 cfs 1,209 cf Primary=0.36 cfs 1,209 cf Secondary=0.00 cfs 0 cf Outflow=0.36 cfs 1,209 cf
Pond BIO1: BIO 1	Peak Elev=59.40' Storage=1,142 cf Inflow=1.50 cfs 5,031 cf Outflow=1.28 cfs 5,031 cf
Pond BIO2: BIO 2	Peak Elev=63.10' Storage=364 cf Inflow=0.76 cfs 2,554 cf Outflow=0.73 cfs 2,554 cf
Pond DMH: DMH 200	Peak Elev=54.51' Inflow=0.81 cfs 2,710 cf Primary=0.69 cfs 2,661 cf Secondary=0.12 cfs 50 cf Outflow=0.81 cfs 2,710 cf
Pond RB1: RB 101,102	Peak Elev=41.62' Storage=160 cf Inflow=0.25 cfs 833 cf Discarded=0.06 cfs 833 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 833 cf
Pond RB2: RB 202,202,203	Peak Elev=46.18' Storage=130 cf Inflow=0.33 cfs 297 cf Discarded=0.09 cfs 297 cf Primary=0.00 cfs 0 cf Outflow=0.09 cfs 297 cf
Pond RB3: RB 300	Peak Elev=58.50' Storage=0 cf Inflow=0.00 cfs 0 cf Discarded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Pond SP1: SP1	Inflow=0.00 cfs 4 cf Primary=0.00 cfs 4 cf
Pond SP2: SP2	Inflow=0.00 cfs_0 cf Primary=0.00 cfs_0 cf
Pond SP3: SP3	Inflow=0.00 cfs_0 cf Primary=0.00 cfs_0 cf
Pond TT1: Tree Trench 1	Peak Elev=54.55' Storage=487 cf Inflow=0.69 cfs 2,661 cf Discarded=0.12 cfs 2,425 cf Primary=0.26 cfs 236 cf Outflow=0.38 cfs 2,661 cf
Pond TT2: Tree Trench 2	Peak Elev=52.98' Storage=153 cf Inflow=0.36 cfs 1,209 cf Discarded=0.03 cfs 912 cf Primary=0.33 cfs 297 cf Outflow=0.36 cfs 1,209 cf
Pond URC1: URC-1	Peak Elev=49.86' Storage=0.020 af Inflow=1.37 cfs 5,317 cf Outflow=0.40 cfs 5,317 cf
Pond URC2: URC-2	Peak Elev=56.54' Storage=1,049 cf Inflow=1.43 cfs 4,948 cf Outflow=0.29 cfs 4,948 cf
Pond URC3: URC-3	Peak Elev=58.14' Storage=623 cf Inflow=0.75 cfs 2,519 cf Outflow=0.13 cfs 2,519 cf

Total Runoff Area = 346,591 sf Runoff Volume = 17,255 cf Average Runoff Depth = 0.60" 76.39% Pervious = 264,770 sf 23.61% Impervious = 81,821 sf

Summary for Subcatchment DA1: DA1

Runoff = 0.00 cfs @ 24.04 hrs, Volume= Routed to Pond SP1 : SP1 4 cf, Depth= 0.00"

A	rea (sf)	CN E	Description		
1	18,297	30 V	Voods, Go	od, HSG A	
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A
	9,642	98 F	Roofs, HSG	βA	
	5,338	98 F	Paved park	ing, HSG A	
1	58,640	38 V	Veighted A	verage	
1	43,660	g	0.56% Per	vious Area	
	14,980	g	.44% Impe	ervious Area	а
			•		
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.6	50	0.1100	0.08		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C
					Woodland Kv= 5.0 fps
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D
					Short Grass Pasture Kv= 7.0 fps
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1
					Woodland Kv= 5.0 fps
14.5	417	Total			

Subcatchment DA1: DA1



Summary for Subcatchment DA2a: DA2a

CN for permeable pavers taken from RI Stormwater Design

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond RB3 : RB 300 0 cf, Depth= 0.00"

	Ar	ea (sf)	CN	Description		
	1	12,073	30	Woods, Go	od, HSG A	
		9,181	39	>75% Gras	s cover, Go	bod, HSG A
*		878	40	Pervious Pa	avers	
	2	22,132	34	Weighted A	verage	
	2	22,132		100.00% P	ervious Are	a
	Тс	Length	Slope	e Velocity	Capacity	Description
(m	in)	(feet)	(ft/ft) (ft/sec)	(cfs)	
13	3.6	50	0.0600	0.06		Sheet Flow, A TO B
						Woods: Dense underbrush n= 0.800 P2= 3.33"
2	2.5	164	0.0490) 1.11		Shallow Concentrated Flow, B to C
						Woodland Kv= 5.0 fps
	1.4	125	0.0480) 1.53		Shallow Concentrated Flow, C to D (RB 3)
						Short Grass Pasture Kv= 7.0 fps
17	7.5	339	Total			

Subcatchment DA2a: DA2a



Summary for Subcatchment DA2b: DA2b pervious

Runoff = 0.00 cfs @ 24.00 hrs, Volume= Routed to Pond BIO2 : BIO 2 1 cf, Depth= 0.00"



Summary for Subcatchment DA2bi: DA2b impervious

Runoff = 0.76 cfs @ 12.07 hrs, Volume= 2,553 cf, Depth= 3.10" Routed to Pond BIO2 : BIO 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

A	Area (sf)	CN	Description		
	7,075	98	Paved park	ing, HSG A	N Contraction of the second seco
*	2,044	98	Sidewalks,	HŠG A	
	775	98	Water Surfa	ace, HSG A	
	9,894	98	Weighted A	verage	
	9,894		100.00% Im	npervious A	rea
Tc (min)	Length (feet)	Slop (ft/fl	e Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry, Direct

Subcatchment DA2bi: DA2b impervious



Summary for Subcatchment DA2c: DA2c

CN for permeable pavers taken from RI Stormwater Design

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP2 : SP2 0 cf, Depth= 0.00"

Α	rea (sf)	CN D	Description		
	18,364	30 V	Voods, Go	od, HSG A	
	14,084	39 >	75% Gras	s cover, Go	ood, HSG A
	32,448	34 V	Veighted A	verage	
	32,448	1	00.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
17.9	50	0.0300	0.05		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
1.2	50	0.0200	0.71		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
2.1	155	0.0320	1.25		Shallow Concentrated Flow, C to SP2
					Short Grass Pasture Kv= 7.0 fps
21.2	255	Total			

Subcatchment DA2c: DA2c



Summary for Subcatchment DA3a: DA3a pervious

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond BIO1 : BIO 1 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

Area	(sf)	CN	Description		
12,3	804	39	>75% Gras	s cover, Go	bod, HSG A
16,6	35	30	Woods, Go	od, HSG A	
28,9	39	34	Weighted A	verage	
28,9	39		100.00% Pe	ervious Are	a
Tc Ler	ngth	Slope	e Velocity	Capacity	Description
<u>(min)</u> (f	eet)	(ft/ft) (ft/sec)	(cfs)	
5.0					Direct Entry, Direct
					-

Subcatchment DA3a: DA3a pervious



Summary for Subcatchment DA3ai: DA3a impervious

Runoff = 1.50 cfs @ 12.07 hrs, Volume= 5,031 cf, Depth= 3.10" Routed to Pond BIO1 : BIO 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

	Area (sf)	CN	Description					
	16,965	98	Paved park	ing, HSG A	N			
*	1,223	98	Sidewalk, H	ISG A				
	1,307	98	Water Surfa	ace, HSG A	Ν			
	19,495	19,495 98 Weighted Average						
	19,495		100.00% Impervious Area					
_				.				
	Fc Length	Slop	e Velocity	Capacity	Description			
<u>(mi</u>	n) (feet)	(ft/f	t) (ft/sec)	(cfs)				
5	.0				Direct Entry, Direct			

Subcatchment DA3ai: DA3a impervious





Summary for Subcatchment DA3b: DA3b pervious

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond DMH : DMH 200 0 cf, Depth= 0.00"

Area (sf)	CN	Description							
2,453	2,453 39 >75% Grass cover, Good, HSG A								
3,792	30	Woods, Good, HSG A							
387	76	Gravel roads, HSG A							
6,632	36	Weighted Average							
6,632		100.00% Pervious Area							
Tc Length	Slop	pe Velocity Capacity Description							
(min) (feet)	(ft/	/ft) (ft/sec) (cfs)							
5.0		Direct Entry, Direct							
Subcatchment DA3b: DA3b pervious									
Hydrograph									



Summary for Subcatchment DA3bi: DA3b impervious

Runoff = 0.81 cfs @ 12.07 hrs, Volume= 2,710 cf, Depth= 3.10" Routed to Pond DMH : DMH 200

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

	Area	ı (sf)	CN	Description		
	9	,746	98	Paved park	ing, HSG A	N
*		756	98	Sidewalks,	HŠG A	
	10	,502	98	Weighted A	verage	
	10	,502		100.00% Im	npervious A	vrea
	TC Le	ength	Slope	e Velocity	Capacity	Description
(m	nin)	(feet)	(ft/ft) (ft/sec)	(cfs)	
	5.0					Direct Entry, Direct

Subcatchment DA3bi: DA3b impervious



0 cf, Depth= 0.00"

Summary for Subcatchment DA3c: DA3c pervious

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond 100 : CB 100

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

Area	a (sf)	CN	Description			
10	,027	39	>75% Gras	s cover, Go	bod, HSG A	
16	6,065	30	Woods, Go	od, HSG A		
26	,092	33	Weighted A	verage		
26	6,092	100.00% Pervious Area				
Tc L	ength	Slop	e Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/f	:) (ft/sec)	(cfs)		
5.0					Direct Entry, Direct	

Subcatchment DA3c: DA3c pervious



Summary for Subcatchment DA3ci: DA3c impervious

Runoff	=	0.25 cfs @	12.07 hrs,	Volume=	833 cf,	Depth=	3.10"
Routed	to Pond	100 : CB 100	C				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 2YR Rainfall=3.33"

Α	rea (sf)	CN	Description		
	2,698	98	Paved park	ing, HSG A	N N N N N N N N N N N N N N N N N N N
*	530	98	Sidewalks,	HŠG A	
	3,228	98	Weighted A	verage	
	3,228		100.00% Im	npervious A	rea
Тс	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	
5.0					Direct Entry, Direct

Subcatchment DA3ci: DA3c impervious



Summary for Subcatchment DA3d: DA3d pervious

Runoff = 0.00 cfs @ 24.00 hrs, Volume= Routed to Pond 200 : CB 200 0 cf, Depth= 0.00"



Summary for Subcatchment DA3di: DA3d impervious

Runoff = 0.36 cfs @ 12.07 hrs, Volume= 1,209 cf, Depth= 3.10" Routed to Pond 200 : CB 200

Area (sf) CN Description	
4,686 98 Paved parking, HSG A	
4,686 100.00% Impervious Area	
Tc Length Slope Velocity Capacity Description (min) (feet) (ft/ft) (ft/sec) (cfs)	
5.0 Direct Entry,	
Subcatchment DA3di: DA3d impervious	
Hydrograph	
0.4 0.8 0.38 0.38	Runoff
0.32 2	
0.26	
⁵ 0.22 ¹ ↓	
£ 0.18	
0.16	
0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72	

Summary for Subcatchment R1E: EAST ROOF

Runoff = 0.71 cfs @ 12.07 hrs, Volume= Routed to Pond URC2 : URC-2 2,393 cf, Depth= 3.10"



Summary for Subcatchment R1W: WEST ROOF

Runoff = 0.75 cfs @ 12.07 hrs, Volume= Routed to Pond URC3 : URC-3 2,519 cf, Depth= 3.10"


Summary for Pond 100: CB 100

[57] Hint: Peaked at 50.32' (Flood elevation advised)

Inflow Are	a =	29,320 sf,	, 11.01% Im	pervious,	Inflow Depth =	0.34"	for 2YR	event
Inflow	=	0.25 cfs @	12.07 hrs,	Volume=	833 (cf		
Outflow	=	0.25 cfs @	12.07 hrs,	Volume=	833 (of, Atter	ו= 0%, La	ig= 0.0 min
Primary	=	0.25 cfs @	12.07 hrs,	Volume=	833 (cf		•
Routed	to Pond	RB1 : RB 10	1,102					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 50.32' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.00'	12.0" Round Culvert L= 4.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 50.00' / 49.98' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.25 cfs @ 12.07 hrs HW=50.32' (Free Discharge) **1=Culvert** (Barrel Controls 0.25 cfs @ 1.72 fps)



Pond 100: CB 100

Summary for Pond 200: CB 200

[57] Hint: Peaked at 52.08' (Flood elevation advised)

Inflow Area	a =	4,883 sf,	95.97% Ir	npervious,	Inflow Depth =	2.97"	for 2YR event
Inflow	=	0.36 cfs @	12.07 hrs,	Volume=	1,209 c	f	
Outflow	=	0.36 cfs @	12.07 hrs,	Volume=	1,209 c	f, Atten	= 0%, Lag= 0.0 min
Primary	=	0.36 cfs @	12.07 hrs,	Volume=	1,209 c	f	-
Routed	to Pond	TT2 : Tree T	rench 2				
Secondary	/ =	0.00 cfs @	0.00 hrs,	Volume=	0 c	f	
Routed	to Pond	RB2 : RB 20	2,202,203				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 52.08' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	52.70'	12.0" Round Culvert L= 4.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 52.70' / 52.68' S= 0.0050 '/' Cc= 0.900
#2	Primary	51.66'	8.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.66' / 51.41' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Secondary	55.79'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.36 cfs @ 12.07 hrs HW=52.08' (Free Discharge) ←2=Culvert (Barrel Controls 0.36 cfs @ 2.20 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=51.66' (Free Discharge) -1=Culvert (Controls 0.00 cfs) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 200: CB 200



Summary for Pond BIO1: BIO 1

Inflow Ar Inflow Outflow Primary Route	ea = = 1. = 1. = 1. ed to Pond U	48,434 sf, 40 50 cfs @ 12 28 cfs @ 12 28 cfs @ 12 RC1 : URC-1	0.25% Imperviou 07 hrs, Volume 12 hrs, Volume 12 hrs, Volume	is, Inflow De ;= { ;= { ;= {	pth = 1.25" fc 5,031 cf 5,031 cf, Atten= 5,031 cf	or 2YR event 14%, Lag= 2.7 min				
Routing I Peak Ele	Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 59.40' @ 12.12 hrs Surf.Area= 1,608 sf Storage= 1,142 cf									
Plug-Flov Center-o	w detention t f-Mass det. t	ime= 67.5 min ime= 67.5 min	calculated for 5 (822.1 - 754.6	5,031 cf (100)	% of inflow)					
Volume	Invert	Avail.Stora	age Storage D	escription						
#1	58.50'	2,210) cf Custom S	stage Data (Prismatic)Listed	below (Recalc)				
Elevatio (fee	n Su t)	rf.Area (sq-ft) (Inc.Store cubic-feet)	Cum.Store (cubic-feet	;)					
58.5	0	800	0	()					
59.0	0	1,380	545	545	5					
60.0	0	1,950	1,665	2,210)					
Device	Routing	Invert	Outlet Devices							
#1	Primary	55.09'	12.0" Round C L= 45.0' CPP, Inlet / Outlet Inv n= 0.013 Corru	ulvert projecting, n ert= 55.09' /	o headwall, Ke= 54.87' S= 0.004 mooth interior F	: 0.900 49 '/' Cc= 0.900 low Area= 0 79 sf				
#2	Device 1	59.25'	24.0" Horiz. Or	ifice/Grate	C= 0.600					
			Limited to weir f	low at low h	eads					
#3	Device 1	55.38'	4.0" Round Culvert L= 38.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 55.38' / 55.19' S= 0.0050 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf							
#4	Device 3	58.50'	2.470 in/hr Exfi	iltration ove	r Surface area					
Primary	Primary OutFlow Max=1.28 cfs @ 12.12 hrs HW=59.40' (Free Discharge)									

-Culvert (Passes 1.28 cfs of 5.83 cfs potential flow) -2=Orifice/Grate (Weir Controls 1.19 cfs @ 1.26 fps) -3=Culvert (Passes 0.09 cfs of 0.62 cfs potential flow) -4=Exfiltration (Exfiltration Controls 0.09 cfs)

Pond BIO1: BIO 1



Summary for Pond BIO2: BIO 2

Inflow Ar Inflow Outflow Primary Route	rea = 0 = 0 = 0 = 0 ed to Pond U	14,564 sf, 6).76 cfs @ 12).73 cfs @ 12).73 cfs @ 12).73 cfs @ 12	7.93% Imperviou 2.07 hrs, Volume 2.09 hrs, Volume 2.09 hrs, Volume	s, Inflow D = = =	epth = 2.10" 2,554 cf 2,554 cf, Atter 2,554 cf	for 2YR event າ= 4%, Lag= 1.3 min			
Routing Peak Ele	Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 63.10' @ 12.09 hrs Surf.Area= 773 sf Storage= 364 cf								
Plug-Flor Center-o	w detention f-Mass det.	time= 40.9 mii time= 40.9 mii	n calculated for 2 n (795.8 - 754.9	,554 cf (100))% of inflow)				
Volume	Invert	Avail.Stor	age Storage De	escription					
#1	62.50'	1,34	0 cf Custom S	tage Data ((Prismatic)List	ed below (Recalc)			
Elevatio (fee	n Sı t)	urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Stor (cubic-fee	re t)				
62.5	50	450	0	•	0				
63.0	0	700	288	28	8				
64.0	0	1,405	1,053	1,34	0				
Device	Routing	Invert	Outlet Devices						
#1	Primary	59.00'	12.0" Round C L= 25.0' CPP, e Inlet / Outlet Inve	ulvert end-section ert= 59.00',	conforming to / 58.88' S= 0.	fill, Ke= 0.500 0048 '/' Cc= 0.900 Flow Area= 0.70 sf			
#2	Device 1	63.00'	24.0" Horiz. Ori	fice/Grate	C= 0.600 neads	110W Alea- 0.79 Si			
#3	Device 1	59.30'	4.0" Round Culvert L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= $59.30' / 59.18'$ S= 0.0048 '/' Cc= 0.900 n= 0.010 PVC smooth interior Flow Area= 0.09 sf						
#4	Device 3	62.50'	2.470 in/hr Exfi	Itration ove	er Surface are	а			
Primary	OutFlow M	lax=0.73 cfs @	0 12.09 hrs HW=	63.10' (Fr	ee Discharge)				

-1=Culvert (Passes 0.73 cfs of 7.18 cfs potential flow) -2=Orifice/Grate (Weir Controls 0.69 cfs @ 1.05 fps) -3=Culvert (Passes 0.04 cfs of 0.63 cfs potential flow) -4=Exfiltration (Exfiltration Controls 0.04 cfs)

Pond BIO2: BIO 2



Summary for Pond DMH: DMH 200

[57] Hint: Peaked at 54.51' (Flood elevation advised)

Inflow Area	a =	17,134 sf,	61.29% In	npervious,	Inflow Depth =	1.90"	for 2YR event	
Inflow	=	0.81 cfs @	12.07 hrs,	Volume=	2,710 cf			
Outflow	=	0.81 cfs @	12.07 hrs,	Volume=	2,710 cf	, Atten	= 0%, Lag= 0.0 mii	n
Primary	=	0.69 cfs @	12.07 hrs,	Volume=	2,661 cf		·	
Routed	to Pond	TT1: Tree T	rench 1					
Secondary	=	0.12 cfs @	12.07 hrs,	Volume=	50 cf			
Routed	to Pond	URC1 : URC	;-1					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 54.51' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	54.30'	12.0" Round Culvert
			Inlet / Outlet Invert= 54.30' / 54.26' S= 0.0044 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior. Flow Area= 0.79 sf
#2	Primary	53.78'	12.0" Round Culvert L= 98.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 53.78' / 53.78' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.69 cfs @ 12.07 hrs HW=54.51' (Free Discharge) ←2=Culvert (Barrel Controls 0.69 cfs @ 1.57 fps)

Secondary OutFlow Max=0.12 cfs @ 12.07 hrs HW=54.51' (Free Discharge) -1=Culvert (Barrel Controls 0.12 cfs @ 1.54 fps)

Pond DMH: DMH 200



Summary for Pond RB1: RB 101,102

Inflow Ar	rea = 2	29,320 sf, 1	1.01%	6 Impervious, Inflow Depth = 0.34" for 2YR event					
Inflow	= 0.2	25 cfs @ 12	2.07 hi	rs, Volume= 833 cf					
Outflow	= 0.0)6 cfs @11	.75 h	rs, Volume= 833 cf, Atten= 76%, Lag= 0.0 min					
Discarde	ed = 0.0)6 cfs @ 11	.75 hi	rs. Volume= 833 cf					
Primary	= 0.0	0 cfs @ 0	00 h	rs, Volume= 0 cf					
Poute	Polyted to Dond SD3 · SD3								
Noule									
Routing	by Stor-Ind me	ethod Time :	Snan	= 0.00-72.00 hrs. dt= 0.01 hrs / 3					
Peak Fle	y eter marma y= 41 62' @ 1	12 44 hrs S	urf Ar	$e_{a=157 \text{ sf}}$ Storage= 160 cf					
		2.11110 0	GITI/ G						
Plug-Flo	w detention tin	me= 12.6 mir	n calc	ulated for 833 cf (100% of inflow)					
Center_0	of-Mass det tin	nc = 12.0 min	n (76	7 2 - 754 6)					
Ochici-o		nc= 12.0 mi	1(70	1.2 - 134.0)					
Volume	Invert	Avail.Stor	age	Storage Description					
#1	41.00'	33	9 cf	6.00'D x 6.00'H Recharger x 2 Inside #2					
#2	39.00'	35	5 cf	10.00'D x 9.00'H Stone x 2					
			-	1,414 cf Overall - 339 cf Embedded = 1,074 cf x 33.0% Voids					
		69	4 cf	Total Available Storage					
				C C C C C C C C C C C C C C C C C C C					
Device	Routing	Invert	Outle	et Devices					
#1	Discarded	39.00'	8.27	0 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01'					
#2	Primary	46.50'	5.0' I	ong x 5.0' breadth Broad-Crested Rectangular Weir X 2.00					
	,		Head	d (feet) 0 20 0 40 0 60 0 80 1 00 1 20 1 40 1 60 1 80 2 00					
			2 50	3 00 3 50 4 00 4 50 5 00 5 50					
			Coof	(English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65					
				. (EIIIIIII) 2.34 2.30 2.70 2.00 2.00 2.00 2.03 2.03 2.03					
			2.00	2.01 2.00 2.00 2.10 2.14 2.19 2.00					
Discord	Discounded OutFlow May $= 0.00$ of $= 0.00$ (I at $= 1.00 = 0.00$) (First Discharms)								
UISCARGED OUTFIOW Max=0.06 CTS @ 11.75 NTS HW=39.09" (Free Discharge)									

1=Exfiltration (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=39.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Pond RB1: RB 101,102



Summary for Pond RB2: RB 202,202,203

[92] Warning: Device #2 is above defined storage

Inflow An Inflow Outflow Discarde Primary Route	rea = = = ed = = ed to Pond	4,883 sf, 95.9 0.33 cfs @ 12.08 0.09 cfs @ 12.03 0.09 cfs @ 12.03 0.00 cfs @ 0.00 SP3 : SP3	7% Impervious, Inflow Depth = 0.73" for 2YR event 8 hrs, Volume= 297 cf 9 hrs, Volume= 297 cf, Atten= 73%, Lag= 0.0 min 9 hrs, Volume= 297 cf 0 hrs, Volume= 0 cf					
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 46.18' @ 12.35 hrs Surf.Area= 236 sf Storage= 130 cf								
Plug-Flo Center-c	Plug-Flow detention time= 15.6 min calculated for 297 cf (100% of inflow) Center-of-Mass det. time= 15.6 min (748.5 - 733.0)							
Volume	Inver	t Avail.Storage	e Storage Description					
#1	46.50)' 509 c	f 6.00'D x 6.00'H Recharger x 3 Inside #2					
#2	44.50)' 532 c	f 10.00'D x 9.00'H Stone x 3					
			2,121 cf Overall - 509 cf Embedded = 1,612 cf x 33.0% Voids					
		1,041 c	f Total Available Storage					
Device	Routing	Invert O	utlet Devices					
#1	Discarded	44.50' 8.	270 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01'					
#2	Primary	55.61' 5 .	0' long x 5.0' breadth Broad-Crested Rectangular Weir X 2.00					
	-	He	ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00					

2.50 3.00 3.50 4.00 4.50 5.00 5.50

Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65

2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88 **Discarded OutFlow** Max=0.09 cfs @ 12.03 hrs HW=44.63' (Free Discharge) **—1=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=44.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Pond RB2: RB 202,202,203



Summary for Pond RB3: RB 300

Inflow Are	ea =	22,132 sf,	0.00%	Impervious,	Inflow Depth	n = 0.00"	for 2YR even	ent
Inflow	=	0.00 cfs @ 0	0.00 hr	s, Volume=		0 cf		
Outflow	=	0.00 cfs @ (0.00 hr	s, Volume=		0 cf, Atten	n= 0%, Lag=	0.0 min
Discarde	d =	0.00 cfs @ 0	0.00 hr	s, Volume=		0 cf	, U	
Primary Route	rimary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Pond SP2 : SP2							
Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 58.50' @ 0.00 hrs Surf.Area= 57 sf Storage= 0 cf								
Plug-Flov Center-of	Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)							
Volume	Inver	t Avail.Stor	rage	Storage Desc	ription			
#1	59.50)' <u>5</u>	50 cf	4.00'D x 4.00	'H Recharg	er Inside #2	2	
#2	58.50	' 13	33 cf	6.00'D x 8.00	'H Stone x	2		
				452 cf Overal	l - 50 cf Emb	bedded = 40	02 cf x 33.09	% Voids
		18	33 cf	Total Availabl	e Storage			
Device	Routing	Invert	Outle	t Devices				
#1 #2	Discarded Primary	58.50' 65.50'	8.270 5.0' l Head 2.50 Coef. 2.65) in/hr Exfiltra ong x 5.0' bro (feet) 0.20 0 3.00 3.50 4.1 (English) 2.3 2.67 2.66 2.1	addition X 2.00 eadth Broad 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.60 0 0.40 0.70 2.70 0.40 0.70 2.70	over Surfa d-Crested F .80 1.00 1 00 5.50 0 2.68 2.60 74 2.79 2.8	ace area Ph Rectangular .20 1.40 1.6 8 2.66 2.65	ase-In= 0.01' Weir 30 1.80 2.00 2.65 2.65
Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=58.50' (Free Discharge) —1=Exfiltration (Controls 0.00 cfs)								

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=58.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond RB3: RB 300



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area	=	158,640 sf,	9.44% In	npervious,	Inflow Depth =	0.00"	for 21	R event
Inflow		=	0.00 cfs @ 2	24.04 hrs,	Volume=	4 c	f		
Primar	y	=	0.00 cfs @ 2	24.04 hrs,	Volume=	4 c	f, Atter	n= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area =	=	54,580 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for 2YR event
Inflow	=	:	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	/ =	:	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Time (hours)

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow /	Area	=	34,203 క	sf, 23.14% Ir	mpervious,	Inflow Depth =	0.00"	for 2YR event
Inflow	:	=	0.00 cfs @	0.00 hrs,	Volume=	0 c	f	
Primary	y :	=	0.00 cfs @	0.00 hrs,	Volume=	0 C	f, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP3: SP3

Time (hours)

Summary for Pond TT1: Tree Trench 1

[81] Warning: Exceeded Pond DMH by 0.29' @ 12.55 hrs

17,134 sf, 61.29% Impervious, Inflow Depth = 1.86" for 2YR event Inflow Area = Inflow 0.69 cfs @ 12.07 hrs, Volume= = 2.661 cf 0.38 cfs @ 12.21 hrs, Volume= Outflow = 2,661 cf, Atten= 45%, Lag= 8.5 min 0.12 cfs @ 11.63 hrs, Volume= Discarded = 2,425 cf 0.26 cfs @ 12.21 hrs, Volume= Primary = 236 cf Routed to Pond URC1 : URC-1

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 54.55' @ 12.21 hrs Surf.Area= 639 sf Storage= 487 cf

Plug-Flow detention time= 16.9 min calculated for 2,661 cf (100% of inflow) Center-of-Mass det. time= 16.9 min (772.1 - 755.2)

Volume	Invert	Avail.Stora	ge	Storage Description					
#1	52.34'	627	′ cf	6.20'W x 103.00'L x 3.00'H Prismatoid					
#2	53.78'	32 cf		1,916 cf Overall - 32 cf Embedded = 1,884 cf x 33.3% Voids 8.0" Round Pipe Storage Inside #1 L = 92 0' S= 0.0050 '/'					
		659) cf	Total Available Storage					
Device	Routing	Invert (Outle	at Devices					
#1	Discarded	52.34'	8.270) in/hr Exfiltration over Surface area Phase-In= 0.01'					
#2	Primary	54.30' '	12.0"	Vert. Orifice/Grate C= 0.600					
		I	Limite	ed to weir flow at low heads					
Discard	Discarded OutFlow Max=0.12 cfs @ 11.63 hrs HW=52.37' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.12 cfs)								

Primary OutFlow Max=0.25 cfs @ 12.21 hrs HW=54.55' (Free Discharge) ←2=Orifice/Grate (Orifice Controls 0.25 cfs @ 1.69 fps)

Pond TT1: Tree Trench 1



Summary for Pond TT2: Tree Trench 2

[81] Warning: Exceeded Pond 200 by 0.97' @ 12.55 hrs

Inflow Area	ı =	4,883 sf,	95.97% In	npervious,	Inflow Depth = 2	.97" for	2YR event
Inflow	=	0.36 cfs @	12.07 hrs,	Volume=	1,209 cf		
Outflow	=	0.36 cfs @	12.08 hrs,	Volume=	1,209 cf,	Atten= 0%	6, Lag= 0.4 min
Discarded	=	0.03 cfs @	11.21 hrs,	Volume=	912 cf		-
Primary	=	0.33 cfs @	12.08 hrs,	Volume=	297 cf		
Routed	to Pond	RB2 : RB 20	2,202,203				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 52.98' @ 12.08 hrs Surf.Area= 150 sf Storage= 153 cf

Plug-Flow detention time= 22.1 min calculated for 1,209 cf (100% of inflow) Center-of-Mass det. time= 22.1 min (776.8 - 754.7)

Volume	Invert	Avail.Stor	age	Storage Description				
#1	50.16'	18	4 cf	5.00'W x 30.00'L x 3.80'H Prismatoid				
				570 cf Overall - 17 cf Embedded = 553 cf x 33.3% Voids				
#2	51.66'	1	7 cf	8.0" Round Pipe Storage Inside #1				
				L= 50.0' S= 0.0050 '/'				
		20	1 cf	Total Available Storage				
Device	Routing	Invert	Outle	et Devices				
#1	Discarded	50.16'	8.27	0 in/hr Exfiltration over Surface area Phase-In= 0.01'				
#2	Primary	52.70'	12.0'	"Vert. Orifice/Grate C= 0.600				
	-		Limit	ed to weir flow at low heads				
Discard	ed OutFlow M	ax=0.03 cfs	s @ 1′	1.21 hrs HW=50.20' (Free Discharge)				
└─1=Ex	-1=Exfiltration (Exfiltration Controls 0.03 cfs)							

Primary OutFlow Max=0.33 cfs @ 12.08 hrs HW=52.98' (Free Discharge) **2=Orifice/Grate** (Orifice Controls 0.33 cfs @ 1.81 fps) Pond TT2: Tree Trench 2



Summary for Pond URC1: URC-1

Inflow Area	a =	65,568 sf,	45.75% Impervious,	Inflow Depth =	0.97"	for 2YR event
Inflow	=	1.37 cfs @	12.11 hrs, Volume=	5,317 cf	-	
Outflow	=	0.40 cfs @	12.05 hrs, Volume=	5,317 cf	f, Atten	= 71%, Lag= 0.0 min
Discarded	=	0.40 cfs @	12.05 hrs, Volume=	5,317 cf		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 49.86' @ 12.51 hrs Surf.Area= 0.048 ac Storage= 0.020 af

Plug-Flow detention time= 10.0 min calculated for 5,316 cf (100% of inflow) Center-of-Mass det. time= 10.0 min (827.6 - 817.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	48.60'	0.087 af	16.33'W x 128.59'L x 7.25'H Field A
			0.350 af Overall - 0.087 af Embedded = 0.262 af x 33.3% Voids
#2A	50.60'	0.087 af	ADS_StormTech MC-3500 d +Cap x 34 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			34 Chambers in 2 Rows
			Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf
		0.175 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	48.60'	8.270 in/hr Exfiltration over Surface area	Phase-In= 0.01'

Discarded OutFlow Max=0.40 cfs @ 12.05 hrs HW=48.68' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.40 cfs)

Pond URC1: URC-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cfOverall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

17 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 125.59' Row Length +18.0" End Stone x 2 = 128.59' Base Length 2 Rows x 77.0" Wide + 6.0" Spacing x 1 + 18.0" Side Stone x 2 = 16.33' Base Width 24.0" Stone Base + 45.0" Chamber Height + 18.0" Stone Cover = 7.25' Field Height

34 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 2 Rows = 3,798.0 cf Chamber Storage

15,227.2 cf Field - 3,798.0 cf Chambers = 11,429.2 cf Stone x 33.3% Voids = 3,805.9 cf Stone Storage

Chamber Storage + Stone Storage = 7,603.9 cf = 0.175 af Overall Storage Efficiency = 49.9% Overall System Size = 128.59' x 16.33' x 7.25'

34 Chambers 564.0 cy Field 423.3 cy Stone





Pond URC1: URC-1



Summary for Pond URC2: URC-2

Inflow Area	ı =	23,838 sf,	80.41% Impervious	, Inflow Depth =	2.49"	for 2YR event
Inflow	=	1.43 cfs @	12.08 hrs, Volume=	4,948 c	f	
Outflow	=	0.29 cfs @	11.87 hrs, Volume=	4,948 c	f, Atten	i= 80%, Lag= 0.0 min
Discarded	=	0.29 cfs @	11.87 hrs, Volume=	4,948 c	f	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 56.54' @ 12.51 hrs Surf.Area= 1,499 sf Storage= 1,049 cf

Plug-Flow detention time= 18.0 min calculated for 4,947 cf (100% of inflow) Center-of-Mass det. time= 18.0 min (793.9 - 775.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	54.50'	2,405 cf	36.08'W x 41.55'L x 6.75'H Field A
			10,120 cf Overall - 2,898 cf Embedded = 7,222 cf x 33.3% Voids
#2A	56.50'	2,898 cf	ADS_StormTech MC-3500 d +Cap x 25 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			25 Chambers in 5 Rows
			Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf
		5,303 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	54.50'	8.270 in/hr Exfiltration over Surface area	Phase-In= 0.01'
Discard	led OutFlow	Max=0.29 cfs	@ 11.87 hrs HW=54.57' (Free Discharge)	

1=Exfiltration (Exfiltration Controls 0.29 cfs)

Pond URC2: URC-2 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

5 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 39.55' Row Length +12.0" End Stone x 2 = 41.55'Base Length 5 Rows x 77.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 36.08' Base Width 24.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

25 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 5 Rows = 2,897.8 cf Chamber Storage

10,120.0 cf Field - 2,897.8 cf Chambers = 7,222.2 cf Stone x 33.3% Voids = 2,405.0 cf Stone Storage

Chamber Storage + Stone Storage = 5,302.8 cf = 0.122 af Overall Storage Efficiency = 52.4%Overall System Size = $41.55' \times 36.08' \times 6.75'$

25 Chambers 374.8 cy Field 267.5 cy Stone





Pond URC2: URC-2



Summary for Pond URC3: URC-3

Inflow Area	a =	9,762 sf,	100.00% Impervious,	Inflow Depth = 3.10	0" for 2YR event
Inflow	=	0.75 cfs @	12.07 hrs, Volume=	2,519 cf	
Outflow	=	0.13 cfs @	11.69 hrs, Volume=	2,519 cf, At	tten= 82%, Lag= 0.0 min
Discarded	=	0.13 cfs @	11.69 hrs, Volume=	2,519 cf	-

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 58.14' @ 12.51 hrs Surf.Area= 823 sf Storage= 623 cf

Plug-Flow detention time= 25.0 min calculated for 2,519 cf (100% of inflow) Center-of-Mass det. time= 25.0 min (779.6 - 754.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	56.30'	1,243 cf	23.25'W x 35.38'L x 6.25'H Field A
			5,141 cf Overall - 1,409 cf Embedded = 3,732 cf x 33.3% Voids
#2A	57.80'	1,409 cf	ADS_StormTech MC-3500 d +Cap x 12 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			12 Chambers in 3 Rows
			Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf
		2,652 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	56.30'	7.000 in/hr Exfiltration over Surface area	Phase-In= 0.01'
Discard	led OutFlow	Max=0.13 cfs	@ 11.69 hrs HW=56.37' (Free Discharge)	

1=Exfiltration (Exfiltration Controls 0.13 cfs)

Pond URC3: URC-3 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

4 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 32.38' Row Length +18.0" End Stone x 2 = 35.38' Base Length 3 Rows x 77.0" Wide + 6.0" Spacing x 2 + 18.0" Side Stone x 2 = 23.25' Base Width 18.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.25' Field Height

12 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 3 Rows = 1,408.8 cf Chamber Storage

5,141.2 cf Field - 1,408.8 cf Chambers = 3,732.3 cf Stone x 33.3% Voids = 1,242.9 cf Stone Storage

Chamber Storage + Stone Storage = 2,651.7 cf = 0.061 af Overall Storage Efficiency = 51.6%Overall System Size = $35.38' \times 23.25' \times 6.25'$

12 Chambers 190.4 cy Field 138.2 cy Stone





Pond URC3: URC-3



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.15" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.07 cfs 1,973 cf
Subcatchment DA2a: DA2a	Runoff Area=22,132 sf 0.00% Impervious Runoff Depth=0.05" Flow Length=339' Tc=17.5 min CN=34 Runoff=0.00 cfs 93 cf
Subcatchment DA2b: DA2b pervious	Runoff Area=4,670 sf 0.00% Impervious Runoff Depth=0.18" Tc=5.0 min CN=39 Runoff=0.00 cfs 70 cf
Subcatchment DA2bi: DA2b impervious	Runoff Area=9,894 sf 100.00% Impervious Runoff Depth=4.66" Tc=5.0 min CN=98 Runoff=1.13 cfs 3,845 cf
Subcatchment DA2c: DA2c	Runoff Area=32,448 sf 0.00% Impervious Runoff Depth=0.05" Flow Length=255' Tc=21.2 min CN=34 Runoff=0.00 cfs 137 cf
Subcatchment DA3a: DA3a pervious	Runoff Area=28,939 sf 0.00% Impervious Runoff Depth=0.05" Tc=5.0 min CN=34 Runoff=0.00 cfs 122 cf
Subcatchment DA3ai: DA3a impervious	Runoff Area=19,495 sf 100.00% Impervious Runoff Depth=4.66" Tc=5.0 min CN=98 Runoff=2.22 cfs 7,576 cf
Subcatchment DA3b: DA3b pervious	Runoff Area=6,632 sf 0.00% Impervious Runoff Depth=0.09" Tc=5.0 min CN=36 Runoff=0.00 cfs 52 cf
Subcatchment DA3bi: DA3b impervious	Runoff Area=10,502 sf 100.00% Impervious Runoff Depth=4.66" Tc=5.0 min CN=98 Runoff=1.20 cfs 4,081 cf
Subcatchment DA3c: DA3c pervious	Runoff Area=26,092 sf 0.00% Impervious Runoff Depth=0.03" Tc=5.0 min CN=33 Runoff=0.00 cfs 72 cf
Subcatchment DA3ci: DA3c impervious	Runoff Area=3,228 sf 100.00% Impervious Runoff Depth=4.66" Tc=5.0 min CN=98 Runoff=0.37 cfs 1,254 cf
Subcatchment DA3d: DA3d pervious	Runoff Area=197 sf 0.00% Impervious Runoff Depth=0.18" Tc=5.0 min CN=39 Runoff=0.00 cfs 3 cf
Subcatchment DA3di: DA3d impervious	Runoff Area=4,686 sf 100.00% Impervious Runoff Depth=4.66" Tc=5.0 min CN=98 Runoff=0.53 cfs 1,821 cf
Subcatchment R1E: EAST ROOF	Runoff Area=9,274 sf 100.00% Impervious Runoff Depth=4.66" Tc=5.0 min CN=98 Runoff=1.06 cfs 3,604 cf
Subcatchment R1W: WEST ROOF	Runoff Area=9,762 sf 100.00% Impervious Runoff Depth=4.66" Tc=5.0 min CN=98 Runoff=1.11 cfs 3,794 cf
Pond 100: CB 100 12.0" Ro	Peak Elev=50.39' Inflow=0.37 cfs 1,327 cf und Culvert n=0.013 L=4.0' S=0.0050 '/' Outflow=0.37 cfs 1,327 cf

19038-POST V4

Type III 24-hr 10YR Rainfall=4.90"

Prepared by {enter your company name here} HydroCAD® 10.10-7a s/n 01445 © 2021 HydroCAD Software Solutions LLC Printed 12/12/2021 Page 59

Pond 200: CB 200	Peak Elev=52.20' Inflow=0.53 cfs 1,824 cf Primary=0.53 cfs 1,824 cf Secondary=0.00 cfs 0 cf Outflow=0.53 cfs 1,824 cf
Pond BIO1: BIO 1	Peak Elev=59.46' Storage=1,240 cf Inflow=2.22 cfs 7,698 cf Outflow=2.07 cfs 7,698 cf
Pond BIO2: BIO 2	Peak Elev=63.14' Storage=390 cf Inflow=1.13 cfs 3,915 cf Outflow=1.09 cfs 3,915 cf
Pond DMH: DMH 200	Peak Elev=54.62' Inflow=1.20 cfs 4,133 cf Primary=0.91 cfs 3,976 cf Secondary=0.28 cfs 158 cf Outflow=1.20 cfs 4,133 cf
Pond RB1: RB 101,102	Peak Elev=43.39' Storage=318 cf Inflow=0.37 cfs 1,327 cf Discarded=0.06 cfs 1,327 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 1,327 cf
Pond RB2: RB 202,202,203	Peak Elev=47.93' Storage=348 cf Inflow=0.50 cfs 632 cf Discarded=0.09 cfs 632 cf Primary=0.00 cfs 0 cf Outflow=0.09 cfs 632 cf
Pond RB3: RB 300	Peak Elev=58.51' Storage=0 cf Inflow=0.00 cfs 93 cf Discarded=0.00 cfs 93 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 93 cf
Pond SP1: SP1	Inflow=0.07 cfs 1,973 cf Primary=0.07 cfs 1,973 cf
Pond SP2: SP2	Inflow=0.00 cfs 137 cf Primary=0.00 cfs 137 cf
Pond SP3: SP3	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Pond TT1: Tree Trench 1	Peak Elev=54.73' Storage=529 cf Inflow=0.91 cfs 3,976 cf Discarded=0.12 cfs 3,273 cf Primary=0.72 cfs 703 cf Outflow=0.84 cfs 3,976 cf
Pond TT2: Tree Trench 2	Peak Elev=53.05' Storage=156 cf Inflow=0.53 cfs 1,824 cf Discarded=0.03 cfs 1,192 cf Primary=0.50 cfs 632 cf Outflow=0.53 cfs 1,824 cf
Pond URC1: URC-1	Peak Elev=51.29' Storage=0.058 af Inflow=3.03 cfs 8,559 cf Outflow=0.40 cfs 8,559 cf
Pond URC2: URC-2	Peak Elev=57.43' Storage=2,131 cf Inflow=2.13 cfs 7,519 cf Outflow=0.29 cfs 7,519 cf
Pond URC3: URC-3	Peak Elev=58.97' Storage=1,134 cf Inflow=1.11 cfs 3,794 cf Outflow=0.13 cfs 3,794 cf

Total Runoff Area = 346,591 sf Runoff Volume = 28,499 cf Average Runoff Depth = 0.99" 76.39% Pervious = 264,770 sf 23.61% Impervious = 81,821 sf

Summary for Subcatchment DA1: DA1

Runoff = 0.07 cfs @ 13.90 hrs, Volume= 1,973 cf, Depth= 0.15" Routed to Pond SP1 : SP1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

A	rea (sf)	CN E	Description					
1	18,297	30 Woods, Good, HSG A						
	25,363	39 >	39 >75% Grass cover, Good, HSG A					
	9,642	98 F	Roofs, HSG	βA				
	5,338	98 F	Paved park	ing, HSG A				
1	58,640	38 V	Veighted A	verage				
1	43,660	9	0.56% Per	vious Area				
	14,980	9	.44% Impe	ervious Area	a			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
10.6	50	0.1100	0.08		Sheet Flow, A TO B			
					Woods: Dense underbrush n= 0.800 P2= 3.33"			
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C			
					Woodland Kv= 5.0 fps			
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D			
					Short Grass Pasture Kv= 7.0 fps			
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1			
					Woodland Kv= 5.0 fps			
14.5	417	Total						

Subcatchment DA1: DA1



Summary for Subcatchment DA2a: DA2a

CN for permeable pavers taken from RI Stormwater Design

Runoff	=	0.00 cfs @	15.85 hrs,	Volume=	93 cf,	Depth= 0.05"
Routed	l to Pond	RB3 : RB 30	0			

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

	A	rea (sf)	CN	Description					
		12,073	30	Woods, Good, HSG A					
		9,181	39	>75% Grass cover, Good, HSG A					
*		878	40	Pervious Pa	avers				
		22,132	34	Weighted A	verage				
22,132 100.00% Pervious Area			100.00% Pe	ervious Are	а				
,	Tc	Length	Slope	Velocity	Capacity	Description			
(r	nin)	(feet)	(ft/ft) (ft/sec)	(CIS)				
	13.6	50	0.0600	0.06		Sheet Flow, A TO B			
						Woods: Dense underbrush n= 0.800 P2= 3.33"			
	2.5	164	0.0490) 1.11		Shallow Concentrated Flow, B to C			
						Woodland Kv= 5.0 fps			
	1.4	125	0.0480) 1.53		Shallow Concentrated Flow, C to D (RB 3)			
						Short Grass Pasture Kv= 7.0 fps			
	17.5	339	Total						

Subcatchment DA2a: DA2a


Summary for Subcatchment DA2b: DA2b pervious

Runoff = 0.00 cfs @ 12.48 hrs, Volume= Routed to Pond BIO2 : BIO 2 70 cf, Depth= 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"



Summary for Subcatchment DA2bi: DA2b impervious

Runoff = 1.13 cfs @ 12.07 hrs, Volume= 3,845 cf, Depth= 4.66" Routed to Pond BIO2 : BIO 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

Area (sf)	CN	Description					
7,075	98	Paved park	ing, HSG A	N Contraction of the second seco			
2,044	98	Sidewalks,	HŠG A				
775	98	Water Surfa	ace, HSG A				
9,894	98	Weighted Average					
9,894		100.00% In	npervious A	rea			
c Length	Slop	e Velocity	Capacity	Description			
ר) (feet)	(ft/f	:) (ft/sec)	(cfs)				
0				Direct Entry, Direct			
	Area (sf) 7,075 2,044 775 9,894 9,894 5c Length n) (feet) 0	Area (sf) CN 7,075 98 2,044 98 775 98 9,894 98 9,894 98 0,894 98 0 (ft/ft	Area (sf)CNDescription7,07598Paved park2,04498Sidewalks,77598Water Surfa9,89498Weighted A9,894100.00% InTcLengthSlopeN(feet)(ft/ft)00	Area (sf)CNDescription7,07598Paved parking, HSG A2,04498Sidewalks, HSG A77598Water Surface, HSG A9,89498Weighted Average9,894100.00% Impervious ATcLengthSlopeN(feet)(ft/ft)00			

Subcatchment DA2bi: DA2b impervious



Summary for Subcatchment DA2c: DA2c

CN for permeable pavers taken from RI Stormwater Design

Runoff	=	0.00 cfs @	15.90 hrs,	Volume=	137 cf,	Depth= 0.05"
Routed	to Pond	SP2 : SP2				-

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

 A	rea (sf)	CN D	escription		
	18,364	30 V	Voods, Goo	od, HSG A	
	14,084	39 >	75% Grass	s cover, Go	ood, HSG A
	32,448	34 V	Veighted A	verage	
	32,448	1	00.00% Pe	ervious Are	а
Тс	Length	Slope	Velocity	Capacity	Description
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
17.9	50	0.0300	0.05		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
1.2	50	0.0200	0.71		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
2.1	155	0.0320	1.25		Shallow Concentrated Flow, C to SP2
					Short Grass Pasture Kv= 7.0 fps

21.2 255 Total

Subcatchment DA2c: DA2c



Summary for Subcatchment DA3a: DA3a pervious

Runoff = 0.00 cfs @ 15.65 hrs, Volume= Routed to Pond BIO1 : BIO 1 122 cf, Depth= 0.05"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

Area (sf) CN	Description					
12,304	39	>75% Gras	s cover, Go	bod, HSG A			
16,635	5 30	Woods, Go	od, HSG A				
28,939) 34	Weighted A	verage				
28,939)	100.00% Pe	100.00% Pervious Area				
Tc Lengt	h Slop	e Velocity	Capacity	Description			
(min) (fee	t) (ft/1	ft) (ft/sec)	(cfs)				
5.0				Direct Entry, Direct			

Subcatchment DA3a: DA3a pervious



Summary for Subcatchment DA3ai: DA3a impervious

Runoff = 2.22 cfs @ 12.07 hrs, Volume= 7,576 cf, Depth= 4.66" Routed to Pond BIO1 : BIO 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

	Area (sf)	CN	Description					
	16,965	98	Paved park	ing, HSG A	Ν			
*	1,223	98	Sidewalk, H	ISG A				
	1,307	98	Water Surfa	ace, HSG A	N			
	19,495	98	Weighted Average					
	19,495		100.00% Im	npervious A	Area			
To	: Length	Slop	e Velocity	Capacity	Description			
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
5.0	1				Direct Entry, Direct			

Subcatchment DA3ai: DA3a impervious



Summary for Subcatchment DA3b: DA3b pervious

Runoff	=	0.00 cfs @	14.96 hrs,	Volume=
Routed	d to F	ond DMH : DMH	200	

52 cf, Depth= 0.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

A	rea (sf)	CN	Description					
	2,453	39	>75% Gras	s cover, Go	bod, HSG A			
	3,792	30	Woods, Go	od, HSG A				
	387	76	Gravel road	ls, HSG A				
	6,632	36	Weighted A	Weighted Average				
	6,632		100.00% Pe	ervious Are	a			
Тс	Length	Slop	e Velocity	Capacity	Description			
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
5.0					Direct Entry, Direct			

Subcatchment DA3b: DA3b pervious



Summary for Subcatchment DA3bi: DA3b impervious

Runoff	=	1.20 cfs @	12.07 hrs,	Volume=	
Routed	d to Poi	nd DMH : DMH	200		

4,081 cf, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

A	rea (sf)	CN	Description				
	9,746	98	Paved park	ing, HSG A	N		
*	756	98	Sidewalks,	HŠG A			
	10,502	98	Weighted A	verage			
	10,502		100.00% Impervious Area				
Tc (min)	Length (feet)	Slop (ft/ff	e Velocity :) (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry, Direct		

Subcatchment DA3bi: DA3b impervious



Summary for Subcatchment DA3c: DA3c pervious

Runoff = 0.00 cfs @ 17.15 hrs, Volume= Routed to Pond 100 : CB 100 72 cf, Depth= 0.03"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

Ar	rea (sf)	CN	Description				
	10,027	39	>75% Gras	s cover, Go	ood, HSG A		
	16,065	30	Woods, Go	od, HSG A			
	26,092	33	Weighted A	verage			
	26,092		100.00% Pervious Area				
Tc	Length	Slop	e Velocity	Capacity	Description		
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)			
5.0					Direct Entry, Direct		

Subcatchment DA3c: DA3c pervious



Summary for Subcatchment DA3ci: DA3c impervious

Runoff = 0.37 cfs @ 12.07 hrs, Volume= 1,254 cf, Depth= 4.66" Routed to Pond 100 : CB 100

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

Α	rea (sf)	CN	Description						
	2,698	98	Paved park	ing, HSG A					
*	530	98	Sidewalks,	HŠG A					
	3,228	98	Weighted A	Weighted Average					
	3,228		100.00% Impervious Area						
Тс	Length	Slop	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)					
5.0					Direct Entry, Direct				

Subcatchment DA3ci: DA3c impervious



Summary for Subcatchment DA3d: DA3d pervious

Runoff = 0.00 cfs @ 12.48 hrs, Volume= Routed to Pond 200 : CB 200 3 cf, Depth= 0.18"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"



Summary for Subcatchment DA3di: DA3d impervious

Runoff = 0.53 cfs @ 12.07 hrs, Volume= 1,821 cf, Depth= 4.66" Routed to Pond 200 : CB 200

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"

A	Area (sf)	CN D	escription								
4,686 98 Paved parking, HSG A											
	4,686 100.00% Impervious Area										
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
5.0					Direct Entry,						
			Subcat	tchment	DA3di: DA3d impervious						
	Hydrograph										
0.55		0.53 cfs									
0.5			+ - - 	+ - + - + - + - + - + - + - +	······································						
0.45											
0.4					Runoff Volume=1-821-cf						
0.35 وَ					Bunoff Donth=4.66"						
<u>9</u> ≥ ^{0.3}											
음 _{0.25}					i c=5.0 min						
0.2					· · · · · · · · · · · · · · · · · · ·						
0.15											
0.1			- + - +								
0.05											
0	0 2 4 6	8 10 12 14 1	6 18 20 22 24	26 28 30 32 34 Tim	4 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 e (hours)						

Summary for Subcatchment R1E: EAST ROOF

Runoff = 1.06 cfs @ 12.07 hrs, Volume= Routed to Pond URC2 : URC-2 3,604 cf, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"



Summary for Subcatchment R1W: WEST ROOF

Runoff = 1.11 cfs @ 12.07 hrs, Volume= Routed to Pond URC3 : URC-3 3,794 cf, Depth= 4.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 10YR Rainfall=4.90"



Summary for Pond 100: CB 100

[57] Hint: Peaked at 50.39' (Flood elevation advised)

Inflow Are	a =	29,320 sf,	11.01% Impervious	, Inflow Depth = 0.54"	for 10YR event
Inflow	=	0.37 cfs @	12.07 hrs, Volume=	1,327 cf	
Outflow	=	0.37 cfs @	12.07 hrs, Volume=	1,327 cf, Atte	en= 0%, Lag= 0.0 min
Primary	=	0.37 cfs @	12.07 hrs, Volume=	1,327 cf	-
Routed	l to Pond	RB1 : RB 10	1,102		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 50.39' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.00'	12.0" Round Culvert L= 4.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 50.00' / 49.98' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.37 cfs @ 12.07 hrs HW=50.39' (Free Discharge) **1=Culvert** (Barrel Controls 0.37 cfs @ 1.90 fps)

Pond 100: CB 100



Summary for Pond 200: CB 200

[57] Hint: Peaked at 52.20' (Flood elevation advised)

Inflow Area	a =	4,883 sf,	95.97% In	npervious,	Inflow Depth = 4.48 "	for 10YR event
Inflow	=	0.53 cfs @	12.07 hrs,	Volume=	1,824 cf	
Outflow	=	0.53 cfs @	12.07 hrs,	Volume=	1,824 cf, Atte	en= 0%, Lag= 0.0 min
Primary	=	0.53 cfs @	12.07 hrs,	Volume=	1,824 cf	-
Routed	to Pond	TT2 : Tree T	rench 2			
Secondary	/ =	0.00 cfs @	0.00 hrs,	Volume=	0 cf	
Routed	to Pond	RB2 : RB 20	2,202,203			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 52.20' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	52.70'	12.0" Round Culvert L= 4.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 52.70' / 52.68' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Primary	51.66'	8.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.66' / 51.41' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Secondary	55.79'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.53 cfs @ 12.07 hrs HW=52.20' (Free Discharge) ←2=Culvert (Barrel Controls 0.53 cfs @ 2.42 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=51.66' (Free Discharge) -1=Culvert (Controls 0.00 cfs) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 200: CB 200



Summary for Pond BIO1: BIO 1

Inflow Ar Inflow Outflow Primary Route	rea = = 2 = 2 = 2 ed to Pond U	48,434 sf, 40. .22 cfs @ 12.0 .07 cfs @ 12.1 .07 cfs @ 12.1 RC1 : URC-1	25% Imperviou 7 hrs, Volume 0 hrs, Volume 0 hrs, Volume	s, Inflow Dep = 7,6 = 7,6 = 7,6	th = 1.91" for 10YR event 598 cf 598 cf, Atten= 7%, Lag= 1.8 min 598 cf		
Routing Peak Ele	by Stor-Ind r ev= 59.46' @	nethod, Time Sp 212.10 hrs Sur	oan= 0.00-72.00 f.Area= 1,642 s	0 hrs, dt= 0.01 of Storage= 1	hrs ,240 cf		
Plug-Flo Center-o	w detention f of-Mass det. 1	time= 62.5 min c time= 62.5 min (alculated for 7 816.1 - 753.5	,697 cf (100%)	of inflow)		
Volume	Invert	Avail.Storag	e Storage De	escription			
#1	58.50'	2,210	cf Custom S	tage Data (Pr	ismatic)Listed below (Recalc)		
Elevatio (fee	on Su t)	ırf.Area (sq-ft) (c	Inc.Store ubic-feet)	Cum.Store (cubic-feet)			
58.5	50	800	0	0			
59.0	00	1,380	545	545			
60.0	00	1,950	1,665	2,210			
Device	Routing	Invert C	outlet Devices				
#1	Primary	55.09' 1 L Ir n	2.0" Round C = 45.0' CPP, nlet / Outlet Inve = 0.013 Corrue	ulvert projecting, no ert= 55.09' / 5 gated PE, smo	headwall, Ke= 0.900 4.87' S= 0.0049 '/' Cc= 0.900 poth interior. Flow Area= 0.79 sf		
#2	Device 1	59.25' 2 L	24.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads				
#3	Device 1	55.38' 4 L Ir n	 3' 4.0" Round Culvert L= 38.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 55.38' / 55.19' S= 0.0050 '/' C n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf 		headwall, Ke= 0.900 5.19' S= 0.0050 '/' Cc= 0.900 or, Flow Area= 0.09 sf		
#4	Device 3	58.50' 2	.470 in/hr Exfi	Itration over	Surface area		
Primary	OutFlow M Ivert (Passe	ax=2.07 cfs @ 1 es 2.07 cfs of 5.8	2.10 hrs HW= 7 cfs potential	59.46' (Free flow)	Discharge)		

-2=Orifice/Grate (Weir Controls 1.97 cfs @ 1.50 fps) -3=Culvert (Passes 0.09 cfs of 0.62 cfs potential flow) -4=Exfiltration (Exfiltration Controls 0.09 cfs)

Pond BIO1: BIO 1



Summary for Pond BIO2: BIO 2

Inflow An Inflow Outflow Primary Route	rea = = 2 = 2 ed to Pond U	14,564 sf, 67 1.13 cfs @ 12.0 1.09 cfs @ 12.0 1.09 cfs @ 12.0 JRC2 : URC-2	93% Imperviou)7 hrs, Volume)9 hrs, Volume)9 hrs, Volume	s, Inflow Depth = = 3,915 = 3,915 = 3,915	3.23" for 10YR event cf cf, Atten= 3%, Lag= 1.1 min cf
Routing Peak Ele	by Stor-Ind ev= 63.14' @	method, Time S 0 12.09 hrs Su	pan= 0.00-72.0 f.Area= 797 sf	0 hrs, dt= 0.01 hrs Storage= 390 cf	3
Plug-Flo Center-c	w detention of-Mass det.	time= 38.8 min time= 38.8 min	calculated for 3 (791.1 - 752.3	,915 cf (100% of i)	nflow)
Volume	Invert	Avail.Stora	ge Storage De	escription	
#1	62.50'	1,340	cf Custom S	tage Data (Prism	natic)Listed below (Recalc)
Elevatio (fee	on S et)	urf.Area (sq-ft) (c	Inc.Store subic-feet)	Cum.Store (cubic-feet)	
62.5	50	450	0		
63.0	00	700	288	288	
64.0	00	1,405	1,053	1,340	
Device	Routing	Invert (Dutlet Devices		
#1	Primary	59.00' 1 L	2.0" Round C = 25.0' CPP, nlet / Outlet Invo = 0.013 Corru	ulvert end-section confo ert= 59.00' / 58.8 gated PE, smooth	orming to fill, Ke= 0.500 3' S= 0.0048 '/' Cc= 0.900 0 interior, Flow Area= 0.79 sf
#2	Device 1	63.00' 2	24.0" Horiz. Ori	fice/Grate C= 0	.600
#3	Device 1	59.30' 4 L	 A.0" Round Culvert L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 59.30' / 59.18' S= 0.0048 '/' Cc= 0. n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf 		adwall, Ke= 0.900 3' S= 0.0048 '/' Cc= 0.900 Flow Area= 0.09 sf
#4	Device 3	62.50'	2.470 in/hr Exfi	Itration over Sur	face area
Primary OutFlow Max=1.09 cfs @ 12.09 hrs HW=63.14' (Free Discharge)					

-2=Orifice/Grate (Weir Controls 1.05 cfs @ 1.21 fps) -3=Culvert (Passes 0.05 cfs of 0.64 cfs potential flow) -4=Exfiltration (Exfiltration Controls 0.05 cfs)

Pond BIO2: BIO 2



Summary for Pond DMH: DMH 200

[57] Hint: Peaked at 54.62' (Flood elevation advised)

Inflow Area	a =	17,134 sf,	61.29% In	npervious,	Inflow Depth =	2.89"	for 10YR event	
Inflow	=	1.20 cfs @	12.07 hrs,	Volume=	4,133 c	f		
Outflow	=	1.20 cfs @	12.07 hrs,	Volume=	4,133 ct	f, Atten	= 0%, Lag= 0.0 min	
Primary	=	0.91 cfs @	12.07 hrs,	Volume=	3,976 ct	f	-	
Routed	Routed to Pond TT1 : Tree Trench 1							
Secondary		0.28 cfs @	12.07 hrs,	Volume=	158 ct	f		
Routed	to Pond	URC1 : URC	2-1					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 54.62' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	54.30'	12.0" Round Culvert L= 9.0' CMP. square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 54.30' / 54.26' S= 0.0044 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Primary	53.78'	12.0" Round Culvert L= 98.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 53.78' / 53.78' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.91 cfs @ 12.07 hrs HW=54.62' (Free Discharge) ←2=Culvert (Barrel Controls 0.91 cfs @ 1.74 fps)

Secondary OutFlow Max=0.28 cfs @ 12.07 hrs HW=54.62' (Free Discharge) -1=Culvert (Barrel Controls 0.28 cfs @ 1.93 fps) Pond DMH: DMH 200



Summary for Pond RB1: RB 101,102

Inflow Ar	rea =	29,320 sf, 1´	1.01%	impervious, Inflow Depth = 0.54" for 10YR event
Inflow	= 0.3	37 cfs @ 12	.07 hı	rs, Volume= 1,327 cf
Outflow	= 0.0)6 cfs @ 11	.65 hi	rs, Volume= 1,327 cf, Atten= 84%, Lag= 0.0 min
Discarde	ed = 0.0)6 cfs @ 11	.65 hi	rs, Volume= 1,327 cf
Primarv	= 0.0)0 cfs @ 0	.00 hi	rs. Volume= 0 cf
Route	ed to Pond SP	3 : SP3		
Routina	by Stor-Ind me	ethod. Time S	Span=	= 0.00-72.00 hrs. dt= 0.01 hrs / 3
Peak Ele	ev= 43.39' @ 1	12.53 hrs Si	urf.Ar	ea = 157 sf Storage = 318 cf
Plug-Flo	w detention tir	ne= 26 7 min	n calc	ulated for 1 327 cf (100% of inflow)
Center-c	of-Mass det. tir	ne= 26.6 mir	n (79	7.1 - 770.5)
••••••				
Volume	Invert	Avail.Stora	age	Storage Description
#1	41.00'	33	9 cf	6.00'D x 6.00'H Recharger x 2 Inside #2
#2	39.00'	35	5 cf	10.00'D x 9.00'H Stone x 2
				1,414 cf Overall - 339 cf Embedded = 1,074 cf x 33.0% Voids
		694	4 cf	Total Available Storage
				ů –
Device	Routing	Invert	Outle	et Devices
#1	Discarded	39.00'	8.270	0 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01
#2	Primary	46.50'	5.0' I	ong x 5.0' breadth Broad-Crested Rectangular Weir X 2.00
	,		Head	d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50	3.00 3.50 4.00 4.50 5.00 5.50
			Coef	(English) 234 250 270 268 268 266 265 265 265
			2 65	267 266 268 270 274 279 288
			2.00	
Discard	ed OutFlow	/lax=0.06 cfs	@1	1.65 hrs HW=39.09' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=39.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Pond RB1: RB 101,102



Time (hours)

Summary for Pond RB2: RB 202,202,203

[92] Warning: Device #2 is above defined storage

Inflow Area	a =	4,883 sf,	95.97% In	npervious,	Inflow Depth =	1.55"	for 10	YR event
Inflow	=	0.50 cfs @	12.08 hrs,	Volume=	632 c	f		
Outflow	=	0.09 cfs @	11.88 hrs,	Volume=	632 c	f, Atten	i= 82%,	Lag= 0.0 min
Discarded	=	0.09 cfs @	11.88 hrs,	Volume=	632 c	f		
Primary	=	0.00 cfs @	0.00 hrs,	Volume=	0 c	f		
Routed	to Pond	SP3 : SP3						

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 47.93' @ 12.47 hrs Surf.Area= 236 sf Storage= 348 cf

Plug-Flow detention time= 36.9 min calculated for 632 cf (100% of inflow) Center-of-Mass det. time= 36.9 min (769.8 - 732.9)

Volume	Invert	Avail.Stora	age Storage Description
#1	46.50'	509	9 cf 6.00'D x 6.00'H Recharger x 3 Inside #2
#2	44.50'	532	2 cf 10.00'D x 9.00'H Stone x 3
			2,121 cf Overall - 509 cf Embedded = 1,612 cf x 33.0% Voids
		1,041	1 cf Total Available Storage
Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary	44.50' 55.61'	8.270 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01' 5.0' long x 5.0' breadth Broad-Crested Rectangular Weir X 2.00 Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Discarded OutFlow Max=0.09 cfs @ 11.88 hrs HW=44.64' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=44.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Hydrograph InflowOutflow 0.50 cfs Inflow Area=4,883 sf Discarded Primary 0.55 Peak Elev=47.93' 0.5 Storage=348 cf 0.45 0.4 0.35 Flow (cfs) 0.3 0.25 0.2 0.09.cfs 0.09.cfs 0.15 0.1 0.00 cfs 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72

Pond RB2: RB 202,202,203

Time (hours)

Summary for Pond RB3: RB 300

Inflow Ar	ea =	22,132 sf, 0.0	0% Impervious, Inflow Depth = 0.05" for 10YR event
Inflow	= 0.0	0 cfs @ 15.8	5 hrs, Volume= 93 cf
Outflow	= 0.0)0 cfs @15.8	Shrs, Volume= 93 cf, Atten= 0%, Lag= 0.5 min
Discarde	ed = 0.0)0 cfs @15.8	Shrs, Volume= 93 cf
Primarv	= 0.0)0 cfs @ 0.0) hrs. Volume= 0 cf
Route	ed to Pond SP	2 : SP2	·····, ······· ····· ·····
Routing b	by Stor-Ind me	ethod Time Sp	an= 0 00-72 00 hrs_dt= 0 01 hrs / 3
Peak Fle	ev= 58 51' @ 1	15.86 hrs Surf	Area= 57 sf Storage= 0 cf
Plug-Flov	w detention tir	ne= 1 1 min ca	culated for 93 cf (100% of inflow)
Center_0	f_Mass dat tir	ne= 1.1 min (1	1/1/7 - 1/1/3 = 0
Ochici-0			,144.7 - 1,143.0)
Volume	Invert	Avail Storad	e Storage Description
#1	50 50'	<u> </u>	of 4 00'D x 4 00'H Pochargor Inside #2
#1 #2	58 50'	133	$f = 6.00 \text{ D x } 4.00 \text{ H Recharger mode } \pi^2$
#2	50.50	155 ($452 \text{ of } 0.00 \text{ D X 0.00 H Stolle } \times 2$
		183 (t Total Available Storage
Device	Routing	Invert O	utlet Devices
#1	Discarded	58.50' 8 .	270 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01'
#2	Primary	65.50' 5 .	0' long x 5.0' breadth Broad-Crested Rectangular Weir
	-	Н	ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
		2.	50 3.00 3.50 4.00 4.50 5.00 5.50
		C	pef (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65
		2	65 2 67 2 66 2 68 2 70 2 74 2 79 2 88
		£.	
Discarde		/ax=0.02 cfs @	15.86 hrs HW=58.51' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=58.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Hydrograph Inflow 0.00 cfs Outflow 0.00.cfs Inflow Area=22,132 sf Discarded 0.00 cfs Primary 0.003 Peak Elev=58.51' 0.003 0.003 Storage=0 cf 0.003 0.003 0.002 0.002 (cfs) 0.002 0.002 Flow 0.002 0.001 0.001 0.001 0.001 0.001 0.000 0.00 cfs

Pond RB3: RB 300

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area	=	158,640 sf,	9.44% In	npervious,	Inflow Depth =	0.15"	for 10	YR event
Inflow	=	=	0.07 cfs @	13.90 hrs,	Volume=	1,973 c	f		
Primary	/ =	=	0.07 cfs @	13.90 hrs,	Volume=	1,973 c	f, Atter	ר= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area	=	54,580 sf,	0.00% In	npervious,	Inflow Depth =	0.03"	for 10	YR event
Inflow		=	0.00 cfs @	15.90 hrs,	Volume=	137 c	f		
Primar	У	=	0.00 cfs @	15.90 hrs,	Volume=	137 c	f, Atte	n= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area =	=	34,203 sf,	23.14% Impervious,	Inflow Depth = 0.00"	for 10YR event
Inflow	=		0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	/ =		0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP3: SP3

Summary for Pond TT1: Tree Trench 1

[81] Warning: Exceeded Pond DMH by 0.29' @ 12.56 hrs

Inflow Area	a =	17,134 sf,	61.29% In	npervious,	Inflow Depth =	2.78"	for 10Y	R event
Inflow	=	0.91 cfs @	12.07 hrs,	Volume=	3,976 cf			
Outflow	=	0.84 cfs @	12.11 hrs,	Volume=	3,976 cf	, Atten	= 8%, La	ag= 2.3 min
Discarded	=	0.12 cfs @	11.44 hrs,	Volume=	3,273 cf			
Primary	=	0.72 cfs @	12.11 hrs,	Volume=	703 cf			
Routed	to Pond	URC1 : URC	-1					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 54.73' @ 12.11 hrs Surf.Area= 639 sf Storage= 529 cf

Plug-Flow detention time= 15.7 min calculated for 3,975 cf (100% of inflow) Center-of-Mass det. time= 15.7 min (768.4 - 752.6)

Volume	Invert	Avail.Stor	rage	Storage Description			
#1	52.34'	62	27 cf	6.20'W x 103.00'L x 3.00'H Prismatoid			
				1,916 cf Overall - 32 cf Embedded = 1,884 cf x 33.3% Voids			
#2	53.78'	3	32 cf	8.0" Round Pipe Storage Inside #1			
				L= 92.0° S= 0.0050 7°			
		65	59 cf	Total Available Storage			
Device	Routing	Invert	Outle	et Devices			
#1	Discarded	52.34'	8.27	0 in/hr Exfiltration over Surface area Phase-In= 0.01'			
#2	Primary	54.30'	12.0'	"Vert. Orifice/Grate C= 0.600			
	-		Limit	ed to weir flow at low heads			
Discarded OutFlow Max=0.12 cfs @ 11.44 hrs HW=52.37' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.12 cfs)							

Primary OutFlow Max=0.72 cfs @ 12.11 hrs HW=54.73' (Free Discharge) ←2=Orifice/Grate (Orifice Controls 0.72 cfs @ 2.23 fps)

Pond TT1: Tree Trench 1



Summary for Pond TT2: Tree Trench 2

[81] Warning: Exceeded Pond 200 by 0.97' @ 12.55 hrs

Inflow Area	ı =	4,883 sf,	95.97% In	npervious,	Inflow Depth = 4.48	for 10YR event
Inflow	=	0.53 cfs @	12.07 hrs,	Volume=	1,824 cf	
Outflow	=	0.53 cfs @	12.08 hrs,	Volume=	1,824 cf, Att	ten= 0%, Lag= 0.3 min
Discarded	=	0.03 cfs @	10.38 hrs,	Volume=	1,192 cf	-
Primary	=	0.50 cfs @	12.08 hrs,	Volume=	632 cf	
Routed	to Pond	RB2 : RB 20	2,202,203			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 53.05' @ 12.08 hrs Surf.Area= 150 sf Storage= 156 cf

Plug-Flow detention time= 21.3 min calculated for 1,824 cf (100% of inflow) Center-of-Mass det. time= 21.3 min (769.2 - 747.9)

Volume	Invert	Avail.Storage	e Storage Description				
#1	50.16'	184 c	f 5.00'W x 30.00'L x 3.80'H Prismatoid				
#2	51.66'	17 c	570 cf Overall - 17 cf Embedded = 553 cf x 33.3% Voids 8.0" Round Pipe Storage Inside #1 L= 50.0' S= 0.0050 '/'				
		201 c	f Total Available Storage				
Device	Routing	Invert Ou	itlet Devices				
#1	Discarded	50.16' 8.2	270 in/hr Exfiltration over Surface area Phase-In= 0.01'				
#2	Primary	52.70' 12 Lir	.0" Vert. Orifice/Grate C= 0.600 nited to weir flow at low heads				
Discarded OutFlow Max=0.03 cfs @ 10.38 hrs HW=50.20' (Free Discharge)							

Primary OutFlow Max=0.50 cfs @ 12.08 hrs HW=53.05' (Free Discharge) ←2=Orifice/Grate (Orifice Controls 0.50 cfs @ 2.02 fps) Pond TT2: Tree Trench 2



Summary for Pond URC1: URC-1

Inflow Area	a =	65,568 sf,	45.75% Impervious,	Inflow Depth = 1	.57" for 10YR event
Inflow	=	3.03 cfs @	12.10 hrs, Volume=	8,559 cf	
Outflow	=	0.40 cfs @	11.86 hrs, Volume=	8,559 cf,	Atten= 87%, Lag= 0.0 min
Discarded	=	0.40 cfs @	11.86 hrs, Volume=	8,559 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 51.29' @ 12.60 hrs Surf.Area= 0.048 ac Storage= 0.058 af

Plug-Flow detention time= 35.5 min calculated for 8,558 cf (100% of inflow) Center-of-Mass det. time= 35.5 min (843.3 - 807.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	48.60'	0.087 af	16.33'W x 128.59'L x 7.25'H Field A
			0.350 af Overall - 0.087 af Embedded = 0.262 af x 33.3% Voids
#2A	50.60'	0.087 af	ADS_StormTech MC-3500 d +Cap x 34 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			34 Chambers in 2 Rows
			Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf
		0.175 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	48.60'	8.270 in/hr Exfiltration over Surface area	Phase-In= 0.01'

Discarded OutFlow Max=0.40 cfs @ 11.86 hrs HW=48.68' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.40 cfs)
Pond URC1: URC-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cfOverall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

17 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 125.59' Row Length +18.0" End Stone x 2 = 128.59' Base Length 2 Rows x 77.0" Wide + 6.0" Spacing x 1 + 18.0" Side Stone x 2 = 16.33' Base Width 24.0" Stone Base + 45.0" Chamber Height + 18.0" Stone Cover = 7.25' Field Height

34 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 2 Rows = 3,798.0 cf Chamber Storage

15,227.2 cf Field - 3,798.0 cf Chambers = 11,429.2 cf Stone x 33.3% Voids = 3,805.9 cf Stone Storage

Chamber Storage + Stone Storage = 7,603.9 cf = 0.175 af Overall Storage Efficiency = 49.9% Overall System Size = 128.59' x 16.33' x 7.25'

34 Chambers 564.0 cy Field 423.3 cy Stone





Pond URC1: URC-1



Summary for Pond URC2: URC-2

Inflow Area	=	23,838 sf,	80.41% Imper	rvious, Infl	low Depth =	3.79" 1	for 10Y	R event
Inflow	=	2.13 cfs @	12.08 hrs, Vol	ume=	7,519 ct	f		
Outflow	=	0.29 cfs @	11.63 hrs, Vol	ume=	7,519 ct	f, Atten=	• 87% ,	Lag= 0.0 min
Discarded	=	0.29 cfs @	11.63 hrs, Vol	ume=	7,519 ct	f		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 57.43' @ 12.60 hrs Surf.Area= 1,499 sf Storage= 2,131 cf

Plug-Flow detention time= 43.7 min calculated for 7,518 cf (100% of inflow) Center-of-Mass det. time= 43.7 min (813.9 - 770.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	54.50'	2,405 cf	36.08'W x 41.55'L x 6.75'H Field A
			10,120 cf Overall - 2,898 cf Embedded = 7,222 cf x 33.3% Voids
#2A	56.50'	2,898 cf	ADS_StormTech MC-3500 d +Cap x 25 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			25 Chambers in 5 Rows
			Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf
		5,303 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	54.50'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'	
Discard	led OutFlow	Max=0.29 cfs	@ 11.63 hrs HW=54.57' (Free Discharge)	

1=Exfiltration (Exfiltration Controls 0.29 cfs)

Pond URC2: URC-2 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

5 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 39.55' Row Length +12.0" End Stone x 2 = 41.55'Base Length 5 Rows x 77.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 36.08' Base Width 24.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

25 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 5 Rows = 2,897.8 cf Chamber Storage

10,120.0 cf Field - 2,897.8 cf Chambers = 7,222.2 cf Stone x 33.3% Voids = 2,405.0 cf Stone Storage

Chamber Storage + Stone Storage = 5,302.8 cf = 0.122 af Overall Storage Efficiency = 52.4%Overall System Size = $41.55' \times 36.08' \times 6.75'$

25 Chambers 374.8 cy Field 267.5 cy Stone





Pond URC2: URC-2



Summary for Pond URC3: URC-3

Inflow Area	a =	9,762 sf,	100.00% Impervious,	Inflow Depth = 4.66	6" for 10YR event
Inflow	=	1.11 cfs @	12.07 hrs, Volume=	3,794 cf	
Outflow	=	0.13 cfs @	11.59 hrs, Volume=	3,794 cf, At	ten= 88%, Lag= 0.0 min
Discarded	=	0.13 cfs @	11.59 hrs, Volume=	3,794 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 58.97' @ 12.61 hrs Surf.Area= 823 sf Storage= 1,134 cf

Plug-Flow detention time= 51.7 min calculated for 3,794 cf (100% of inflow) Center-of-Mass det. time= 51.7 min (799.1 - 747.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	56.30'	1,243 cf	23.25'W x 35.38'L x 6.25'H Field A
			5,141 cf Overall - 1,409 cf Embedded = 3,732 cf x 33.3% Voids
#2A	57.80'	1,409 cf	ADS_StormTech MC-3500 d +Cap x 12 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			12 Chambers in 3 Rows
			Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf
		2.652 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	56.30'	7.000 in/hr Exfiltration over Surface area	Phase-In= 0.01'
Discard	led OutFlow	Max=0.13 cfs	@ 11.59 hrs HW=56.36' (Free Discharge)	

1=Exfiltration (Exfiltration Controls 0.13 cfs)

Pond URC3: URC-3 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cfOverall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

4 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 32.38' Row Length +18.0" End Stone x 2 = 35.38' Base Length 3 Rows x 77.0" Wide + 6.0" Spacing x 2 + 18.0" Side Stone x 2 = 23.25' Base Width 18.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.25' Field Height

12 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 3 Rows = 1,408.8 cf Chamber Storage

5,141.2 cf Field - 1,408.8 cf Chambers = 3,732.3 cf Stone x 33.3% Voids = 1,242.9 cf Stone Storage

Chamber Storage + Stone Storage = 2,651.7 cf = 0.061 af Overall Storage Efficiency = 51.6%Overall System Size = $35.38' \times 23.25' \times 6.25'$

12 Chambers 190.4 cy Field 138.2 cy Stone





Pond URC3: URC-3



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.43" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.53 cfs 5,628 cf
Subcatchment DA2a: DA2a	Runoff Area=22,132 sf 0.00% Impervious Runoff Depth=0.23" Flow Length=339' Tc=17.5 min CN=34 Runoff=0.02 cfs 427 cf
Subcatchment DA2b: DA2b pervious	Runoff Area=4,670 sf 0.00% Impervious Runoff Depth=0.48" Tc=5.0 min CN=39 Runoff=0.02 cfs 187 cf
Subcatchment DA2bi: DA2b impervious	Runoff Area=9,894 sf 100.00% Impervious Runoff Depth=5.88" Tc=5.0 min CN=98 Runoff=1.41 cfs 4,849 cf
Subcatchment DA2c: DA2c	Runoff Area=32,448 sf 0.00% Impervious Runoff Depth=0.23" Flow Length=255' Tc=21.2 min CN=34 Runoff=0.02 cfs 625 cf
Subcatchment DA3a: DA3a pervious	Runoff Area=28,939 sf 0.00% Impervious Runoff Depth=0.23" Tc=5.0 min CN=34 Runoff=0.03 cfs 558 cf
Subcatchment DA3ai: DA3a impervious	Runoff Area=19,495 sf 100.00% Impervious Runoff Depth=5.88" Tc=5.0 min CN=98 Runoff=2.78 cfs 9,555 cf
Subcatchment DA3b: DA3b pervious	Runoff Area=6,632 sf 0.00% Impervious Runoff Depth=0.32" Tc=5.0 min CN=36 Runoff=0.01 cfs 179 cf
Subcatchment DA3bi: DA3b impervious	Runoff Area=10,502 sf 100.00% Impervious Runoff Depth=5.88" Tc=5.0 min CN=98 Runoff=1.50 cfs 5,147 cf
Subcatchment DA3c: DA3c pervious	Runoff Area=26,092 sf 0.00% Impervious Runoff Depth=0.19" Tc=5.0 min CN=33 Runoff=0.02 cfs 412 cf
Subcatchment DA3ci: DA3c impervious	Runoff Area=3,228 sf 100.00% Impervious Runoff Depth=5.88" Tc=5.0 min CN=98 Runoff=0.46 cfs 1,582 cf
Subcatchment DA3d: DA3d pervious	Runoff Area=197 sf 0.00% Impervious Runoff Depth=0.48" Tc=5.0 min CN=39 Runoff=0.00 cfs 8 cf
Subcatchment DA3di: DA3d impervious	Runoff Area=4,686 sf 100.00% Impervious Runoff Depth=5.88" Tc=5.0 min CN=98 Runoff=0.67 cfs 2,297 cf
Subcatchment R1E: EAST ROOF	Runoff Area=9,274 sf 100.00% Impervious Runoff Depth=5.88" Tc=5.0 min CN=98 Runoff=1.32 cfs 4,546 cf
Subcatchment R1W: WEST ROOF	Runoff Area=9,762 sf 100.00% Impervious Runoff Depth=5.88" Tc=5.0 min CN=98 Runoff=1.39 cfs 4,785 cf
Pond 100: CB 100 12.0" Ro	Peak Elev=50.44' Inflow=0.46 cfs 1,995 cf und Culvert n=0.013 L=4.0' S=0.0050 '/' Outflow=0.46 cfs 1,995 cf

19038-POST V4 Type III 24-hr 25YR Rainfall=6.12" Printed 12/12/2021 Prepared by {enter your company name here} HydroCAD® 10.10-7a s/n 01445 © 2021 HydroCAD Software Solutions LLC Page 108 Peak Elev=52.29' Inflow=0.67 cfs 2,305 cf Pond 200: CB 200 Primary=0.67 cfs 2,305 cf Secondary=0.00 cfs 0 cf Outflow=0.67 cfs 2,305 cf Pond BIO1: BIO 1 Peak Elev=59.50' Storage=1,300 cf Inflow=2.78 cfs 10,113 cf Outflow=2.61 cfs 10,113 cf Peak Elev=63.16' Storage=409 cf Inflow=1.41 cfs 5,036 cf Pond BIO2: BIO 2 Outflow=1.38 cfs 5,036 cf Pond DMH: DMH 200 Peak Elev=54.70' Inflow=1.50 cfs 5.326 cf Primary=1.07 cfs 5,046 cf Secondary=0.43 cfs 280 cf Outflow=1.50 cfs 5,326 cf Peak Elev=44.93' Storage=457 cf Inflow=0.46 cfs 1,995 cf Pond RB1: RB 101,102 Discarded=0.06 cfs 1,995 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 1,995 cf Peak Elev=49.38' Storage=543 cf Inflow=0.64 cfs 936 cf Pond RB2: RB 202,202,203 Discarded=0.09 cfs 936 cf Primary=0.00 cfs 0 cf Outflow=0.09 cfs 936 cf

Pond RB3: RB 300Peak Elev=58.56' Storage=1 cf Inflow=0.02 cfs 427 cfDiscarded=0.02 cfs 427 cfPrimary=0.00 cfs 0 cf Outflow=0.02 cfs 427 cf

 Pond SP1: SP1
 Inflow=0.53 cfs
 5,628 cf

 Primary=0.53 cfs
 5,628 cf

 Pond SP2: SP2
 Inflow=0.02 cfs
 625 cf

Pond SP3: SP3

Primary=0.02 cfs 625 cf

Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf

 Pond TT1: Tree Trench 1
 Peak Elev=54.79' Storage=543 cf
 Inflow=1.07 cfs
 5,046 cf

 Discarded=0.12 cfs
 3,937 cf
 Primary=0.92 cfs
 1,109 cf
 Outflow=1.04 cfs
 5,046 cf

 Pond TT2: Tree Trench 2
 Peak Elev=53.10' Storage=159 cf
 Inflow=0.67 cfs
 2,305 cf

 Discarded=0.03 cfs
 1,369 cf
 Primary=0.64 cfs
 936 cf
 Outflow=0.67 cfs
 2,305 cf

 Pond URC1: URC-1
 Peak Elev=52.18' Storage=0.090 af
 Inflow=3.92 cfs
 11,503 cf

 Pond URC2: URC-2
 Peak Elev=58.17' Storage=2,982 cf
 Inflow=2.68 cfs
 9,582 cf

 Pond URC3: URC-3
 Peak Elev=59.72' Storage=1,564 cf
 Inflow=1.39 cfs
 4,785 cf

Total Runoff Area = 346,591 sf Runoff Volume = 40,785 cf Average Runoff Depth = 1.41" 76.39% Pervious = 264,770 sf 23.61% Impervious = 81,821 sf

Summary for Subcatchment DA1: DA1

Runoff = 0.53 cfs @ 12.49 hrs, Volume= 5,628 cf, Depth= 0.43" Routed to Pond SP1 : SP1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

A	rea (sf)	CN E	Description						
1	18,297	30 V	30 Woods, Good, HSG A						
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A				
	9,642	98 F	Roofs, HSG	βA					
	5,338	98 F	Paved park	ing, HSG A					
1	58,640	38 V	Veighted A	verage					
1	43,660	g	0.56% Per	vious Area					
	14,980	g	.44% Impe	ervious Area	a				
			-						
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
10.6	50	0.1100	0.08		Sheet Flow, A TO B				
					Woods: Dense underbrush n= 0.800 P2= 3.33"				
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C				
					Woodland Kv= 5.0 fps				
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D				
					Short Grass Pasture Kv= 7.0 fps				
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1				
					Woodland Kv= 5.0 fps				
14.5	417	Total							

Subcatchment DA1: DA1



Summary for Subcatchment DA2a: DA2a

CN for permeable pavers taken from RI Stormwater Design

Runoff = 0.02 cfs @ 13.01 hrs, Volume= 427 cf, Depth= 0.23" Routed to Pond RB3 : RB 300

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

	A	rea (sf)	CN I	Description						
		12,073 9,181	30 \ 39 ;	Noods, Good, HSG A >75% Grass cover, Good, HSG A						
*		878	40 I	Pervious Pa	avers					
		22,132	34 \	Neighted A	verage					
		22,132		100.00% Pe	ervious Are	а				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	13.6	50	0.0600	0.06		Sheet Flow, A TO B				
						Woods: Dense underbrush n= 0.800 P2= 3.33"				
	2.5	164	0.0490	1.11		Shallow Concentrated Flow, B to C				
		405	0.0400	4 50		Woodland Kv= 5.0 fps				
	1.4	125	0.0480	1.53		Sharlow Concentrated Flow, C to D (RB 3) Short Grass Pasture Ky= 7.0 fps				
	17.5	339	Total							

Subcatchment DA2a: DA2a



Summary for Subcatchment DA2b: DA2b pervious

Runoff = 0.02 cfs @ 12.32 hrs, Volume= Routed to Pond BIO2 : BIO 2 187 cf, Depth= 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"



Summary for Subcatchment DA2bi: DA2b impervious

Runoff = 1.41 cfs @ 12.07 hrs, Volume= 4,849 cf, Depth= 5.88" Routed to Pond BIO2 : BIO 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

	Area (sf)	CN	Description						
	7,075	98	Paved park	ing, HSG A	N Contraction of the second seco				
*	2,044	98	Sidewalks,	Sidewalks, HŠG A					
	775	98	Water Surfa	ace, HSG A	l l				
	9,894	98	Weighted A	verage					
	9,894		100.00% In	100.00% Impervious Area					
To (min)	c Length) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	Description				
5.0)				Direct Entry, Direct				

Subcatchment DA2bi: DA2b impervious





Summary for Subcatchment DA2c: DA2c

CN for permeable pavers taken from RI Stormwater Design

Runoff	=	0.02 cfs @	13.07 hrs,	Volume=	625 cf,	Depth= 0.23"	
Routed	to Pond	SP2 : SP2					

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

A	rea (sf)	CN [Description		
	18,364	30 V	Voods, Go	od, HSG A	
	14,084	39 >	>75% Gras	s cover, Go	DOD, HSG A
	32,448	34 V	Veighted A	verage	
	32,448	1	00.00% Pe	ervious Are	а
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
17.9	50	0.0300	0.05		Sheet Flow, A TO B
					Woods: Dense underbrush n= 0.800 P2= 3.33"
1.2	50	0.0200	0.71		Shallow Concentrated Flow, B to C
					Woodland Kv= 5.0 fps
2.1	155	0.0320	1.25		Shallow Concentrated Flow, C to SP2
					Short Grass Pasture Kv= 7.0 fps

21.2 255 Total

Subcatchment DA2c: DA2c



Summary for Subcatchment DA3a: DA3a pervious

Runoff = 0.03 cfs @ 12.47 hrs, Volume= Routed to Pond BIO1 : BIO 1 558 cf, Depth= 0.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

Α	rea (sf)	CN	Description		
	12,304	39	>75% Gras	s cover, Go	bod, HSG A
	16,635	30	Woods, Go	od, HSG A	
	28,939	34	Weighted A	verage	
	28,939		100.00% Pe	ervious Are	a
Tc	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	
5.0					Direct Entry, Direct

Subcatchment DA3a: DA3a pervious



Summary for Subcatchment DA3ai: DA3a impervious

Runoff = 2.78 cfs @ 12.07 hrs, Volume= 9,555 cf, Depth= 5.88" Routed to Pond BIO1 : BIO 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

	Area (sf)	CN	Description		
	16,965	98	Paved park	ing, HSG A	N Contraction of the second seco
*	1,223	98	Sidewalk, H	ISG A	
	1,307	98	Water Surfa	ace, HSG A	
	19,495	98	Weighted A	verage	
	19,495		100.00% In	npervious A	rea
To (min	c Length) (feet)	Slop (ft/fl	e Velocity t) (ft/sec)	Capacity (cfs)	Description
5.0)		, , , ,		Direct Entry, Direct

Subcatchment DA3ai: DA3a impervious





Summary for Subcatchment DA3b: DA3b pervious

Runoff	=	0.01 cfs @	12.40 hrs,	Volume=
Routed	d to P	ond DMH : DMH	200	

179 cf, Depth= 0.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

Α	rea (sf)	CN	Description		
	2,453	39	>75% Gras	s cover, Go	bod, HSG A
	3,792	30	Woods, Go	od, HSG A	
	387	76	Gravel road	ls, HSG A	
	6,632	36	Weighted A	verage	
	6,632		100.00% P	ervious Are	a
_					
Tc	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/f	i) (ft/sec)	(cfs)	
5.0					Direct Entry, Direct

Subcatchment DA3b: DA3b pervious



Summary for Subcatchment DA3bi: DA3b impervious

Runoff	=	1.50 cfs @	12.07 hrs,	Volume=
Routed	to F	Pond DMH : DMF	200	

5,147 cf, Depth= 5.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

rea (sf)	CN	Description		
9,746	98	Paved park	ing, HSG A	
756	98	Sidewalks,	HŠG A	
10,502	98	Weighted A	verage	
10,502		100.00% Im	npervious A	rea
Length	Slope	e Velocity	Capacity	Description
(feet)	(ft/ft	:) (ft/sec)	(cfs)	
				Direct Entry, Direct
	<u>rea (sf)</u> 9,746 756 10,502 10,502 Length (feet)	<u>vrea (sf) CN</u> 9,746 98 756 98 10,502 98 10,502 Length Slope (feet) (ft/ft	area (sf)CNDescription9,74698Paved park75698Sidewalks,10,50298Weighted A10,502100.00%ImLengthSlopeVelocity(feet)(ft/ft)(ft/sec)	area (sf)CNDescription9,74698Paved parking, HSG A75698Sidewalks, HSG A10,50298Weighted Average10,502100.00%Impervious ALengthSlopeVelocityCapacity(feet)(ft/ft)(ft/sec)(cfs)

Subcatchment DA3bi: DA3b impervious



Summary for Subcatchment DA3c: DA3c pervious

Runoff = 0.02 cfs @ 13.74 hrs, Volume= Routed to Pond 100 : CB 100 412 cf, Depth= 0.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

Α	rea (sf)	CN	Description		
	10,027	39	>75% Gras	s cover, Go	ood, HSG A
	16,065	30	Woods, Go	od, HSG A	
	26,092	33	Weighted A	verage	
	26,092		100.00% Pe	ervious Are	ea
_					
Tc	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	
5.0					Direct Entry, Direct

Subcatchment DA3c: DA3c pervious



Summary for Subcatchment DA3ci: DA3c impervious

Runoff = 0.46 cfs @ 12.07 hrs, Volume= 1,582 cf, Depth= 5.88" Routed to Pond 100 : CB 100

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

A	rea (sf)	CN	Description		
	2,698	98	Paved park	ing, HSG A	Ν
*	530	98	Sidewalks,	HŠG A	
	3,228	98	Weighted A	verage	
	3,228		100.00% Im	npervious A	Area
Тс	Length	Slop	e Velocity	Capacity	Description
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	
5.0					Direct Entry, Direct

Subcatchment DA3ci: DA3c impervious



Summary for Subcatchment DA3d: DA3d pervious

Runoff = 0.00 cfs @ 12.32 hrs, Volume= Routed to Pond 200 : CB 200 8 cf, Depth= 0.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"



Summary for Subcatchment DA3di: DA3d impervious

Runoff = 0.67 cfs @ 12.07 hrs, Volume= 2,297 cf, Depth= 5.88" Routed to Pond 200 : CB 200

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"

	А	rea (sf)	CN D	escription					
		4,686	98 P	aved park	ing, HSG A	4			
		4,686	10	00.00% In	npervious A	Area			
(m	Tc nin)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
	5.0					Direct Entry,			
				Subca	tchment	DA3di: DA3d	l impervious		
					Hydro	graph			
									Runoff
	0.7		0.67 cfs		+-+-+	 	· · · · · · · · · · · · · · · · · · ·		
	0.65	/ I I I I / _!!!!		 <u> </u> - <u> </u>			Type III 2	24-hr	
	0.6				+ - + - + - +	2	5YR Rainfall=	6.12"	
	0.55					R H	noff Aroa=4-6	86 sf	
	0.5					└─└─└─╵ ── ┪ <u>╴</u> ╝ <u>╴</u> ┙╸			
~	0.45					Runo	ff volume=2,2	97 CT	
(cfs	0.4					\mathbb{R}	lunoff Depth=	5.88"	
No	0.35			 - + - -		 - - - -	Tc=5.0	min	
ш	0.3							N_00	
	0.25							N-30	
	0.2								
	0.15						+ - + - + - + - + - + - + - + - + - + -		
	0.1								
	0.05						+ - + - + - + - + - + - + - + - + - + -		
	0-								
	(02468	3 10 12 14 1	6 18 20 22 24	26 28 30 32 3 Tim	34 36 38 40 42 44 46 4 hours)	18 50 52 54 56 58 60 62 64 6	6 68 70 72	

Summary for Subcatchment R1E: EAST ROOF

Runoff = 1.32 cfs @ 12.07 hrs, Volume= Routed to Pond URC2 : URC-2 4,546 cf, Depth= 5.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"



Summary for Subcatchment R1W: WEST ROOF

Runoff = 1.39 cfs @ 12.07 hrs, Volume= Routed to Pond URC3 : URC-3 4,785 cf, Depth= 5.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 25YR Rainfall=6.12"



Summary for Pond 100: CB 100

[57] Hint: Peaked at 50.44' (Flood elevation advised)

Inflow Area	a =	29,320 sf,	11.01% Imperv	ious, Inflow	/ Depth =	0.82"	for 25Y	'R event
Inflow	=	0.46 cfs @	12.07 hrs, Volu	me=	1,995 c	f		
Outflow	=	0.46 cfs @	12.07 hrs, Volu	me=	1,995 ct	f, Atten	= 0%, L	ag= 0.0 min
Primary	=	0.46 cfs @	12.07 hrs, Volu	me=	1,995 ct	f		•
Routed	to Pond	RB1 : RB 10	1,102					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 50.44' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.00'	12.0" Round Culvert L= 4.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 50.00' / 49.98' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.46 cfs @ 12.07 hrs HW=50.44' (Free Discharge) **1=Culvert** (Barrel Controls 0.46 cfs @ 2.01 fps)



Pond 100: CB 100

Summary for Pond 200: CB 200

[57] Hint: Peaked at 52.29' (Flood elevation advised)

Inflow Area	a =	4,883 s	f, 95.97% Ir	npervious,	Inflow Depth =	5.66"	for 25YR event
Inflow	=	0.67 cfs @	12.07 hrs,	Volume=	2,305 c	f	
Outflow	=	0.67 cfs @	12.07 hrs,	Volume=	2,305 c	f, Atten	= 0%, Lag= 0.0 min
Primary	=	0.67 cfs @	12.07 hrs,	Volume=	2,305 c	f	-
Routed	to Pond	TT2 : Tree	Trench 2				
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	0 c	f	
Routed	to Pond	RB2 : RB 2	02,202,203				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 52.29' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	52.70'	12.0" Round Culvert L= 4.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 52.70' / 52.68' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Primary	51.66'	8.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.66' / 51.41' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Secondary	55.79'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.67 cfs @ 12.07 hrs HW=52.29' (Free Discharge) ←2=Culvert (Barrel Controls 0.67 cfs @ 2.54 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=51.66' (Free Discharge) -1=Culvert (Controls 0.00 cfs) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 200: CB 200



Summary for Pond BIO1: BIO 1

Inflow Ar Inflow Outflow Primary Route	rea = = 2 = 2 = 2 ed to Pond U	48,434 sf, 40 .78 cfs @ 12 .61 cfs @ 12 .61 cfs @ 12 RC1 : URC-1	0.25% Impervious .07 hrs, Volume= .10 hrs, Volume= .10 hrs, Volume=	s, Inflow D = 1 = 1 = 1	epth = 2.51" 0,113 cf 0,113 cf, Atten 0,113 cf	for 25YR event ı= 6%, Lag= 1.7 min
Routing Peak Ele	by Stor-Ind n ev= 59.50' @	nethod, Time S 12.10 hrs Si	Span= 0.00-72.00 urf.Area= 1,663 s) hrs, dt= 0 f Storage	.01 hrs = 1,300 cf	
Plug-Flo Center-o	w detention t f-Mass det. t	ime= 61.0 mir ime= 61.0 mir	n calculated for 10 n(819.9 - 758.8)),112 cf (10))0% of inflow)	
Volume	Invert	Avail.Stor	age Storage De	escription		
#1	58.50'	2,21	0 cf Custom St	tage Data	(Prismatic) Liste	ed below (Recalc)
Elevatio (fee	n Su t)	rf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Stor (cubic-fee	e t)	
58.5	50	800	0		0	
59.0	0	1,380	545	54	5	
60.0	0	1,950	1,665	2,21	0	
Device	Routing	Invert	Outlet Devices			
#1	Primary	55.09'	12.0" Round C L= $45.0'$ CPP, p Inlet / Outlet Inver- n= 0.013 Corruc	ulvert projecting, ert= 55.09'	no headwall, K / 54.87' S= 0.0	e= 0.900 0049 '/' Cc= 0.900 Elow Area= 0.79 sf
#2	Device 1	59.25'	24.0" Horiz. Orifice/Grate C= 0.600			
#3 #4	Device 1	55.38'	4.0" Round Culvert L= 38.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 55.38' / 55.19' S= 0.0050 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf 2.470 in/br Exfiltration over Surface area			
Primary OutFlow Max=2.60 cfs @ 12.10 hrs HW=59.50' (Free Discharge)						

Ivert (Passes 2.60 cfs of 5.90 cfs potential flow)

-2=Orifice/Grate (Weir Controls 2.51 cfs @ 1.62 fps) -3=Culvert (Passes 0.10 cfs of 0.63 cfs potential flow) -4=Exfiltration (Exfiltration Controls 0.10 cfs)

Pond BIO1: BIO 1



Summary for Pond BIO2: BIO 2

Inflow Ar Inflow Outflow Primary Route	rea = = 1 = 1 = 1 ed to Pond U	14,564 sf, 6 .41 cfs @ 12 .38 cfs @ 12 .38 cfs @ 12 RC2 : URC-2	7.93% Imperviou .07 hrs, Volume .09 hrs, Volume .09 hrs, Volume	s, Inflow De = 5 = 5 = 5	pth = 4.15" fo ,036 cf ,036 cf, Atten= 3 ,036 cf	r 25YR event 3%, Lag= 1.1 min	
Routing I Peak Ele	by Stor-Ind n ev= 63.16' @	nethod, Time S 12.09 hrs Si	Span= 0.00-72.0 urf.Area= 814 sf	0 hrs, dt= 0.0 Storage= 4)1 hrs 09 cf		
Plug-Flov Center-o	w detention t f-Mass det. t	time= 37.5 mir time= 37.5 mir	n calculated for 5 n (789.3 - 751.7	,036 cf (100 ⁰)	% of inflow)		
Volume	Invert	Avail.Stor	age Storage D	escription			
#1	62.50'	1,34	0 cf Custom S	tage Data (I	Prismatic)Listed	below (Recalc)	
Elevatio (fee	n Su t)	ırf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	: •		
62.5	0	450	0	 C)		
63.0	0	700	288	288)		
64.0	0	1,405	1,053	1,340	I		
Device	Routing	Invert	Outlet Devices				
#1	Primary	59.00'	12.0" Round C L= 25.0' CPP, Inlet / Outlet Inv n= 0.013 Corru	ulvert end-section ert= 59.00' / gated PE, sr	conforming to fill, 58.88' S= 0.004 nooth interior. Fl	Ke= 0.500 8 '/' Cc= 0.900 ow Area= 0.79 sf	
#2	#2 Device 1 63.00'		24.0" Horiz. Orifice/Grate C= 0.600				
	D · · · ·	50.001	Limited to weir f	low at low he	ads		
#3	Device 1	59.30'	4.0" Round Cu L= 25.0' CPP, Inlet / Outlet Inv n= 0.010 PVC,	l vert projecting, n ert= 59.30' / smooth inter	o headwall, Ke= 59.18' S= 0.004 ior, Flow Area=	0.900 8 '/' Cc= 0.900 0.09 sf	
#4	Device 3	62.50	2.4/U IN/Nr Exfi		r Surtace area		
Primary	OutFlow Masse	ax=1.38 cfs @ s 1 38 cfs of 7	24 cfs potential	:63.16' (Fre flow)	e Discharge)		

Culvert (Passes 1.38 cfs of 7.24 cfs potential flow)
 -2=Orifice/Grate (Weir Controls 1.33 cfs @ 1.31 fps)
 -3=Culvert (Passes 0.05 cfs of 0.64 cfs potential flow)
 -4=Exfiltration (Exfiltration Controls 0.05 cfs)

Pond BIO2: BIO 2



Summary for Pond DMH: DMH 200

[57] Hint: Peaked at 54.70' (Flood elevation advised)

Inflow Area	a =	17,134 sf	,61.29% In	npervious,	Inflow Depth =	3.73"	for 25YR event
Inflow	=	1.50 cfs @	12.07 hrs,	Volume=	5,326 c	f	
Outflow	=	1.50 cfs @	12.07 hrs,	Volume=	5,326 ct	f, Atten	= 0%, Lag= 0.0 min
Primary	=	1.07 cfs @	12.07 hrs,	Volume=	5,046 ct	f	-
Routed	to Pond	TT1: Tree T	rench 1				
Secondary	=	0.43 cfs @	12.07 hrs,	Volume=	280 ct	f	
Routed	to Pond	URC1 : URC	C-1				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 54.70' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	54.30'	12.0" Round Culvert
			L= 9.0° CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 54.30' / 54.26' S= 0.0044 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Primary	53.78'	12.0" Round Culvert
	2		L= 98.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 53.78' / 53.78' S= 0.0000 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.07 cfs @ 12.07 hrs HW=54.70' (Free Discharge) ←2=Culvert (Barrel Controls 1.07 cfs @ 1.84 fps)

Secondary OutFlow Max=0.43 cfs @ 12.07 hrs HW=54.70' (Free Discharge) -1=Culvert (Barrel Controls 0.43 cfs @ 2.15 fps) Pond DMH: DMH 200



Summary for Pond RB1: RB 101,102

Inflow Ar Inflow Outflow Discarde Primary Route	rea = 0 = 0 ed = 0 = 0 ed to Pond S	29,320 sf, 11.0 .46 cfs @ 12.07 .06 cfs @ 11.61 .06 cfs @ 11.61 .00 cfs @ 0.00 P3 : SP3	1% Impervious, Inflow Depth = 0.82" for 25YR event 1 hrs, Volume= 1,995 cf hrs, Volume= 1,995 cf, Atten= 87%, Lag= 0.0 min hrs, Volume= 1,995 cf 0 hrs, Volume= 0 cf				
Routing Peak Ele	Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 44.93' @ 12.74 hrs Surf.Area= 157 sf Storage= 457 cf						
Plug-Flor Center-o	Plug-Flow detention time= 48.0 min calculated for 1,994 cf (100% of inflow) Center-of-Mass det. time= 48.0 min (851.4 - 803.5)						
Volume	Invert	Avail.Storage	e Storage Description				
#1	41.00'	339 c	f 6.00'D x 6.00'H Recharger x 2 Inside #2				
#2	39.00'	355 c	f 10.00'D x 9.00'H Stone x 2 1,414 cf Overall - 339 cf Embedded = 1,074 cf x 33.0% Voids				
	694 cf Total Available Storage						
Device	Routing	Invert O	utlet Devices				
#1 #2	Discarded Primary	39.00' 8. 46.50' 5. He 2. Co 2.	270 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01' D' long x 5.0' breadth Broad-Crested Rectangular Weir X 2.00 ead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 50 3.00 3.50 4.00 4.50 5.00 5.50 bef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 55 2.67 2.66 2.68 2.70 2.74 2.79 2.88				
Discarded OutFlow Max=0.06 cfs @ 11.61 hrs HW=39.09' (Free Discharge)							

1=Exfiltration (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=39.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)
Pond RB1: RB 101,102



Summary for Pond RB2: RB 202,202,203

[92] Warning: Device #2 is above defined storage

Inflow Area	a =	4,883 sf,	95.97% In	npervious,	Inflow Depth =	2.30"	for 25Y	'R event
Inflow	=	0.64 cfs @	12.07 hrs,	Volume=	936 c	f		
Outflow	=	0.09 cfs @	11.74 hrs,	Volume=	936 c	f, Atter	n= 86%,	Lag= 0.0 min
Discarded	=	0.09 cfs @	11.74 hrs,	Volume=	936 c	f		•
Primary	=	0.00 cfs @	0.00 hrs,	Volume=	0 c	f		
Routed	to Pond	SP3 : SP3						

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 49.38' @ 12.52 hrs Surf.Area= 236 sf Storage= 543 cf

Plug-Flow detention time= 55.6 min calculated for 936 cf (100% of inflow) Center-of-Mass det. time= 55.6 min (789.5 - 733.9)

Volume	Invert	Avail.Stora	age Storage Description
#1	46.50'	50	9 cf 6.00'D x 6.00'H Recharger x 3 Inside #2
#2	44.50'	53	2 cf 10.00'D x 9.00'H Stone x 3 2 121 cf Overall - 509 cf Embedded = 1 612 cf, x 33 0% Voids
		1,04	1 cf Total Available Storage
Device	Routing	Invert	Outlet Devices
#1 #2	Discarded Primary	44.50' 55.61'	8.270 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01' 5.0' long x 5.0' breadth Broad-Crested Rectangular Weir X 2.00 Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Discarded OutFlow Max=0.09 cfs @ 11.74 hrs HW=44.61' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=44.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Hydrograph Inflow 0.64 cfs Outflow Inflow Area=4,883 sf Discarded Primary 0.7 Peak Elev=49.38' 0.65 0.6 Storage=543 cf 0.55 0.5 0.45 (cfs) 0.4 Flow 0.35 0.3 0.25 <u>0 09 cfs</u> 0.2 0.09 cfs 0.15 0.1 0.00 cfs 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72

Time (hours)

Pond RB2: RB 202,202,203

Summary for Pond RB3: RB 300

Inflow Ar	ea =	22,132 sf, 0.0	00% Impervious, Inflow Depth = 0.23" for 25YR event			
Inflow	= 0	.02 cfs @ 13.0	1 hrs, Volume= 427 cf			
Outflow	= 0	.02 cfs @ 13.0	2 hrs, Volume= 427 cf, Atten= 0%, Lag= 0.7 min			
Discarde	d = 0	.02 cfs @13.0	2 hrs, Volume= 427 cf			
Primary	= 0	.00 cfs @0.0	0 hrs, Volume= 0 cf			
Route	d to Pond S	P2 : SP2	,			
Routing b	by Stor-Ind n	nethod, Time Sp	oan= 0.00-72.00 hrs, dt= 0.01 hrs / 3			
Peak Ele	v= 58.56' @	13.02 hrs Sur	f.Area= 57 sf Storage= 1 cf			
	U		5			
Plug-Flo	w detention t	time= 1.1 min ca	alculated for 426 cf (100% of inflow)			
Center-o	f-Mass det. t	time= 1.1 min (⁻	1,027.1 - 1,026.0)			
		Υ.				
Volume	Invert	Avail.Storag	je Storage Description			
#1	59.50'	50	cf 4.00'D x 4.00'H Recharger Inside #2			
#2	58.50'	133	cf 6.00'D x 8.00'H Stone x 2			
			452 cf Overall - 50 cf Embedded = 402 cf x 33.0% Voids			
		183	cf Total Available Storage			
			······································			
Device	Routing	Invert C	Dutlet Devices			
#1	Discarded	58.50' 8	.270 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01'			
#2	Primary	65.50' 5	.0' long x 5.0' breadth Broad-Crested Rectangular Weir			
			lead (feet) 0.20, 0.40, 0.60, 0.80, 1.00, 1.20, 1.40, 1.60, 1.80, 2.00			
		2	50 3 00 3 50 4 00 4 50 5 00 5 50			
			Coef (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65			
		2	65 267 266 268 270 274 279 288			
		-				
Discarded OutFlow Max=0.02 cfs @ 13.02 hrs HW=58.56' (Free Discharge)						

1=Exfiltration (Exfiltration Controls 0.02 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=58.50' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond RB3: RB 300



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area	=	158,640 sf,	9.44% Imperviou	s, Inflow Depth = (0.43" for 2	5YR event
Inflow	:	=	0.53 cfs @	12.49 hrs, Volume	= 5,628 cf		
Primary	y :	=	0.53 cfs @	12.49 hrs, Volume	= 5,628 cf,	, Atten= 0%	, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow <i>J</i>	Area	=	54,580 sf,	0.00% In	npervious,	Inflow Depth =	0.14"	for 25	5YR event	
Inflow		=	0.02 cfs @	13.07 hrs,	Volume=	625 cf	F			
Primar	y	=	0.02 cfs @	13.07 hrs,	Volume=	625 ct	f, Atter	ר= 0%,	Lag= 0.0 mir	n

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	rea =	34,203 sf,	23.14% Impervious,	Inflow Depth = 0.00"	for 25YR event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	i= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP3: SP3

Summary for Pond TT1: Tree Trench 1

[81] Warning: Exceeded Pond DMH by 0.28' @ 12.57 hrs

Inflow Area	ı =	17,134 sf,	61.29% In	npervious,	Inflow Depth =	3.53"	for 25YR event
Inflow	=	1.07 cfs @	12.07 hrs,	Volume=	5,046 c	f	
Outflow	=	1.04 cfs @	12.09 hrs,	Volume=	5,046 c	f, Atten	= 2%, Lag= 1.2 min
Discarded	=	0.12 cfs @	11.21 hrs,	Volume=	3,937 c	f	
Primary	=	0.92 cfs @	12.09 hrs,	Volume=	1,109 c ⁻	f	
Routed	to Pond	URC1 : URC	-1				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 54.79' @ 12.09 hrs Surf.Area= 639 sf Storage= 543 cf

Plug-Flow detention time= 16.2 min calculated for 5,045 cf (100% of inflow) Center-of-Mass det. time= 16.2 min (769.7 - 753.6)

Volume	Invert	Avail.Stor	age	Storage Description			
#1	52.34'	62	7 cf	6.20'W x 103.00'L x 3.00'H Prismatoid			
	50 701		.	1,916 cf Overall - 32 cf Embedded = 1,884 cf x 33.3% Voids			
#2	53.78'	3	2 cf	8.0" Round Pipe Storage Inside #1 L= 92.0' S= 0.0050 '/'			
		65	9 cf	Total Available Storage			
Device	Routing	Invert	Outle	et Devices			
#1	Discarded	52.34'	8.270	0 in/hr Exfiltration over Surface area Phase-In= 0.01'			
#2	Primary	54.30'	12.0'	"Vert. Orifice/Grate C= 0.600			
			Limit	ed to weir flow at low heads			
Discard [€] —1=Ex	Discarded OutFlow Max=0.12 cfs @ 11.21 hrs HW=52.37' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.12 cfs)						

Primary OutFlow Max=0.92 cfs @ 12.09 hrs HW=54.79' (Free Discharge) ←2=Orifice/Grate (Orifice Controls 0.92 cfs @ 2.39 fps) Pond TT1: Tree Trench 1



Summary for Pond TT2: Tree Trench 2

[81] Warning: Exceeded Pond 200 by 0.96' @ 12.58 hrs

Inflow Area = 4,883 sf, 95.97% Impervious, Inflow Depth = 5.66" for 25YR event Inflow 0.67 cfs @ 12.07 hrs, Volume= = 2,305 cf 0.67 cfs @ 12.07 hrs, Volume= Outflow = 2,305 cf, Atten= 0%, Lag= 0.3 min Discarded = 0.03 cfs @ 9.71 hrs, Volume= 1,369 cf 0.64 cfs @ 12.07 hrs, Volume= Primary = 936 cf Routed to Pond RB2 : RB 202,202,203

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 53.10' @ 12.07 hrs Surf.Area= 150 sf Storage= 159 cf

Plug-Flow detention time= 21.0 min calculated for 2,304 cf (100% of inflow) Center-of-Mass det. time= 21.0 min (765.7 - 744.6)

Volume	Invert	Avail.Stora	ge Storage Description			
#1	50.16'	184	cf 5.00'W x 30.00'L x 3.80'H Prismatoid			
#2	51.66'	17	570 cf Overall - 17 cf Embedded = 553 cf x 33.3% Voids cf 8.0" Round Pipe Storage Inside #1 L= 50.0' S= 0.0050 '/'			
		201	cf Total Available Storage			
Device	Routing	Invert (Dutlet Devices			
#1	Discarded	50.16' 8	.270 in/hr Exfiltration over Surface area Phase-In= 0.01'			
#2	Primary	52.70' 1 L	2.0" Vert. Orifice/Grate C= 0.600 imited to weir flow at low heads			
Discarded OutFlow Max=0.03 cfs @ 9.71 hrs HW=50.20' (Free Discharge)						

Primary OutFlow Max=0.64 cfs @ 12.07 hrs HW=53.10' (Free Discharge) ←2=Orifice/Grate (Orifice Controls 0.64 cfs @ 2.16 fps)

Pond TT2: Tree Trench 2



Summary for Pond URC1: URC-1

Inflow Area	a =	65,568 sf,	45.75% Impervious,	Inflow Depth =	2.11"	for 25YR e	event
Inflow	=	3.92 cfs @	12.09 hrs, Volume=	11,503 ct	F		
Outflow	=	0.40 cfs @	11.70 hrs, Volume=	11,503 ct	f, Atten=	= 90%, Lag	g= 0.0 min
Discarded	=	0.40 cfs @	11.70 hrs, Volume=	11,503 ct	f		-

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 52.18' @ 12.72 hrs Surf.Area= 0.048 ac Storage= 0.090 af

Plug-Flow detention time= 62.9 min calculated for 11,501 cf (100% of inflow) Center-of-Mass det. time= 62.9 min (872.3 - 809.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	48.60'	0.087 af	16.33'W x 128.59'L x 7.25'H Field A
			0.350 af Overall - 0.087 af Embedded = 0.262 af x 33.3% Voids
#2A	50.60'	0.087 af	ADS_StormTech MC-3500 d +Cap x 34 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			34 Chambers in 2 Rows
			Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf
		0.175 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	48.60'	8.270 in/hr Exfiltration over Surface area	Phase-In= 0.01'

Discarded OutFlow Max=0.40 cfs @ 11.70 hrs HW=48.68' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.40 cfs)

Pond URC1: URC-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cfOverall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

17 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 125.59' Row Length +18.0" End Stone x 2 = 128.59' Base Length 2 Rows x 77.0" Wide + 6.0" Spacing x 1 + 18.0" Side Stone x 2 = 16.33' Base Width 24.0" Stone Base + 45.0" Chamber Height + 18.0" Stone Cover = 7.25' Field Height

34 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 2 Rows = 3,798.0 cf Chamber Storage

15,227.2 cf Field - 3,798.0 cf Chambers = 11,429.2 cf Stone x 33.3% Voids = 3,805.9 cf Stone Storage

Chamber Storage + Stone Storage = 7,603.9 cf = 0.175 af Overall Storage Efficiency = 49.9% Overall System Size = 128.59' x 16.33' x 7.25'

34 Chambers 564.0 cy Field 423.3 cy Stone





Pond URC1: URC-1



Summary for Pond URC2: URC-2

Inflow Area	ı =	23,838 sf,	80.41% Impervious,	Inflow Depth = 4	4.82" fo	r 25YR event
Inflow	=	2.68 cfs @	12.08 hrs, Volume=	9,582 cf		
Outflow	=	0.29 cfs @	11.50 hrs, Volume=	9,582 cf,	, Atten= 8	39%, Lag= 0.0 min
Discarded	=	0.29 cfs @	11.50 hrs, Volume=	9,582 cf		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 58.17' @ 12.78 hrs Surf.Area= 1,499 sf Storage= 2,982 cf

Plug-Flow detention time= 66.9 min calculated for 9,581 cf (100% of inflow) Center-of-Mass det. time= 66.9 min (834.6 - 767.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	54.50'	2,405 cf	36.08'W x 41.55'L x 6.75'H Field A
			10,120 cf Overall - 2,898 cf Embedded = 7,222 cf x 33.3% Voids
#2A	56.50'	2,898 cf	ADS_StormTech MC-3500 d +Cap x 25 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			25 Chambers in 5 Rows
			Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf
		5,303 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	54.50'	8.270 in/hr Exfiltration over Surface area	^o hase-In= 0.01'
Discard	led OutFlow	Max=0.29 cfs	@ 11.50 hrs HW=54.57' (Free Discharge)	

1=Exfiltration (Exfiltration Controls 0.29 cfs)

Pond URC2: URC-2 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

5 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 39.55' Row Length +12.0" End Stone x 2 = 41.55'Base Length 5 Rows x 77.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 36.08' Base Width 24.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

25 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 5 Rows = 2,897.8 cf Chamber Storage

10,120.0 cf Field - 2,897.8 cf Chambers = 7,222.2 cf Stone x 33.3% Voids = 2,405.0 cf Stone Storage

Chamber Storage + Stone Storage = 5,302.8 cf = 0.122 af Overall Storage Efficiency = 52.4%Overall System Size = $41.55' \times 36.08' \times 6.75'$

25 Chambers 374.8 cy Field 267.5 cy Stone





Pond URC2: URC-2



Summary for Pond URC3: URC-3

Inflow Area	ı =	9,762 sf,	100.00% Impervious,	Inflow Depth =	5.88" for 2	25YR event
Inflow	=	1.39 cfs @	12.07 hrs, Volume=	4,785 cf		
Outflow	=	0.13 cfs @	11.39 hrs, Volume=	4,785 cf	, Atten= 90%	6, Lag= 0.0 min
Discarded	=	0.13 cfs @	11.39 hrs, Volume=	4,785 cf		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 59.72' @ 12.82 hrs Surf.Area= 823 sf Storage= 1,564 cf

Plug-Flow detention time= 76.9 min calculated for 4,785 cf (100% of inflow) Center-of-Mass det. time= 76.9 min (820.9 - 743.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	56.30'	1,243 cf	23.25'W x 35.38'L x 6.25'H Field A
			5,141 cf Overall - 1,409 cf Embedded = 3,732 cf x 33.3% Voids
#2A	57.80'	1,409 cf	ADS_StormTech MC-3500 d +Cap x 12 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			12 Chambers in 3 Rows
			Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf
		2,652 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	56.30'	7.000 in/hr Exfiltration over Surface area	Phase-In= 0.01'
Discard	ed OutFlow	Max=0.13 cfs	@ 11.39 hrs HW=56.36' (Free Discharge)	

1=Exfiltration (Exfiltration Controls 0.13 cfs)

Pond URC3: URC-3 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cfOverall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

4 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 32.38' Row Length +18.0" End Stone x 2 = 35.38' Base Length 3 Rows x 77.0" Wide + 6.0" Spacing x 2 + 18.0" Side Stone x 2 = 23.25' Base Width 18.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.25' Field Height

12 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 3 Rows = 1,408.8 cf Chamber Storage

5,141.2 cf Field - 1,408.8 cf Chambers = 3,732.3 cf Stone x 33.3% Voids = 1,242.9 cf Stone Storage

Chamber Storage + Stone Storage = 2,651.7 cf = 0.061 af Overall Storage Efficiency = 51.6%Overall System Size = $35.38' \times 23.25' \times 6.25'$

12 Chambers 190.4 cy Field 138.2 cy Stone





Pond URC3: URC-3



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

SubcatchmentDA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=1.30" Flow Length=417' Tc=14.5 min CN=38 Runoff=2.86 cfs 17,162 cf
Subcatchment DA2a: DA2a	Runoff Area=22,132 sf 0.00% Impervious Runoff Depth=0.91" Flow Length=339' Tc=17.5 min CN=34 Runoff=0.21 cfs 1,675 cf
Subcatchment DA2b: DA2b pervious	Runoff Area=4,670 sf 0.00% Impervious Runoff Depth=1.40" Tc=5.0 min CN=39 Runoff=0.13 cfs 545 cf
SubcatchmentDA2bi: DA2b impervious	Runoff Area=9,894 sf 100.00% Impervious Runoff Depth=8.32" Tc=5.0 min CN=98 Runoff=1.98 cfs 6,860 cf
SubcatchmentDA2c: DA2c	Runoff Area=32,448 sf 0.00% Impervious Runoff Depth=0.91" Flow Length=255' Tc=21.2 min CN=34 Runoff=0.29 cfs 2,456 cf
Subcatchment DA3a: DA3a pervious	Runoff Area=28,939 sf 0.00% Impervious Runoff Depth=0.91" Tc=5.0 min CN=34 Runoff=0.34 cfs 2,190 cf
SubcatchmentDA3ai: DA3a impervious	Runoff Area=19,495 sf 100.00% Impervious Runoff Depth=8.32" Tc=5.0 min CN=98 Runoff=3.90 cfs 13,516 cf
Subcatchment DA3b: DA3b pervious	Runoff Area=6,632 sf 0.00% Impervious Runoff Depth=1.10" Tc=5.0 min CN=36 Runoff=0.12 cfs 608 cf
SubcatchmentDA3bi: DA3b impervious	Runoff Area=10,502 sf 100.00% Impervious Runoff Depth=8.32" Tc=5.0 min CN=98 Runoff=2.10 cfs 7,281 cf
Subcatchment DA3c: DA3c pervious	Runoff Area=26,092 sf 0.00% Impervious Runoff Depth=0.82" Tc=5.0 min CN=33 Runoff=0.23 cfs 1,775 cf
Subcatchment DA3ci: DA3c impervious	Runoff Area=3,228 sf 100.00% Impervious Runoff Depth=8.32" Tc=5.0 min CN=98 Runoff=0.65 cfs 2,238 cf
Subcatchment DA3d: DA3d pervious	Runoff Area=197 sf 0.00% Impervious Runoff Depth=1.40" Tc=5.0 min CN=39 Runoff=0.01 cfs 23 cf
Subcatchment DA3di: DA3d impervious	Runoff Area=4,686 sf 100.00% Impervious Runoff Depth=8.32" Tc=5.0 min CN=98 Runoff=0.94 cfs 3,249 cf
SubcatchmentR1E: EAST ROOF	Runoff Area=9,274 sf 100.00% Impervious Runoff Depth=8.32" Tc=5.0 min CN=98 Runoff=1.85 cfs 6,430 cf
SubcatchmentR1W: WEST ROOF	Runoff Area=9,762 sf 100.00% Impervious Runoff Depth=8.32" Tc=5.0 min CN=98 Runoff=1.95 cfs 6,768 cf
Pond 100: CB 100 12.0" Rd	Peak Elev=50.60' Inflow=0.80 cfs 4,013 cf ound Culvert n=0.013 L=4.0' S=0.0050 '/' Outflow=0.80 cfs 4,013 cf

19038-POST V4

Type III 24-hr 100YR Rainfall=8.56"

Prepared by {enter your company name here} HydroCAD® 10.10-7a s/n 01445 © 2021 HydroCAD Software Solutions LLC

Printed 12/12/2021 Page 157

Pond 200: CB 200	Peak Elev=52.60' Inflow=0.94 cfs 3,272 cf Primary=0.94 cfs 3,272 cf Secondary=0.00 cfs 0 cf Outflow=0.94 cfs 3,272 cf
Pond BIO1: BIO 1	Peak Elev=59.58' Storage=1,434 cf Inflow=4.13 cfs 15,707 cf Outflow=3.92 cfs 15,707 cf
Pond BIO2: BIO 2	Peak Elev=63.21' Storage=452 cf Inflow=2.10 cfs 7,405 cf Outflow=2.05 cfs 7,405 cf
Pond DMH: DMH 200	Peak Elev=54.87' Inflow=2.20 cfs 7,889 cf Primary=1.39 cfs 7,239 cf Secondary=0.81 cfs 649 cf Outflow=2.20 cfs 7,889 cf
Pond RB1: RB 101,102	Peak Elev=46.58' Storage=604 cf Inflow=0.80 cfs 4,013 cf Discarded=0.06 cfs 3,084 cf Primary=0.53 cfs 929 cf Outflow=0.59 cfs 4,014 cf
Pond RB2: RB 202,202,203	Peak Elev=52.27' Storage=932 cf Inflow=0.91 cfs 1,628 cf Discarded=0.09 cfs 1,628 cf Primary=0.00 cfs 0 cf Outflow=0.09 cfs 1,628 cf
Pond RB3: RB 300	Peak Elev=65.56' Storage=165 cf Inflow=0.21 cfs 1,675 cf Discarded=0.02 cfs 1,025 cf Primary=0.18 cfs 652 cf Outflow=0.21 cfs 1,677 cf
Pond SP1: SP1	Inflow=2.86 cfs 17,162 cf Primary=2.86 cfs 17,162 cf
Pond SP2: SP2	Inflow=0.47 cfs 3,109 cf Primary=0.47 cfs 3,109 cf
Pond SP3: SP3	Inflow=0.53 cfs 929 cf Primary=0.53 cfs 929 cf
Pond TT1: Tree Trench 1	Peak Elev=54.89' Storage=563 cf Inflow=1.39 cfs 7,239 cf Discarded=0.12 cfs 5,115 cf Primary=1.25 cfs 2,125 cf Outflow=1.37 cfs 7,240 cf
Pond TT2: Tree Trench 2	Peak Elev=53.19' Storage=163 cf Inflow=0.94 cfs 3,272 cf Discarded=0.03 cfs 1,644 cf Primary=0.91 cfs 1,628 cf Outflow=0.94 cfs 3,272 cf
Pond URC1: URC-1	Peak Elev=55.76' Storage=0.173 af Inflow=5.91 cfs 18,481 cf Outflow=0.40 cfs 18,481 cf
Pond URC2: URC-2	Peak Elev=60.66' Storage=5,008 cf Inflow=3.88 cfs 13,834 cf Outflow=0.29 cfs 13,834 cf
Pond URC3: URC-3	Peak Elev=62.12' Storage=2,533 cf Inflow=1.95 cfs 6,768 cf Outflow=0.13 cfs 6,768 cf

Total Runoff Area = 346,591 sf Runoff Volume = 72,776 cf Average Runoff Depth = 2.52" 76.39% Pervious = 264,770 sf 23.61% Impervious = 81,821 sf

Summary for Subcatchment DA1: DA1

Runoff = 2.86 cfs @ 12.26 hrs, Volume= 17,162 cf, Depth= 1.30" Routed to Pond SP1 : SP1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	rea (sf)	CN E	Description			
118,297 30 Woods, Good, HSG A						
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A	
	9,642	98 F	Roofs, HSG	βA		
	5,338	98 F	Paved park	ing, HSG A		
1	58,640	38 V	Veighted A	verage		
1	43,660	g	0.56% Per	vious Area		
	14,980	g	.44% Impe	ervious Area	a	
			-			
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
10.6	50	0.1100	0.08		Sheet Flow, A TO B	
					Woods: Dense underbrush n= 0.800 P2= 3.33"	
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C	
					Woodland Kv= 5.0 fps	
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D	
					Short Grass Pasture Kv= 7.0 fps	
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1	
					Woodland Kv= 5.0 fps	
14.5	417	Total				

Subcatchment DA1: DA1



Summary for Subcatchment DA2a: DA2a

CN for permeable pavers taken from RI Stormwater Design

Runoff = 0.21 cfs @ 12.43 hrs, Volume= 1,675 cf, Depth= 0.91" Routed to Pond RB3 : RB 300

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

	A	rea (sf)	CN	Description		
		12,073	30	Woods, Go	od, HSG A	
		9,181	39	>75% Gras	s cover, Go	ood, HSG A
*		878	40	Pervious Pa	avers	
		22,132	34	Weighted A	verage	
22,132 100.00% Pervious Area				100.00% Pe	ervious Are	a
	Тс	Length	Slope	e Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)) (ft/sec)	(cfs)	
	13.6	50	0.0600	0.06		Sheet Flow, A TO B
						Woods: Dense underbrush n= 0.800 P2= 3.33"
	2.5	164	0.0490) 1.11		Shallow Concentrated Flow, B to C
						Woodland Kv= 5.0 fps
	1.4	125	0.0480) 1.53		Shallow Concentrated Flow, C to D (RB 3)
						Short Grass Pasture Kv= 7.0 fps
	17.5	339	Total			

Subcatchment DA2a: DA2a



Summary for Subcatchment DA2b: DA2b pervious

Runoff = 0.13 cfs @ 12.10 hrs, Volume= Routed to Pond BIO2 : BIO 2 545 cf, Depth= 1.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"



Summary for Subcatchment DA2bi: DA2b impervious

Runoff = 1.98 cfs @ 12.07 hrs, Volume= 6,860 cf, Depth= 8.32" Routed to Pond BIO2 : BIO 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	Area (sf)	CN	Description			
	7,075	98	Paved park	ing, HSG A	N	
*	2,044	98	Sidewalks,	HŠG A		
	775	98	Water Surfa	ace, HSG A	Ν	
	9,894	98	Weighted A	verage		
	9,894		100.00% Impervious Area			
-		~		0		
IC	Length	Slop	e Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft	t) (ft/sec)	(cfs)		
5.0					Direct Entry, Direct	

Subcatchment DA2bi: DA2b impervious





Summary for Subcatchment DA2c: DA2c

CN for permeable pavers taken from RI Stormwater Design

Runoff	=	0.29 cfs @	12.50 hrs,	Volume=	2,456 cf,	Depth= 0.91"
Routed	l to Pond	SP2 : SP2				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

/	Area (sf)	CN D	Description						
	18,364	30 V	30 Woods, Good, HSG A						
	14,084	39 >	39 >75% Grass cover, Good, HSG A						
	32,448	34 V	Veighted A	verage					
32,448 100.00% Pervious Area					а				
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
17.9	50	0.0300	0.05		Sheet Flow, A TO B				
					Woods: Dense underbrush n= 0.800 P2= 3.33"				
1.2	50	0.0200	0.71		Shallow Concentrated Flow, B to C				
					Woodland Kv= 5.0 fps				
2.1	155	0.0320	1.25		Shallow Concentrated Flow, C to SP2				
					Short Grass Pasture Kv= 7.0 fps				

21.2 255 Total

Subcatchment DA2c: DA2c



Summary for Subcatchment DA3a: DA3a pervious

Runoff = 0.34 cfs @ 12.13 hrs, Volume= 2,190 cf, Depth= 0.91" Routed to Pond BIO1 : BIO 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	rea (sf)	CN	Description					
	12,304	39	>75% Grass cover, Good, HSG A					
	16,635	30	Woods, Good, HSG A					
	28,939	34 Weighted Average						
	28,939 100.00% Pervious Area				a			
Тс	Length	Slop	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	t) (ft/sec)	(cfs)				
5.0					Direct Entry, Direct			

Subcatchment DA3a: DA3a pervious



Summary for Subcatchment DA3ai: DA3a impervious

Runoff = 3.90 cfs @ 12.07 hrs, Volume= 13,516 cf, Depth= 8.32" Routed to Pond BIO1 : BIO 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

	Area (sf)	CN	Description					
	16,965	98	Paved park	ing, HSG A	N			
*	1,223	98	Sidewalk, H	ISG A				
	1,307	98	Water Surfa	ace, HSG A	L Contraction of the second			
	19,495	98	98 Weighted Average					
	19,495		100.00% In	npervious A	rea			
Тс	l enath	Slon	e Velocity	Canacity	Description			
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)	Description			
5.0	(1001)	(101	(17000)	(010)	Direct Entry Direct			
5.0					Direct Entry, Direct			

Subcatchment DA3ai: DA3a impervious



Summary for Subcatchment DA3b: DA3b pervious

Runoff	=	0.12 cfs @	12.11 hrs,	Volume=
Route	d to P	ond DMH : DMH	200	

608 cf, Depth= 1.10"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	rea (sf)	CN	Description					
	2,453	39	>75% Gras	s cover, Go	bod, HSG A			
	3,792	30	Woods, Go	od, HSG A				
	387	76	Gravel road	ls, HSG A				
	6,632	36	Weighted Average					
	6,632		100.00% Pervious Area					
Тс	Length	Slop	e Velocity	Capacity	Description			
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
5.0					Direct Entry, Direct			

Subcatchment DA3b: DA3b pervious



Summary for Subcatchment DA3bi: DA3b impervious

Runoff	=	2.10 cfs @	12.07 hrs,	Volume=	7,281 cf,	Depth=	8.32"
Routed	to Pond	DMH : DMH	200				

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

Paved parking, HSG A					
Sidewalks, HSG A					
100.00% Impervious Area					
-					

Subcatchment DA3bi: DA3b impervious



Summary for Subcatchment DA3c: DA3c pervious

Runoff = 0.23 cfs @ 12.27 hrs, Volume= Routed to Pond 100 : CB 100 1,775 cf, Depth= 0.82"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

A	rea (sf)	CN	Description					
	10,027	39	>75% Grass cover, Good, HSG A					
	16,065	30	Woods, Good, HSG A					
	26,092	33	Weighted Average					
	26,092	100.00% Pervious Area						
Tc	Length	Slop	e Velocity	Capacity	Description			
(min)	(feet)	(ft/f	t) (ft/sec)	(cfs)				
5.0					Direct Entry, Direct			
					-			

Subcatchment DA3c: DA3c pervious



Summary for Subcatchment DA3ci: DA3c impervious

Runoff = 0.65 cfs @ 12.07 hrs, Volume= 2,238 cf, Depth= 8.32" Routed to Pond 100 : CB 100

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"

Α	rea (sf)	CN	Description						
	2,698	98	Paved parking, HSG A						
*	530	98	Sidewalks, HSG A						
	3,228	98	Weighted Average						
	3,228		100.00% Impervious Area						
Тс	Length	Slope	e Velocity	Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
5.0					Direct Entry, Direct				

Subcatchment DA3ci: DA3c impervious



Summary for Subcatchment DA3d: DA3d pervious

Runoff = 0.01 cfs @ 12.10 hrs, Volume= Routed to Pond 200 : CB 200 23 cf, Depth= 1.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"


Summary for Subcatchment DA3di: DA3d impervious

Runoff = 0.94 cfs @ 12.07 hrs, Volume= 3,249 cf, Depth= 8.32" Routed to Pond 200 : CB 200

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"



Summary for Subcatchment R1E: EAST ROOF

Runoff = 1.85 cfs @ 12.07 hrs, Volume= Routed to Pond URC2 : URC-2 6,430 cf, Depth= 8.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"



Summary for Subcatchment R1W: WEST ROOF

Runoff = 1.95 cfs @ 12.07 hrs, Volume= Routed to Pond URC3 : URC-3 6,768 cf, Depth= 8.32"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr 100YR Rainfall=8.56"



Summary for Pond 100: CB 100

[57] Hint: Peaked at 50.60' (Flood elevation advised)

Inflow Area	a =	29,320 sf,	11.01% In	npervious,	Inflow Depth =	1.64"	for 100YF	≀ event
Inflow	=	0.80 cfs @	12.09 hrs,	Volume=	4,013 c	f		
Outflow	=	0.80 cfs @	12.09 hrs,	Volume=	4,013 c	f, Atten	= 0%, Lag	= 0.0 min
Primary	=	0.80 cfs @	12.09 hrs,	Volume=	4,013 c	f		
Routed	to Pond	RB1 : RB 10	1,102					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 50.60' @ 12.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.00'	12.0" Round Culvert L= 4.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 50.00' / 49.98' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.80 cfs @ 12.09 hrs HW=50.60' (Free Discharge) ☐ 1=Culvert (Barrel Controls 0.80 cfs @ 2.32 fps)



Pond 100: CB 100

Summary for Pond 200: CB 200

[57] Hint: Peaked at 52.60' (Flood elevation advised)

Inflow Area	a =	4,883 sf,	95.97% In	npervious,	Inflow Depth =	8.04"	for 100YR ever	nt
Inflow	=	0.94 cfs @	12.07 hrs,	Volume=	3,272 c	f		
Outflow	=	0.94 cfs @	12.07 hrs,	Volume=	3,272 c	f, Atten	= 0%, Lag= 0.0	min
Primary	=	0.94 cfs @	12.07 hrs,	Volume=	3,272 c	f	·	
Routed	to Pond	TT2 : Tree T	rench 2					
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	0 c	f		
Routed	to Pond	RB2 : RB 20	2,202,203					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 52.60' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	52.70'	12.0" Round Culvert L= 4.0' CMP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 52.70' / 52.68' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf
#2	Primary	51.66'	8.0" Round Culvert
			Inlet / Outlet Invert= $51.66'$ / $51.41'$ S= $0.0050'$ // Cc= 0.900 n= 0.013 Corrugated PE smooth interior Flow Area= 0.35 sf
#3	Secondary	55.79'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir
	,		Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50
			Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.94 cfs @ 12.07 hrs HW=52.60' (Free Discharge) ←2=Culvert (Barrel Controls 0.94 cfs @ 2.70 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=51.66' (Free Discharge) -1=Culvert (Controls 0.00 cfs) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 200: CB 200



Summary for Pond BIO1: BIO 1

Inflow Ar Inflow Outflow Primary Route	rea = = 4 = 3 = 3 ed to Pond U	48,434 sf, 4 .13 cfs @ 12 .92 cfs @ 12 .92 cfs @ 12 RC1 : URC-1	0.25% Impervious 2.07 hrs, Volumes 2.10 hrs, Volumes 2.10 hrs, Volumes	s, Inflow D = 1 = 1 = 1)epth = 3.89" 15,707 cf 15,707 cf, Atter 15,707 cf	for 100YR event n= 5%, Lag= 1.6 min
Routing Peak Ele	by Stor-Ind n ev= 59.58' @	nethod, Time 12.10 hrs S	Span= 0.00-72.00 urf.Area= 1,708 s	0 hrs, dt= 0 f Storage).01 hrs e= 1,434 cf	
Plug-Flo Center-o	w detention t of-Mass det. t	time= 55.0 mii time= 55.0 mii	n calculated for 1 n (821.3 - 766.3)	5,705 cf (1))	00% of inflow)	
Volume	Invert	Avail.Stor	age Storage De	escription		
#1	58.50'	2,21	0 cf Custom St	tage Data	(Prismatic)List	ed below (Recalc)
Elevatio (fee	on Su t)	ırf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Stor (cubic-fee	re et)	
58.5	50	800	0	•	0	
59.0	00	1,380	545	54	15	
60.0	00	1,950	1,665	2,21	10	
Device	Routing	Invert	Outlet Devices			
#1	Primary	55.09'	12.0" Round C L= 45.0' CPP, p Inlet / Outlet Inve	ulvert projecting, ert= 55.09'	no headwall, K / 54.87' S= 0.4 smooth interior	(e= 0.900 0049 '/' Cc= 0.900 Elow Area= 0.79 sf
#2	Device 1	59.25'	24.0" Horiz. Ori	fice/Grate	C= 0.600 heads	
#3	Device 1	55.38'	4.0" Round Cu L= 38.0' CPP, p Inlet / Outlet Inve n= 0.010 PVC, s	Ivert projecting, ert= 55.38' smooth inte	no headwall, K / 55.19' S= 0.0 erior, Flow Are	‰= 0.900 0050 '/' Cc= 0.900 a= 0.09 sf
#4	Device 3	58.50'	2.470 in/hr Exfil	Itration ov	er Surface are	а
Primary	OutFlow M	ax=3.91 cfs @	0 12.10 hrs HW=	59.58' (Fi	ree Discharge)	

-1=Culvert (Passes 3.91 cfs of 5.96 cfs potential flow) -2=Orifice/Grate (Weir Controls 3.82 cfs @ 1.87 fps) -3=Culvert (Passes 0.10 cfs of 0.63 cfs potential flow) -4=Exfiltration (Exfiltration Controls 0.10 cfs)

Pond BIO1: BIO 1



Summary for Pond BIO2: BIO 2

Inflow An Inflow Outflow Primary Route	rea = = = = ed to Pond	14,564 sf, 6 2.10 cfs @ 12 2.05 cfs @ 12 2.05 cfs @ 12 URC2 : URC-2	7.93% Impervio .07 hrs, Volume .09 hrs, Volume .09 hrs, Volume	us, Inflow Depth e= 7,40 e= 7,40 e= 7,40	= 6.10" for 100YR event 5 cf 5 cf, Atten= 2%, Lag= 1.0 min 5 cf
Routing Peak Ele	by Stor-Ind ev= 63.21' (l method, Time S @ 12.09 hrs S	Span= 0.00-72.0 urf.Area= 849 st	00 hrs, dt= 0.01 h Storage= 452 d	rs cf
Plug-Flo Center-c	w detention of-Mass det	n time= 34.8 mir time= 34.8 mir	n calculated for n (786.3 - 751.5	7,404 cf (100% o 5)	f inflow)
Volume	Inve	rt Avail.Stor	age Storage D	Description	
#1	62.50)' 1,34	0 cf Custom	Stage Data (Pris	matic)Listed below (Recalc)
Elevatio (fee	on S	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
62.5	50	450	0	0	
63.0	00	700	288	288	
64.0	00	1,405	1,053	1,340	
Device	Routing	Invert	Outlet Devices		
#1	Primary	59.00'	12.0" Round (L= 25.0' CPP, Inlet / Outlet Inv n= 0.013 Corru	Culvert end-section con vert= 59.00' / 58.3 ugated PE, smoo	forming to fill, Ke= 0.500 38' S= 0.0048 '/' Cc= 0.900 th interior. Flow Area= 0.79 sf
#2	Device 1	63.00'	24.0" Horiz. O	rifice/Grate C=	0.600
#3	Device 1	59.30'	4.0" Round C L= 25.0' CPP, Inlet / Outlet In n= 0.010 PVC.	projecting, no he vert= 59.30' / 59. , smooth interior.	s eadwall, Ke= 0.900 18' S= 0.0048 '/' Cc= 0.900 Flow Area= 0.09 sf
#4	Device 3	62.50'	2.470 in/hr Ext	filtration over Su	irface area
Primary	OutFlow Ilvert (Pas	Max=2.05 cfs @ ses 2.05 cfs of 7) 12.09 hrs HW 2.29 cfs potentia	=63.21' (Free D I flow)	ischarge)

2=Orifice/Grate (Weir Controls 2.00 cfs @ 1.50 fps)
 3=Culvert (Passes 0.05 cfs of 0.64 cfs potential flow)
 4=Exfiltration (Exfiltration Controls 0.05 cfs)

Pond BIO2: BIO 2



Summary for Pond DMH: DMH 200

[57] Hint: Peaked at 54.87' (Flood elevation advised)

Inflow Area	a =	17,134 sf,	61.29% In	npervious,	Inflow Depth =	5.53"	for 100YF	R event
Inflow	=	2.20 cfs @	12.07 hrs,	Volume=	7,889 0	of		
Outflow	=	2.20 cfs @	12.07 hrs,	Volume=	7,889 0	of, Atten	= 0%, Lag	= 0.0 min
Primary	=	1.39 cfs @	12.07 hrs,	Volume=	7,239 0	of	-	
Routed	to Pond	TT1: Tree T	rench 1					
Secondary	=	0.81 cfs @	12.07 hrs,	Volume=	649 0	of		
Routed	to Pond	URC1 : URC	;-1					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 54.87' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	54.30'	12.0" Round Culvert L= 9.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 54.30' / 54.26' S= 0.0044 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior. Flow Area= 0.79 sf
#2	Primary	53.78'	12.0" Round Culvert L= 98.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 53.78' / 53.78' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=1.39 cfs @ 12.07 hrs HW=54.87' (Free Discharge) ←2=Culvert (Barrel Controls 1.39 cfs @ 2.03 fps)

Secondary OutFlow Max=0.81 cfs @ 12.07 hrs HW=54.87' (Free Discharge) -1=Culvert (Barrel Controls 0.81 cfs @ 2.53 fps)

Pond DMH: DMH 200



Summary for Pond RB1: RB 101,102

Inflow Ar Inflow Outflow Discarde	rea = (= (ed = (29,320 sf, 11.01).80 cfs @ 12.09 l).59 cfs @ 12.18 l).06 cfs @ 11.34 l	% Impervious, Inflow Depth = 1.64" for 100YR event hrs, Volume= 4,013 cf hrs, Volume= 4,014 cf, Atten= 26%, Lag= 5.5 min hrs, Volume= 3,084 cf				
Primary Route	= (ed to Pond S).53 cfs @ 12.18 l \$P3 : SP3	hrs, Volume= 929 cf				
Routing l Peak Ele	Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 46.58' @ 12.18 hrs Surf.Area= 157 sf Storage= 604 cf						
Plug-Flov Center-o	w detention f-Mass det.	time= (not calculat time= 69.4 min (89	ed: outflow precedes inflow) 97.5 - 828.1)				
Volume	Invert	Avail.Storage	Storage Description				
#1	41.00'	339 cf	6.00'D x 6.00'H Recharger x 2 Inside #2				
#2	39.00'	355 cf	10.00'D x 9.00'H Stone x 2				
			1,414 cf Overall - 339 cf Embedded = 1,074 cf x 33.0% Voids				
		694 cf	Total Available Storage				
Device	Routing	Invert Out	let Devices				
#1 #2	Discarded Primary	39.00' 8.2 7 46.50' 5.0' Hea 2.50 Coe 2.6	70 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01' Iong x 5.0' breadth Broad-Crested Rectangular Weir X 2.00 ad (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 0 3.00 3.50 4.00 4.50 5.00 5.50 ef. (English) 2.34 2.50 2.70 2.68 2.66 2.65 2.65 5 2.67 2.66 2.68 2.70 2.74 2.79 2.88				
Discarded OutFlow Max=0.06 cfs @ 11.34 hrs HW=39.09' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.06 cfs)							

Primary OutFlow Max=0.48 cfs @ 12.18 hrs HW=46.57' (Free Discharge) **2=Broad-Crested Rectangular Weir** (Weir Controls 0.48 cfs @ 0.64 fps)

Pond RB1: RB 101,102



Summary for Pond RB2: RB 202,202,203

[92] Warning: Device #2 is above defined storage

Inflow Area	a =	4,883 sf,	95.97% Im	pervious,	Inflow Depth =	4.00"	for 100)YR event
Inflow	=	0.91 cfs @	12.07 hrs, \	/olume=	1,628 cf			
Outflow	=	0.09 cfs @	11.61 hrs, \	/olume=	1,628 cf	, Atten	= 90%,	Lag= 0.0 min
Discarded	=	0.09 cfs @	11.61 hrs, \	/olume=	1,628 cf			-
Primary	=	0.00 cfs @	0.00 hrs, \	/olume=	0 cf			
Routed	to Pond	SP3 : SP3						

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 52.27' @ 12.60 hrs Surf.Area= 236 sf Storage= 932 cf

Plug-Flow detention time= 90.9 min calculated for 1,628 cf (100% of inflow) Center-of-Mass det. time= 90.9 min (826.6 - 735.7)

Volume	Invert	Avail.Stor	age	Storage Description
#1	46.50'	50)9 cf	6.00'D x 6.00'H Recharger x 3 Inside #2
#2	44.50'	53	32 cf	10.00'D x 9.00'H Stone x 3
				2,121 cf Overall - 509 cf Embedded = 1,612 cf x 33.0% Voids
		1,04	1 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1 #2	Discarded Primary	44.50' 55.61'	8.27 5.0' I Head 2.50 Coef 2.65	O in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01' Iong x 5.0' breadth Broad-Crested Rectangular Weir X 2.00 Add (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 3.00 3.50 4.00 4.50 5.00 5.50 5.00 5.61 2.66 2.65 2.65 2.65 2.65 2.65 2.65 2.65 2.65 2.65 2.66 2.68 2.70 2.88

Discarded OutFlow Max=0.09 cfs @ 11.61 hrs HW=44.61' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.09 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=44.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Pond RB2: RB 202,202,203



Summary for Pond RB3: RB 300

Inflow Are	ea =	22,132 sf, 0.0	00% Impervious, Inflow Depth = 0.91" for 100YR event		
Inflow	=	0.21 cfs @ 12.4	3 hrs, Volume= 1,675 cf		
Outflow	=	0.21 cfs @ 12.4	8 hrs, Volume= 1,677 cf, Atten= 1%, Lag= 2.7 min		
Discarde	d =	0.02 cfs @ 12.1	3 hrs, Volume= 1,025 cf		
Primary Route	= d to Pond s	0.18 cfs @ 12.4 SP2 : SP2	8 hrs, Volume= 652 cf		
Routing b Peak Ele	oy Stor-Ind v= 65.56' (method, Time Sp ② 12.48 hrs Sur	oan= 0.00-72.00 hrs, dt= 0.01 hrs / 3 f.Area= 57 sf Storage= 165 cf		
Plug-Flow Center-of	w detention f-Mass det.	time= 64.5 min o time= 65.2 min (calculated for 1,675 cf (100% of inflow) (1,009.0 - 943.8)		
Volume	Inver	t Avail.Storaç	ge Storage Description		
#1	59.50	' 50	cf 4.00'D x 4.00'H Recharger Inside #2		
#2	58.50	' 133	cf 6.00'D x 8.00'H Stone x 2		
			452 cf Overall - 50 cf Embedded = 402 cf x 33.0% Voids		
		183	cf Total Available Storage		
Device	Routing	Invert C	Dutlet Devices		
#1 #2	Discarded Primary	58.50' 8 65.50' 5 H 2 C 2	3.270 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01' 5.0' long x 5.0' breadth Broad-Crested Rectangular Weir lead (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.66 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88		
Discarded OutFlow Max=0.02 cfs @ 12.13 hrs HW=58.60' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.02 cfs)					

Primary OutFlow Max=0.17 cfs @ 12.48 hrs HW=65.56' (Free Discharge) 2=Broad-Crested Rectangular Weir (Weir Controls 0.17 cfs @ 0.57 fps)

Pond RB3: RB 300



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area	=	158,640 sf,	9.44% In	npervious,	Inflow Depth =	1.30"	for 10	0YR event
Inflow	=	=	2.86 cfs @	12.26 hrs,	Volume=	17,162 cf			
Primary	/ =	=	2.86 cfs @	12.26 hrs,	Volume=	17,162 cf	, Attei	n= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area	=	54,580 sf,	0.00% Imperv	vious, Ir	Inflow Depth =	0.68"	for 10	00YR event
Inflow		=	0.47 cfs @	12.48 hrs, Volu	ıme=	3,109 cf	•		
Primary	y	=	0.47 cfs @	12.48 hrs, Volu	ıme=	3,109 ct	f, Atten	= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	rea =	34,203 sf,	23.14% Impervious,	Inflow Depth = 0.33	" for 100YR event
Inflow	=	0.53 cfs @	12.18 hrs, Volume=	929 cf	
Primary	=	0.53 cfs @	12.18 hrs, Volume=	929 cf, At	ten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP3: SP3

Summary for Pond TT1: Tree Trench 1

[81] Warning: Exceeded Pond DMH by 0.26' @ 12.58 hrs

Inflow Area	a =	17,134 sf,	61.29% In	npervious,	Inflow Depth =	5.07"	for 100YR event
Inflow	=	1.39 cfs @	12.07 hrs,	Volume=	7,239 c	f	
Outflow	=	1.37 cfs @	12.09 hrs,	Volume=	7,240 c	f, Atter	n= 2%, Lag= 1.0 min
Discarded	=	0.12 cfs @	10.53 hrs,	Volume=	5,115 c	f	
Primary	=	1.25 cfs @	12.09 hrs,	Volume=	2,125 c	f	
Routed	to Pond	URC1 : URC	-1				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 54.89' @ 12.09 hrs Surf.Area= 639 sf Storage= 563 cf

Plug-Flow detention time= 17.2 min calculated for 7,239 cf (100% of inflow) Center-of-Mass det. time= 17.2 min (773.0 - 755.8)

Volume	Invert	Avail.Stora	ge	Storage Description
#1	52.34'	627	′ cf	6.20'W x 103.00'L x 3.00'H Prismatoid
				1,916 cf Overall - 32 cf Embedded = 1,884 cf x 33.3% Voids
#2	53.78'	32	cf	8.0" Round Pipe Storage Inside #1
				L= 92.0' S= 0.0050 '/'
		659) cf	Total Available Storage
Device	Routing	Invert (Outle	et Devices
#1	Discarded	52.34' 8	8.270) in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	54.30' 1	12.0'	Vert. Orifice/Grate C= 0.600
		L	Limite	ed to weir flow at low heads
Discard	ed OutFlow M	ax=0.12 cfs (@ 10	0.53 hrs HW=52.37' (Free Discharge)
└─1=Ex	filtration (Exfi	tration Contro	ols 0	.12 cfs)

Primary OutFlow Max=1.24 cfs @ 12.09 hrs HW=54.89' (Free Discharge) **2=Orifice/Grate** (Orifice Controls 1.24 cfs @ 2.61 fps)

Pond TT1: Tree Trench 1



Summary for Pond TT2: Tree Trench 2

[81] Warning: Exceeded Pond 200 by 0.96' @ 13.57 hrs

Inflow Area = 4,883 sf, 95.97% Impervious, Inflow Depth = 8.04" for 100YR event Inflow 0.94 cfs @ 12.07 hrs, Volume= = 3,272 cf 0.94 cfs @ 12.07 hrs, Volume= Outflow = 3,272 cf, Atten= 0%, Lag= 0.3 min 8.60 hrs, Volume= Discarded = 0.03 cfs @ 1,644 cf 0.91 cfs @ 12.07 hrs, Volume= Primary = 1,628 cf Routed to Pond RB2 : RB 202,202,203

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 53.19' @ 12.07 hrs Surf.Area= 150 sf Storage= 163 cf

Plug-Flow detention time= 20.4 min calculated for 3,271 cf (100% of inflow) Center-of-Mass det. time= 20.4 min (761.0 - 740.6)

Volume	Invert	Avail.Storag	e Storage Description
#1	50.16'	184 o	of 5.00'W x 30.00'L x 3.80'H Prismatoid
#2	51.66'	17 0	570 cf Overall - 17 cf Embedded = 553 cf x 33.3% Voids f 8.0'' Round Pipe Storage Inside #1 L= 50.0' S= 0.0050 '/'
		201 0	of Total Available Storage
Device	Routing	Invert O	utlet Devices
#1	Discarded	50.16' 8.	270 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	52.70' 1 2	2.0" Vert. Orifice/Grate C= 0.600
		Li	mited to weir flow at low heads
Discard	ed OutFlow M	ax=0.03 cfs @	8.60 hrs_HW=50.20' (Free Discharge)

1=Exfiltration (Exfiltration Controls 0.03 cfs)

Primary OutFlow Max=0.91 cfs @ 12.07 hrs HW=53.19' (Free Discharge) ←2=Orifice/Grate (Orifice Controls 0.91 cfs @ 2.38 fps)

Pond TT2: Tree Trench 2



Summary for Pond URC1: URC-1

[79] Warning: Submerged Pond BIO1 Primary device # 1 INLET by 0.67'
[81] Warning: Exceeded Pond DMH by 1.61' @ 13.37 hrs
[81] Warning: Exceeded Pond TT1 by 1.36' @ 13.39 hrs

 Inflow Area =
 65,568 sf, 45.75% Impervious, Inflow Depth = 3.38" for 100YR event

 Inflow =
 5.91 cfs @ 12.09 hrs, Volume=
 18,481 cf

 Outflow =
 0.40 cfs @ 11.52 hrs, Volume=
 18,481 cf, Atten= 93%, Lag= 0.0 min

 Discarded =
 0.40 cfs @ 11.52 hrs, Volume=
 18,481 cf

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 55.76' @ 13.31 hrs Surf.Area= 0.048 ac Storage= 0.173 af

Plug-Flow detention time= 145.7 min calculated for 18,478 cf (100% of inflow) Center-of-Mass det. time= 145.7 min (954.1 - 808.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	48.60'	0.087 af	16.33'W x 128.59'L x 7.25'H Field A
			0.350 af Overall - 0.087 af Embedded = 0.262 af x 33.3% Voids
#2A	50.60'	0.087 af	ADS_StormTech MC-3500 d +Cap x 34 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			34 Chambers in 2 Rows
			Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf
		0.175 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Discarded	48.60'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'
Discard	ed OutFlow M filtration (Exfi	lax=0.40 cfs Itration Cont	@ 11.52 hrs HW=48.67' (Free Discharge) trols 0.40 cfs)

Pond URC1: URC-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

17 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 125.59' Row Length +18.0" End Stone x 2 = 128.59' Base Length 2 Rows x 77.0" Wide + 6.0" Spacing x 1 + 18.0" Side Stone x 2 = 16.33' Base Width 24.0" Stone Base + 45.0" Chamber Height + 18.0" Stone Cover = 7.25' Field Height

34 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 2 Rows = 3,798.0 cf Chamber Storage

15,227.2 cf Field - 3,798.0 cf Chambers = 11,429.2 cf Stone x 33.3% Voids = 3,805.9 cf Stone Storage

Chamber Storage + Stone Storage = 7,603.9 cf = 0.175 af Overall Storage Efficiency = 49.9% Overall System Size = 128.59' x 16.33' x 7.25'

34 Chambers 564.0 cy Field 423.3 cy Stone





Pond URC1: URC-1



Summary for Pond URC2: URC-2

[79] Warning: Submerged Pond BIO2 Primary device # 1 INLET by 1.66'

Inflow Area	=	23,838 sf,	80.41% Imper	rvious, Inflov	v Depth =	6.96"	for 1	00YR event
Inflow	=	3.88 cfs @	12.08 hrs, Vol	lume=	13,834 c	f		
Outflow	=	0.29 cfs @	11.16 hrs, Vol	lume=	13,834 c	f, Atten	= 93%	6, Lag= 0.0 min
Discarded	=	0.29 cfs @	11.16 hrs, Vol	lume=	13,834 c	f		

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 60.66' @ 13.22 hrs Surf.Area= 1,499 sf Storage= 5,008 cf

Plug-Flow detention time= 127.5 min calculated for 13,833 cf (100% of inflow) Center-of-Mass det. time= 127.5 min (892.0 - 764.6)

Volume	Invert	Avail.Storage	Storage Description
#1A	54.50'	2,405 cf	36.08'W x 41.55'L x 6.75'H Field A
			10,120 cf Overall - 2,898 cf Embedded = 7,222 cf x 33.3% Voids
#2A	56.50'	2,898 cf	ADS_StormTech MC-3500 d +Cap x 25 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			25 Chambers in 5 Rows
			Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf
		5,303 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	54.50'	8.270 in/hr Exfiltration over Surface area	Phase-In= 0.01'

Discarded OutFlow Max=0.29 cfs @ 11.16 hrs HW=54.57' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.29 cfs)

Pond URC2: URC-2 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

5 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 39.55' Row Length +12.0" End Stone x 2 = 41.55' Base Length 5 Rows x 77.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 36.08' Base Width 24.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

25 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 5 Rows = 2,897.8 cf Chamber Storage

10,120.0 cf Field - 2,897.8 cf Chambers = 7,222.2 cf Stone x 33.3% Voids = 2,405.0 cf Stone Storage

Chamber Storage + Stone Storage = 5,302.8 cf = 0.122 af Overall Storage Efficiency = 52.4%Overall System Size = $41.55' \times 36.08' \times 6.75'$

25 Chambers 374.8 cy Field 267.5 cy Stone





Pond URC2: URC-2



Summary for Pond URC3: URC-3

Inflow Area	a =	9,762 sf,	100.00% Imperv	/ious, Inflo	ow Depth =	8.32"	for 100	YR event
Inflow	=	1.95 cfs @	12.07 hrs, Volu	ime=	6,768 c	of		
Outflow	=	0.13 cfs @	10.97 hrs, Volu	ime=	6,768 c	of, Atten	i= 93%,	Lag= 0.0 min
Discarded	=	0.13 cfs @	10.97 hrs, Volu	ime=	6,768 c	of		-

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 62.12' @ 13.25 hrs Surf.Area= 823 sf Storage= 2,533 cf

Plug-Flow detention time= 138.2 min calculated for 6,767 cf (100% of inflow) Center-of-Mass det. time= 138.2 min (877.7 - 739.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	56.30'	1,243 cf	23.25'W x 35.38'L x 6.25'H Field A
			5,141 cf Overall - 1,409 cf Embedded = 3,732 cf x 33.3% Voids
#2A	57.80'	1,409 cf	ADS_StormTech MC-3500 d +Cap x 12 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			12 Chambers in 3 Rows
			Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf
		2.652 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices		
#1	Discarded	56.30'	7.000 in/hr Exfiltration over Surface area	Phase-In= 0.01'	
Discarded OutFlow Max=0.13 cfs @ 10.97 hrs HW=56.36' (Free Discharge)					

1=Exfiltration (Exfiltration Controls 0.13 cfs)

Pond URC3: URC-3 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

4 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 32.38' Row Length +18.0" End Stone x 2 = 35.38' Base Length 3 Rows x 77.0" Wide + 6.0" Spacing x 2 + 18.0" Side Stone x 2 = 23.25' Base Width 18.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.25' Field Height

12 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 3 Rows = 1,408.8 cf Chamber Storage

5,141.2 cf Field - 1,408.8 cf Chambers = 3,732.3 cf Stone x 33.3% Voids = 1,242.9 cf Stone Storage

Chamber Storage + Stone Storage = 2,651.7 cf = 0.061 af Overall Storage Efficiency = 51.6%Overall System Size = $35.38' \times 23.25' \times 6.25'$

12 Chambers 190.4 cy Field 138.2 cy Stone





Pond URC3: URC-3



Time span=0.00-72.00 hrs, dt=0.01 hrs, 7201 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment DA1: DA1	Runoff Area=158,640 sf 9.44% Impervious Runoff Depth=0.00" Flow Length=417' Tc=14.5 min CN=38 Runoff=0.00 cfs 0 cf
Subcatchment DA2a: DA2a	Runoff Area=22,132 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=339' Tc=17.5 min CN=34 Runoff=0.00 cfs 0 cf
Subcatchment DA2b: DA2b pervious	Runoff Area=4,670 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=39 Runoff=0.00 cfs 0 cf
Subcatchment DA2bi: DA2b impervious	Runoff Area=9,894 sf 100.00% Impervious Runoff Depth=1.00" Tc=5.0 min CN=98 Runoff=0.26 cfs 821 cf
Subcatchment DA2c: DA2c	Runoff Area=32,448 sf 0.00% Impervious Runoff Depth=0.00" Flow Length=255' Tc=21.2 min CN=34 Runoff=0.00 cfs 0 cf
Subcatchment DA3a: DA3a pervious	Runoff Area=28,939 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=34 Runoff=0.00 cfs 0 cf
Subcatchment DA3ai: DA3a impervious	Runoff Area=19,495 sf 100.00% Impervious Runoff Depth=1.00" Tc=5.0 min CN=98 Runoff=0.51 cfs 1,617 cf
Subcatchment DA3b: DA3b pervious	Runoff Area=6,632 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=36 Runoff=0.00 cfs 0 cf
Subcatchment DA3bi: DA3b impervious	Runoff Area=10,502 sf 100.00% Impervious Runoff Depth=1.00" Tc=5.0 min CN=98 Runoff=0.28 cfs 871 cf
Subcatchment DA3c: DA3c pervious	Runoff Area=26,092 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=33 Runoff=0.00 cfs 0 cf
Subcatchment DA3ci: DA3c impervious	Runoff Area=3,228 sf 100.00% Impervious Runoff Depth=1.00" Tc=5.0 min CN=98 Runoff=0.08 cfs 268 cf
Subcatchment DA3d: DA3d pervious	Runoff Area=197 sf 0.00% Impervious Runoff Depth=0.00" Tc=5.0 min CN=39 Runoff=0.00 cfs 0 cf
Subcatchment DA3di: DA3d impervious	Runoff Area=4,686 sf 100.00% Impervious Runoff Depth=1.00" Tc=5.0 min CN=98 Runoff=0.12 cfs 389 cf
SubcatchmentR1E: EAST ROOF	Runoff Area=9,274 sf 100.00% Impervious Runoff Depth=1.00" Tc=5.0 min CN=98 Runoff=0.24 cfs 769 cf
SubcatchmentR1W: WEST ROOF	Runoff Area=9,762 sf 100.00% Impervious Runoff Depth=1.00" Tc=5.0 min CN=98 Runoff=0.26 cfs 810 cf
Pond 100: CB 100 12.0" R	Peak Elev=50.18' Inflow=0.08 cfs 268 cf ound Culvert n=0.013 L=4.0' S=0.0050 '/' Outflow=0.08 cfs 268 cf

19038-POST V4

Prepared by {enter your company name here} HydroCAD® 10.10-7a s/n 01445 © 2021 HydroCAD Software Solutions LLC

 Type III 24-hr
 WQv Rainfall=1.21"

 Printed
 12/12/2021

 LC
 Page 206

Pond 200: CB 200	Peak Elev=51.89' Inflow=0.12 cfs 389 cf Primary=0.12 cfs 389 cf Secondary=0.00 cfs 0 cf Outflow=0.12 cfs 389 cf
Pond BIO1: BIO 1	Peak Elev=58.98' Storage=517 cf Inflow=0.51 cfs 1,617 cf Outflow=0.08 cfs 1,617 cf
Pond BIO2: BIO 2	Peak Elev=62.96' Storage=260 cf Inflow=0.26 cfs 821 cf Outflow=0.04 cfs 821 cf
Pond DMH: DMH 200	Peak Elev=54.25' Inflow=0.28 cfs 871 cf Primary=0.28 cfs 871 cf Secondary=0.00 cfs 0 cf Outflow=0.28 cfs 871 cf
Pond RB1: RB 101,102	Peak Elev=39.23' Storage=12 cf Inflow=0.08 cfs 268 cf Discarded=0.06 cfs 268 cf Primary=0.00 cfs 0 cf Outflow=0.06 cfs 268 cf
Pond RB2: RB 202,202,203	Peak Elev=44.50' Storage=0 cf Inflow=0.00 cfs 0 cf Discarded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Pond RB3: RB 300	Peak Elev=58.50' Storage=0 cf Inflow=0.00 cfs 0 cf Discarded=0.00 cfs 0 cf Primary=0.00 cfs 0 cf Outflow=0.00 cfs 0 cf
Pond SP1: SP1	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Pond SP2: SP2	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Pond SP3: SP3	Inflow=0.00 cfs 0 cf Primary=0.00 cfs 0 cf
Pond TT1: Tree Trench 1	Peak Elev=52.72' Storage=82 cf Inflow=0.28 cfs 871 cf Discarded=0.12 cfs 871 cf Primary=0.00 cfs 0 cf Outflow=0.12 cfs 871 cf
Pond TT2: Tree Trench 2	Peak Elev=51.78' Storage=81 cf Inflow=0.12 cfs 389 cf Discarded=0.03 cfs 389 cf Primary=0.00 cfs 0 cf Outflow=0.03 cfs 389 cf
Pond URC1: URC-1	Peak Elev=48.61' Storage=0.000 af Inflow=0.08 cfs 1,617 cf Outflow=0.08 cfs 1,617 cf
Pond URC2: URC-2	Peak Elev=54.56' Storage=31 cf Inflow=0.28 cfs 1,590 cf Outflow=0.26 cfs 1,590 cf
Pond URC3: URC-3	Peak Elev=56.55' Storage=68 cf Inflow=0.26 cfs 810 cf Outflow=0.13 cfs 810 cf

Total Runoff Area = 346,591 sf Runoff Volume = 5,545 cf Average Runoff Depth = 0.19" 76.39% Pervious = 264,770 sf 23.61% Impervious = 81,821 sf
Summary for Subcatchment DA1: DA1

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP1 : SP1 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

A	rea (sf)	CN E	Description						
1	18,297	30 V	30 Woods, Good, HSG A						
	25,363	39 >	75% Gras	s cover, Go	ood, HSG A				
	9,642	98 F	Roofs, HSG	βA					
	5,338	98 F	aved park	ing, HSG A					
1	58,640	38 V	Veighted A	verage					
1	43,660	g	0.56% Per	vious Area					
	14,980	g	.44% Impe	ervious Area	а				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
10.6	50	0.1100	0.08		Sheet Flow, A TO B				
					Woods: Dense underbrush n= 0.800 P2= 3.33"				
0.2	28	0.2860	2.67		Shallow Concentrated Flow, B TO C				
					Woodland Kv= 5.0 fps				
0.7	99	0.1110	2.33		Shallow Concentrated Flow, C TO D				
					Short Grass Pasture Kv= 7.0 fps				
3.0	240	0.0690	1.31		Shallow Concentrated Flow, D to SP1				
					Woodland Kv= 5.0 fps				
14.5	417	Total							

Subcatchment DA1: DA1



Summary for Subcatchment DA2a: DA2a

CN for permeable pavers taken from RI Stormwater Design

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond RB3 : RB 300 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

	A	rea (sf)	CN	Description								
		12,073	30	Woods, Go	Voods, Good, HSG A							
		9,181	39	>75% Gras	s cover, Go	bod, HSG A						
*		878	40	Pervious Pa	avers							
		22,132	34	Weighted A	verage							
		22,132		100.00% Pe	ervious Are	a						
	_											
	Тс	Length	Slope	e Velocity	Capacity	Description						
(m	in)	(feet)	(ft/ft) (ft/sec)	(cfs)							
13	3.6	50	0.0600	0.06		Sheet Flow, A TO B						
						Woods: Dense underbrush n= 0.800 P2= 3.33"						
	2.5	164	0.0490) 1.11		Shallow Concentrated Flow, B to C						
						Woodland Kv= 5.0 fps						
	1.4	125	0.0480) 1.53		Shallow Concentrated Flow, C to D (RB 3)						
						Short Grass Pasture Kv= 7.0 fps						
1	7.5	339	Total									

Subcatchment DA2a: DA2a



0 cf, Depth= 0.00"

Summary for Subcatchment DA2b: DA2b pervious

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond BIO2 : BIO 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"



0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

Summary for Subcatchment DA2bi: DA2b impervious

Runoff = 0.26 cfs @ 12.07 hrs, Volume= 821 cf, Depth= 1.00" Routed to Pond BIO2 : BIO 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

A	rea (sf)	CN	Description					
	7,075	98	Paved park	ing, HSG A	N			
*	2,044	98	Sidewalks,	HŠG A				
	775	98	Water Surfa	ace, HSG A				
	9,894	98	Weighted Average					
	9,894		100.00% Impervious Area					
Tc (min)	Length (feet)	Slop (ft/ff	e Velocity t) (ft/sec)	Capacity (cfs)	Description			
5.0					Direct Entry, Direct			

Subcatchment DA2bi: DA2b impervious



Summary for Subcatchment DA2c: DA2c

CN for permeable pavers taken from RI Stormwater Design

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond SP2 : SP2 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

Α	rea (sf)	CN D	Description						
	18,364	30 V	30 Woods, Good, HSG A						
	14,084	39 >	75% Gras	s cover, Go	ood, HSG A				
	32,448	34 V	Veighted A	verage					
	32,448	1	00.00% Pe	ervious Are	а				
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
17.9	50	0.0300	0.05		Sheet Flow, A TO B				
					Woods: Dense underbrush n= 0.800 P2= 3.33"				
1.2	50	0.0200	0.71		Shallow Concentrated Flow, B to C				
					Woodland Kv= 5.0 fps				
2.1	155	0.0320	1.25		Shallow Concentrated Flow, C to SP2				
					Short Grass Pasture Kv= 7.0 fps				
21.2	255	Total							

Subcatchment DA2c: DA2c



Summary for Subcatchment DA3a: DA3a pervious

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond BIO1 : BIO 1 0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

A	rea (sf)	CN	Description		
	12,304	39	>75% Gras	s cover, Go	bod, HSG A
	16,635	30	Woods, Go	od, HSG A	
	28,939	34	Weighted A	verage	
	28,939		100.00% Pe	ervious Are	a
Tc	Length	Slop	e Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/f	t) (ft/sec)	(cfs)	
5.0					Direct Entry, Direct
					•

Subcatchment DA3a: DA3a pervious



Summary for Subcatchment DA3ai: DA3a impervious

Runoff = 0.51 cfs @ 12.07 hrs, Volume= 1,617 cf, Depth= 1.00" Routed to Pond BIO1 : BIO 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

A	Area (sf)	CN	Description					
	16,965	98	Paved park	ing, HSG A	N N N N N N N N N N N N N N N N N N N			
*	1,223	98	Sidewalk, H	ISG A				
1,307 98 Water Surface, HSG A								
	19,495	98	98 Weighted Average					
	19,495		100.00% In	pervious A	rea			
-		~		.				
IC	Length	Slope	e Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft	:) (ft/sec)	(cfs)				
5.0					Direct Entry, Direct			

Subcatchment DA3ai: DA3a impervious



Summary for Subcatchment DA3b: DA3b pervious

[45] Hint: Runoff=Zero

Runoff 0.00 hrs, Volume= = 0.00 cfs @ Routed to Pond DMH : DMH 200

0 cf, Depth= 0.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

Area (sf)	CN	Description						
2,453	39	>75% Grass cover, Good, HSG A						
3,792	30	Woods, Good, HSG A						
387	76	Gravel roads, HSG A						
6,632	36	Weighted Average	_					
6,632		100.00% Pervious Area						
Tc Length (min) (feet)	Slor (ft/	pe Velocity Capacity Description /ft) (ft/sec) (cfs)						
5.0		Direct Entry, Direct						
Subcatchment DA3b: DA3b pervious								



Time (hours)

Summary for Subcatchment DA3bi: DA3b impervious

Runoff	=	0.28 cfs @	12.07 hrs,	Volume=
Route	d to Po	nd DMH : DMH	200	

871 cf, Depth= 1.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

A	Area (sf)	CN	Description					
	9,746	98	Paved parking, HSG A					
*	756	98	Sidewalks,	HŠG A				
	10,502	98	8 Weighted Average					
	10,502		100.00% Impervious Area					
Tc (min)	Length (feet)	Slop (ft/fl	e Velocity t) (ft/sec)	Capacity (cfs)	Description			
5.0	· /			X /	Direct Entry, Direct			

Subcatchment DA3bi: DA3b impervious



0 cf, Depth= 0.00"

Summary for Subcatchment DA3c: DA3c pervious

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond 100 : CB 100

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

Area (s	f) C	N D	escription		
10,02	27 3	39 >	75% Gras	s cover, Go	bod, HSG A
16,06	5 3	30 V	Voods, Go	od, HSG A	
26,09)2 3	33 V	Veighted A	verage	
26,09	2	1	00.00% Pe	ervious Are	а
Tc Leng	gth S	Slope	Velocity	Capacity	Description
<u>(min)</u> (fe	et)	(ft/ft)	(ft/sec)	(cfs)	
5.0					Direct Entry, Direct

Subcatchment DA3c: DA3c pervious



268 cf, Depth= 1.00"

Summary for Subcatchment DA3ci: DA3c impervious

Runoff = 0.08 cfs @ 12.07 hrs, Volume= Routed to Pond 100 : CB 100

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

Α	rea (sf)	CN	Description					
	2,698	98	Paved parking, HSG A					
*	530	98	Sidewalks,	HŠG A				
	3,228	98	Weighted Average					
	3,228		100.00% Impervious Area					
Тс	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)				
5.0					Direct Entry, Direct			

Subcatchment DA3ci: DA3c impervious



0 cf, Depth= 0.00"

Summary for Subcatchment DA3d: DA3d pervious

[45] Hint: Runoff=Zero

Runoff = 0.00 cfs @ 0.00 hrs, Volume= Routed to Pond 200 : CB 200

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"



Summary for Subcatchment DA3di: DA3d impervious

Runoff = 0.12 cfs @ 12.07 hrs, Volume= 389 cf, Depth= 1.00" Routed to Pond 200 : CB 200

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"

A	Area (sf)	CN D	escription							
	4,686	98 P	aved park	ing, HSG A	4					
	4,686	1(00.00% In	npervious A	Area					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
5.0					Direct Entry,					
			Subcat	tchment	DA3di: DA3d impervious					
	Hydrograph									
0.13		0.12 cfs								
0.12					Type III 24-hr					
0.11					WQv Rainfall=1.21"					
0.1					\mathbf{P} upoff \mathbf{A} roo $\pm \mathbf{A}$ 696 cf					
0.09					Runon Area-4,000 SI					
^{0.08} و					Runoff Volume=389 cf					
ک 0.07 م					Runoff Depth=1.00"					
6 10.06					$T_{c}=5.0$ min					
0.05										
0.04					CN=98					
0.03										
0.02				+ - + - + - + - + - + - + - +						
0.01										
0			finiti finiti finiti							
	0 2 4 6 8	3 10 12 14 1	6 18 20 22 24	26 28 30 32 3 Tim	34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 ne (hours)					

Summary for Subcatchment R1E: EAST ROOF

Runoff = 0.24 cfs @ 12.07 hrs, Volume= Routed to Pond URC2 : URC-2 769 cf, Depth= 1.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"



Summary for Subcatchment R1W: WEST ROOF

Runoff = 0.26 cfs @ 12.07 hrs, Volume= Routed to Pond URC3 : URC-3 810 cf, Depth= 1.00"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Type III 24-hr WQv Rainfall=1.21"



Inflow
Primary

Summary for Pond 100: CB 100

[57] Hint: Peaked at 50.18' (Flood elevation advised)

(cfs)

Flow

0.02-0.015-0.01-0.005-0-

Inflow Area	a =	29,320 sf,	11.01% Imp	ervious,	Inflow Depth =	0.11"	for WQv event
Inflow	=	0.08 cfs @	12.07 hrs, V	'olume=	268 c	f	
Outflow	=	0.08 cfs @	12.07 hrs, V	′olume=	268 c	f, Atten	= 0%, Lag= 0.0 min
Primary	=	0.08 cfs @	12.07 hrs, V	'olume=	268 c	f	•
Routed	to Pond	RB1 : RB 10	1,102				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 50.18' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	50.00'	12.0" Round Culvert L= 4.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 50.00' / 49.98' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.08 cfs @ 12.07 hrs HW=50.18' (Free Discharge) ☐ 1=Culvert (Barrel Controls 0.08 cfs @ 1.32 fps)



0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

Pond 100: CB 100

Summary for Pond 200: CB 200

[57] Hint: Peaked at 51.89' (Flood elevation advised)

Inflow Area	a =	4,883 sf,	95.97% In	npervious,	Inflow Depth = ().96"	for WQv event	
Inflow	=	0.12 cfs @	12.07 hrs,	Volume=	389 cf			
Outflow	=	0.12 cfs @	12.07 hrs,	Volume=	389 cf,	Atten=	= 0%, Lag= 0.0 mi	n
Primary	=	0.12 cfs @	12.07 hrs,	Volume=	389 cf		•	
Routed	to Pond	TT2 : Tree T	rench 2					
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	0 cf			
Routed	to Pond	RB2 : RB 20	2,202,203					

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 51.89' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	52.70'	12.0" Round Culvert L= 4.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 52.70' / 52.68' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE smooth interior. Flow Area= 0.79 sf
#2	Primary	51.66'	8.0" Round Culvert L= 50.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 51.66' / 51.41' S= 0.0050 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.35 sf
#3	Secondary	55.79'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=0.12 cfs @ 12.07 hrs HW=51.89' (Free Discharge) ←2=Culvert (Barrel Controls 0.12 cfs @ 1.67 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=51.66' (Free Discharge) -1=Culvert (Controls 0.00 cfs) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 200: CB 200



Summary for Pond BIO1: BIO 1

Inflow Are Inflow Outflow Primary Route	ea = = = = ed to Pond	48,434 sf, 40 0.51 cfs @ 12 0.08 cfs @ 12 0.08 cfs @ 12 URC1 : URC-1	0.25% Imperviou .07 hrs, Volume .55 hrs, Volume .55 hrs, Volume	s, Inflow Dep = 1,6 = 1,6 = 1,6	th = 0.4 517 cf 517 cf, A 517 cf	0" f	for W 85%,	Qv even , Lag= 2	ıt 28.5 min
Routing b Peak Ele	oy Stor-Ind v= 58.98' (method, Time 3 @ 12.55 hrs S	Span= 0.00-72.00 urf.Area= 1,357 s) hrs, dt= 0.01 f Storage= 5	l hrs 517 cf				
Plug-Flov Center-of	w detentior f-Mass det	n time= 49.6 mir . time= 49.6 mir	n calculated for 1 n (830.5 - 780.8	.617 cf (100%)	of inflow	/)			
Volume	Inver	t Avail.Stor	age Storage De	escription					
#1	58.50)' 2,21	0 cf Custom S	tage Data (Pr	rismatic)	Liste	d belo	w (Reca	ılc)
Elevatior (feet	n S	Surf.Area (sa-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)					
58.50	<u>,</u> 0	800	0						
59.00	0	1,380	545	545					
60.00	0	1,950	1,665	2,210					
Device	Routing	Invert	Outlet Devices						
#1	Primary	55.09'	12.0" Round C L= $45.0'$ CPP, J Inlet / Outlet Invo	ulvert projecting, no ert= 55.09' / 5 pated PE smo	headwal 4.87' S=	ll, Ke = 0.00	= 0.90)49 '/' Flow A)0 Cc= 0. Area= 0	900 79 sf
#2	Device 1	59.25'	24.0" Horiz. Ori	fice/Grate C	>= 0.600 ads	nor, i			
#3	Device 1	55.38'	Limited to weir flow at low heads 4.0" Round Culvert L= 38.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 55.38' / 55.19' S= 0.0050 '/' Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.09 sf						
#4	Device 3	58.50'	2.470 in/hr Exfi	tration over	Surface	area			
Primary	OutFlow	Max=0.08 cfs @) 12.55 hrs HW=	58.98' (Free	Discharç	ge)			

1=Culvert (Passes 0.08 cfs of 5.50 cfs potential flow)

-2=Orifice/Grate (Controls 0.00 cfs)
 -3=Culvert (Passes 0.08 cfs of 0.58 cfs potential flow)
 -4=Exfiltration (Exfiltration Controls 0.08 cfs)

Pond BIO1: BIO 1



Summary for Pond BIO2: BIO 2

Inflow An Inflow Outflow Primary Route	rea = = = = ed to Pond	14,564 sf, 6 0.26 cfs @ 12 0.04 cfs @ 12 0.04 cfs @ 12 URC2 : URC-2	7.93% Imperviou 207 hrs, Volume 2.55 hrs, Volume 2.55 hrs, Volume	ıs, Inflow Dep ș= ș= ș=	oth = 0.6 821 cf 821 cf, <i>A</i> 821 cf	8" for V Atten= 85%	VQv event 6, Lag= 28.7 min	
Routing Peak Ele	by Stor-Ind ev= 62.96' (method, Time 3 @ 12.55 hrs S	Span= 0.00-72.0 urf.Area= 680 sf	0 hrs, dt= 0.0 Storage= 26	1 hrs 60 cf			
Plug-Flo Center-c	w detentior of-Mass det	n time= 48.3 mir . time= 48.3 mir	n calculated for 8 n (829.1 - 780.8	321 cf (100% o)	of inflow)			
Volume	Inver	t Avail.Stor	age Storage D	escription				
#1	62.50)' 1,34	0 cf Custom S	Stage Data (P	rismatic)	Listed bel	ow (Recalc)	
Elevatio (fee	on S et)	Surf.Area (sɑ-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)				
62.5	50	450	0	0				
63.0	00	700	288	288				
64.0	00	1,405	1,053	1,340				
Device	Routing	Invert	Outlet Devices					
#1	Primary	59.00'	12.0" Round C L= 25.0' CPP, Inlet / Outlet Inv n= 0.013 Corru	culvert end-section c ert= 59.00' / t igated PE. sm	conformin 58.88' Si looth inte	g to fill, K = 0.0048 '/ rior. Flow	e= 0.500 /' Cc= 0.900 Area= 0.79 sf	
#2	Device 1	63.00'	24.0" Horiz. Or	ifice/Grate (C = 0.600	,		
#3	Device 1	59.30'	Limited to weir flow at low heads 4.0" Round Culvert L= 25.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 59.30' / 59.18' S= 0.0048 '/' Cc= 0.900 n= 0.010 PVC smooth interior. Flow Area= 0.09 sf					
#4	Device 3	62.50'	2.470 in/hr Exf	iltration over	Surface	area		
Primary	Primary OutFlow Max=0.04 cfs @ 12.55 hrs HW=62.96' (Free Discharge)							

-2=Orifice/Grate (Controls 0.00 cfs) -3=Culvert (Passes 0.04 cfs of 0.62 cfs potential flow) -4=Exfiltration (Exfiltration Controls 0.04 cfs)

Pond BIO2: BIO 2



Summary for Pond DMH: DMH 200

[57] Hint: Peaked at 54.25' (Flood elevation advised)

Inflow Area	a =	17,134 sf,	61.29% In	npervious,	Inflow Depth = 0).61" fc	or WQv event
Inflow	=	0.28 cfs @	12.07 hrs,	Volume=	871 cf		
Outflow	=	0.28 cfs @	12.07 hrs,	Volume=	871 cf,	Atten=	0%, Lag= 0.0 min
Primary	=	0.28 cfs @	12.07 hrs,	Volume=	871 cf		-
Routed	to Pond	TT1: Tree T	rench 1				
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	0 cf		
Routed	to Pond	URC1 : URC	;-1				

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 54.25' @ 12.07 hrs

Device	Routing	Invert	Outlet Devices
#1	Secondary	54.30'	12.0" Round Culvert L= 9.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 54 30' / 54 26' S= 0.0044 '/' Cc= 0.900
#2	Primary	53.78'	n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf 12.0" Round Culvert L= 98.0' CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 53.78' / 53.78' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.79 sf

Primary OutFlow Max=0.28 cfs @ 12.07 hrs HW=54.25' (Free Discharge) →2=Culvert (Barrel Controls 0.28 cfs @ 1.11 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=53.78' (Free Discharge)

Pond DMH: DMH 200



Summary for Pond RB1: RB 101,102

Inflow Ar	rea = 2	29,320 sf, 1	1.01%	6 Impervious, Inflow Depth = 0.11" for WQv event		
Inflow	= 0.0	8 cfs @ 12	2.07 hi	rs, Volume= 268 cf		
Outflow	= 0.0	6 cfs @ 12	2.03 hi	rs, Volume= 268 cf, Atten= 29%, Lag= 0.0 min		
Discarde	ed = 0.0	6 cfs @ 12	2.03 hi	rs, Volume= 268 cf		
Primarv	= 0.0	0 cfs @ 0).00 hi	rs. Volume= 0 cf		
Route	ed to Pond SP	3 : SP3				
Routina	bv Stor-Ind me	ethod. Time	Span=	= 0.00-72.00 hrs. dt= 0.01 hrs / 3		
Peak Fle	ev= 39 23' @ 1	2 14 hrs S	urf Ar	ea= 157 sf Storage= 12 cf		
		2	on market and			
Plug-Flo	w detention tin	ne= 1.5 min	calcul	lated for 268 cf (100% of inflow)		
Center-o	f-Mass det tin	ne=1.5 min	(782	3 - 780 8)		
e entrer e			(
Volume	Invert	Avail.Stor	age	Storage Description		
#1	41.00'	33	9 cf	6.00'D x 6.00'H Recharger x 2 Inside #2		
#2	39.00'	35	5 cf	10.00'D x 9.00'H Stone x 2		
				1,414 cf Overall - 339 cf Embedded = 1,074 cf x 33.0% Voids		
		69	4 cf	Total Available Storage		
				5		
Device	Routing	Invert	Outle	et Devices		
#1	Discarded	39.00'	8.270	0 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01'		
#2	Primary	46.50'	5.0' I	ong x 5.0' breadth Broad-Crested Rectangular Weir X 2.00		
	,		Head	d (feet) 0 20 0 40 0 60 0 80 1 00 1 20 1 40 1 60 1 80 2 00		
			2 50	3 00 3 50 4 00 4 50 5 00 5 50		
			Coof	(English) 234 250 270 268 268 266 265 265 265		
			2 65	267 266 268 270 274 270 288		
			2.00	2.01 2.00 2.00 2.10 2.14 2.13 2.00		
Discarded OutFlow Max=0.06 cfs @ 12.03 hrs HW=39.09' (Free Discharge)						

1=Exfiltration (Exfiltration Controls 0.06 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=39.00' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Pond RB1: RB 101,102



Summary for Pond RB2: RB 202,202,203

[92] Warning: Device #2 is above defined storage

Inflow Area	a =	4,883 sf,	95.97% Impervious,	Inflow Depth = 0.00" for WQv eve	ent
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Outflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atten= 0%, Lag=	0.0 min
Discarded	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Routed	to Pond	SP3 : SP3			

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 44.50' @ 0.00 hrs Surf.Area= 236 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Stor	rage	Storage Description					
#1	46.50'	50)9 cf	6.00'D x 6.00'H Recharger x 3 Inside #2					
#2	44.50	00		2,121 cf Overall - 509 cf Embedded = $1,612$ cf x 33.0% Voids					
		1,04	1 cf	cf Total Available Storage					
Device	Routing	Invert	Outle	et Devices					
#1 #2	Discarded Primary	44.50' 55.61'	8.27 5.0' I Head 2.50 Coef 2.65	0 in/hr Exfiltration X 2.00 over Surface area Phase-In= 0.01' long x 5.0' breadth Broad-Crested Rectangular Weir X 2.00 d (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 3.00 3.50 4.00 4.50 5.00 5.50 f. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88					

Discarded OutFlow Max=0.00 cfs @ 0.00 hrs HW=44.50' (Free Discharge) **1=Exfiltration** (Controls 0.00 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=44.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs) Pond RB2: RB 202,202,203



Summary for Pond RB3: RB 300

Inflow Are	ea =	22,132 sf,	0.00%	Impervious, Inflov	v Depth = 0.00	for WQv event
Inflow	=	0.00 cfs @ (0.00 hrs	, Volume=	0 cf	
Outflow	=	0.00 cfs @ (0.00 hrs	, Volume=	0 cf, Att	en= 0%, Lag= 0.0 min
Discarded	= t	0.00 cfs @ (0.00 hrs	, Volume=	0 cf	, C
Primary	= d to Dond	0.00 cfs @ (0.00 hrs	, Volume=	0 cf	
Rouled		372.372				
Routina b	v Stor-Ind	method. Time	Span=	0.00-72.00 hrs. dt	= 0.01 hrs / 3	
Peak Elev	v= 58.50' (@ 0.00 hrs Su	urf.Area	= 57 sf Storage=	0 cf	
Plua-Flow	v detentior	n time= (not cal	culated	: initial storage ex	ceeds outflow)	
Center-of	-Mass det	time= (not cal	culated	: no inflow)		
				·		
Volume	Inver	t Avail.Sto	rage	Storage Descriptio	n	
#1	59.50)' <u>5</u>	50 cf	4.00'D x 4.00'H Re	echarger Inside	#2
#2	58.50	י 13	33 cf	5.00'D x 8.00'H St	one x 2	
				<u> 152 cf Overall - 50</u>	cf Embedded =	402 cf x 33.0% Voids
		18	33 cf	Fotal Available Sto	orage	
Device	Routina	Invert	Outlet	Devices		
#1	Discarded	58 50'	8.270	in/hr Exfiltration	X 2.00 over Su	face area Phase-In= 0.01'
#2	Primary	65 50'	5.0' lo	ng x 5 0' breadtl	n Broad-Creste	d Rectangular Weir
	. mary	00.00	Head	(feet) 0 20 0 40	0 60 0 80 1 00	1 20 1 40 1 60 1 80 2 00
			2 50	3003504004	50 5 00 5 50	
			Coef	(English) 2.34 2	50 2 70 2 68 2	68 2 66 2 65 2 65 2 65
			2.65	2.67 2.66 2.68 2	.70 2.74 2.79	2.88
Discarde	d OutFlow	v Max=0.00 cfs Controls 0.00 c	s @ 0.0 cfs)	0 hrs HW=58.50'	(Free Discharg	le)
			,			

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=58.50' (Free Discharge) ←2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond RB3: RB 300



Summary for Pond SP1: SP1

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	rea =	158,640 sf,	9.44% Impervious,	Inflow Depth = 0.00"	for WQv event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP1: SP1

Time (hours)

Summary for Pond SP2: SP2

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	rea =	54,580 sf,	0.00% Impervious,	Inflow Depth = 0.00"	for WQv event
Inflow	=	0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	=	0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP2: SP2

Time (hours)

Summary for Pond SP3: SP3

[40] Hint: Not Described (Outflow=Inflow)

Inflow A	Area =	=	34,203 sf,	23.14% Impervious,	Inflow Depth = 0.00"	for WQv event
Inflow	=		0.00 cfs @	0.00 hrs, Volume=	0 cf	
Primary	/ =		0.00 cfs @	0.00 hrs, Volume=	0 cf, Atter	n= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs



Pond SP3: SP3
Summary for Pond TT1: Tree Trench 1

Inflow Ar	ea =	17,134 sf, 61	1.29%	6 Impervious, Inflow Depth = 0.61" for WQv event
Inflow	= 0	.28 cfs @ 12.	.07 hi	rs, Volume= 871 cf
Outflow	= 0	.12 cfs @ 11.	96 hi	rs, Volume= 871 cf, Atten= 56%, Lag= 0.0 min
Discarde	d = 0	.12 cfs @ 11.	.96 hi	rs, Volume= 871 cf
Primary Route	= 0 ed to Pond U	.00 cfs @ 0. RC1 : URC-1	.00 hi	rs, Volume= 0 cf
Routing I Peak Ele	oy Stor-Ind n v= 52.72' @	nethod, Time S 12.23 hrs St	Span= urf.Ar	= 0.00-72.00 hrs, dt= 0.01 hrs / 3 ea= 639 sf Storage= 82 cf
Plug-Flov Center-o	w detention t f-Mass det. t	time= 3.1 min (time= 3.0 min (calcu (783	lated for 871 cf (100% of inflow) .9 - 780.8)
Volume	Invert	Avail.Stora	age	Storage Description
#1	52.34'	62	7 cf	6.20'W x 103.00'L x 3.00'H Prismatoid 1.916 cf Overall - 32 cf Embedded = 1.884 cf x 33.3% Voids
#2	53.78'	32	2 cf	8.0" Round Pipe Storage Inside #1 L= 92.0' S= 0.0050 '/'
		659	9 cf	Total Available Storage
Device	Routing	Invert	Outle	et Devices
#1 #2	Discarded Primary	52.34' 54.30'	8.27 12.0 Limit	0 in/hr Exfiltration over Surface area Phase-In= 0.01' ' Vert. Orifice/Grate C= 0.600 red to weir flow at low heads
Discarde	ed OutFlow filtration (E>	Max=0.12 cfs xfiltration Cont	@ 1 ⁻ rols 0	1.96 hrs HW=52.37' (Free Discharge) 0.12 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=52.34' (Free Discharge) ←2=Orifice/Grate (Controls 0.00 cfs)

Pond TT1: Tree Trench 1



Summary for Pond TT2: Tree Trench 2

[81] Warning: Exceeded Pond 200 by 0.01' @ 12.48 hrs

Inflow Area = 4,883 sf, 95.97% Impervious, Inflow Depth = 0.96" for WQv event Inflow 0.12 cfs @ 12.07 hrs, Volume= = 389 cf 0.03 cfs @ 11.75 hrs, Volume= Outflow = 389 cf, Atten= 77%, Lag= 0.0 min Discarded = 0.03 cfs @ 11.75 hrs, Volume= 389 cf Primary = 0.00 cfs @ 0.00 hrs, Volume= 0 cf Routed to Pond RB2 : RB 202,202,203

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs / 3 Peak Elev= 51.78' @ 12.45 hrs Surf.Area= 150 sf Storage= 81 cf

Plug-Flow detention time= 14.2 min calculated for 389 cf (100% of inflow) Center-of-Mass det. time= 14.2 min (795.0 - 780.8)

Volume	Invert	Avail.Storage	e Storage Description
#1	50.16'	184 c	of 5.00'W x 30.00'L x 3.80'H Prismatoid
#2	51.66'	17 c	570 cf Overall - 17 cf Embedded = 553 cf x 33.3% Voids f 8.0'' Round Pipe Storage Inside #1 L= 50.0' S= 0.0050 '/'
		201 c	of Total Available Storage
Device	Routing	Invert Ou	utlet Devices
#1	Discarded	50.16' 8. 2	270 in/hr Exfiltration over Surface area Phase-In= 0.01'
#2	Primary	52.70' 12 Lii	2.0" Vert. Orifice/Grate C= 0.600 mited to weir flow at low heads
Discard	ed OutFlow M filtration (Exfil	ax=0.03 cfs @ tration Control) 11.75 hrs HW=50.20' (Free Discharge) s 0.03 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=50.16' (Free Discharge) ←2=Orifice/Grate (Controls 0.00 cfs) Pond TT2: Tree Trench 2



Summary for Pond URC1: URC-1

Inflow Area	ı =	65,568 sf,	45.75% Impervious,	Inflow Depth = 0.3	0" for WQv event
Inflow	=	0.08 cfs @	12.55 hrs, Volume=	1,617 cf	
Outflow	=	0.08 cfs @	12.59 hrs, Volume=	1,617 cf, A	tten= 0%, Lag= 2.4 min
Discarded	=	0.08 cfs @	12.59 hrs, Volume=	1,617 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 48.61' @ 12.59 hrs Surf.Area= 0.048 ac Storage= 0.000 af

Plug-Flow detention time= 2.1 min calculated for 1,617 cf (100% of inflow) Center-of-Mass det. time= 2.1 min (832.6 - 830.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	48.60'	0.087 af	16.33'W x 128.59'L x 7.25'H Field A
			0.350 af Overall - 0.087 af Embedded = 0.262 af x 33.3% Voids
#2A	50.60'	0.087 af	ADS_StormTech MC-3500 d +Cap x 34 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			34 Chambers in 2 Rows
			Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf
		0.175 af	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	48.60'	8.270 in/hr Exfiltration over Surface area Phase-In= 0.01'	

Discarded OutFlow Max=0.40 cfs @ 12.59 hrs HW=48.61' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.40 cfs)

Pond URC1: URC-1 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cfOverall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 2 rows = 59.6 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

17 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 125.59' Row Length +18.0" End Stone x 2 = 128.59' Base Length 2 Rows x 77.0" Wide + 6.0" Spacing x 1 + 18.0" Side Stone x 2 = 16.33' Base Width 24.0" Stone Base + 45.0" Chamber Height + 18.0" Stone Cover = 7.25' Field Height

34 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 2 Rows = 3,798.0 cf Chamber Storage

15,227.2 cf Field - 3,798.0 cf Chambers = 11,429.2 cf Stone x 33.3% Voids = 3,805.9 cf Stone Storage

Chamber Storage + Stone Storage = 7,603.9 cf = 0.175 af Overall Storage Efficiency = 49.9% Overall System Size = 128.59' x 16.33' x 7.25'

34 Chambers 564.0 cy Field 423.3 cy Stone





Pond URC1: URC-1



Summary for Pond URC2: URC-2

Inflow Area	=	23,838 sf,	80.41% Impervious,	Inflow Depth = 0.80	" for WQv event
Inflow	=	0.28 cfs @	12.07 hrs, Volume=	1,590 cf	
Outflow	=	0.26 cfs @	12.10 hrs, Volume=	1,590 cf, At	ten= 6%, Lag= 1.7 min
Discarded	=	0.26 cfs @	12.10 hrs, Volume=	1,590 cf	

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 54.56' @ 12.10 hrs Surf.Area= 1,499 sf Storage= 31 cf

Plug-Flow detention time= 2.0 min calculated for 1,590 cf (100% of inflow) Center-of-Mass det. time= 2.0 min (807.7 - 805.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	54.50'	2,405 cf	36.08'W x 41.55'L x 6.75'H Field A
			10,120 cf Overall - 2,898 cf Embedded = 7,222 cf x 33.3% Voids
#2A	56.50'	2,898 cf	ADS_StormTech MC-3500 d +Cap x 25 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			25 Chambers in 5 Rows
			Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf
		5,303 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	54.50'	8.270 in/hr Exfiltration over Surface area	Phase-In= 0.01'
Discord		1-x-0.00 of	\bigcirc 10 10 hm $\downarrow N = 54 561 (Error Discharge)$	

Discarded OutFlow Max=0.29 cfs @ 12.10 hrs HW=54.56' (Free Discharge) **1=Exfiltration** (Exfiltration Controls 0.29 cfs)

Pond URC2: URC-2 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 5 rows = 149.0 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

5 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 39.55' Row Length +12.0" End Stone x 2 = 41.55' Base Length 5 Rows x 77.0" Wide + 6.0" Spacing x 4 + 12.0" Side Stone x 2 = 36.08' Base Width 24.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.75' Field Height

25 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 5 Rows = 2,897.8 cf Chamber Storage

10,120.0 cf Field - 2,897.8 cf Chambers = 7,222.2 cf Stone x 33.3% Voids = 2,405.0 cf Stone Storage

Chamber Storage + Stone Storage = 5,302.8 cf = 0.122 af Overall Storage Efficiency = 52.4%Overall System Size = $41.55' \times 36.08' \times 6.75'$

25 Chambers 374.8 cy Field 267.5 cy Stone





Pond URC2: URC-2



Summary for Pond URC3: URC-3

Inflow Area	a =	9,762 sf,	100.00% Impervious	, Inflow Depth = ⁻	1.00" for WQv event
Inflow	=	0.26 cfs @	12.07 hrs, Volume=	810 cf	
Outflow	=	0.13 cfs @	12.00 hrs, Volume=	810 cf,	Atten= 48%, Lag= 0.0 min
Discarded	=	0.13 cfs @	12.00 hrs, Volume=	810 cf	-

Routing by Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.01 hrs Peak Elev= 56.55' @ 12.19 hrs Surf.Area= 823 sf Storage= 68 cf

Plug-Flow detention time= 3.3 min calculated for 810 cf (100% of inflow) Center-of-Mass det. time= 3.3 min (784.1 - 780.8)

Volume	Invert	Avail.Storage	Storage Description
#1A	56.30'	1,243 cf	23.25'W x 35.38'L x 6.25'H Field A
			5,141 cf Overall - 1,409 cf Embedded = 3,732 cf x 33.3% Voids
#2A	57.80'	1,409 cf	ADS_StormTech MC-3500 d +Cap x 12 Inside #1
			Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf
			Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap
			12 Chambers in 3 Rows
			Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf
		2.652 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices	
#1	Discarded	56.30'	7.000 in/hr Exfiltration over Surface area	Phase-In= 0.01'
Discard	led OutFlow	Max=0.13 cfs	@ 12.00 hrs HW=56.36' (Free Discharge)	

1=Exfiltration (Exfiltration Controls 0.13 cfs)

Pond URC3: URC-3 - Chamber Wizard Field A

Chamber Model = ADS_StormTechMC-3500 d +Cap (ADS StormTech®MC-3500 d rev 03/14 with Cap volume)

Effective Size= 70.4"W x 45.0"H => 15.33 sf x 7.17'L = 110.0 cf Overall Size= 77.0"W x 45.0"H x 7.50'L with 0.33' Overlap Cap Storage= 14.9 cf x 2 x 3 rows = 89.4 cf

77.0" Wide + 6.0" Spacing = 83.0" C-C Row Spacing

4 Chambers/Row x 7.17' Long +1.85' Cap Length x 2 = 32.38' Row Length +18.0" End Stone x 2 = 35.38' Base Length 3 Rows x 77.0" Wide + 6.0" Spacing x 2 + 18.0" Side Stone x 2 = 23.25' Base Width 18.0" Stone Base + 45.0" Chamber Height + 12.0" Stone Cover = 6.25' Field Height

12 Chambers x 110.0 cf + 14.9 cf Cap Volume x 2 x 3 Rows = 1,408.8 cf Chamber Storage

5,141.2 cf Field - 1,408.8 cf Chambers = 3,732.3 cf Stone x 33.3% Voids = 1,242.9 cf Stone Storage

Chamber Storage + Stone Storage = 2,651.7 cf = 0.061 af Overall Storage Efficiency = 51.6%Overall System Size = $35.38' \times 23.25' \times 6.25'$

12 Chambers 190.4 cy Field 138.2 cy Stone





Pond URC3: URC-3



Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes
State	Massachusetts
Location	
Longitude	70.538 degrees West
Latitude	41.783 degrees North
Elevation	0 feet
Date/Time	Thu, 21 Jan 2021 13:15:17 -0500

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.29	0.44	0.55	0.72	0.90	1.14	1yr	0.78	1.12	1.33	1.69	2.17	2.79	3.15	1yr	2.47	3.03	3.49	4.03	4.66	1yr
2yr	0.36	0.56	0.69	0.92	1.15	1.45	2yr	0.99	1.37	1.68	2.11	2.65	3.33	3.71	2yr	2.95	3.57	4.07	4.81	5.46	2yr
5yr	0.44	0.68	0.85	1.14	1.46	1.85	5yr	1.26	1.74	2.15	2.68	3.34	4.15	4.67	5yr	3.67	4.49	5.08	5.96	6.68	5yr
10yr	0.50	0.78	0.99	1.34	1.75	2.23	10yr	1.51	2.09	2.59	3.23	3.99	4.90	5.57	10yr	4.34	5.35	6.01	7.00	7.78	10yr
25yr	0.59	0.94	1.20	1.66	2.21	2.85	25yr	1.91	2.66	3.31	4.11	5.04	6.12	7.02	25yr	5.41	6.75	7.52	8.68	9.52	25yr
50yr	0.68	1.10	1.41	1.97	2.65	3.43	50yr	2.29	3.20	3.98	4.93	6.01	7.24	8.37	50yr	6.40	8.05	8.91	10.22	11.10	50yr
100yr	0.79	1.28	1.65	2.33	3.18	4.12	100yr	2.74	3.85	4.79	5.91	7.16	8.56	9.98	100yr	7.58	9.60	10.56	12.03	12.94	100yr
200yr	0.91	1.49	1.92	2.76	3.81	4.96	200yr	3.29	4.64	5.77	7.10	8.54	10.13	11.91	200yr	8.97	11.45	12.52	14.18	15.09	200yr
500yr	1.12	1.84	2.39	3.47	4.85	6.33	500yr	4.18	5.93	7.36	9.01	10.78	12.67	15.05	500yr	11.22	14.47	15.69	17.62	18.51	500yr

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.25	0.38	0.47	0.63	0.77	0.92	1yr	0.67	0.90	1.09	1.52	1.97	2.31	2.89	1yr	2.04	2.78	3.07	3.60	4.39	1yr
2yr	0.35	0.54	0.66	0.90	1.11	1.35	2yr	0.96	1.32	1.57	2.09	2.62	3.20	3.62	2yr	2.84	3.48	3.92	4.64	5.37	2yr
5yr	0.40	0.61	0.76	1.04	1.33	1.62	5yr	1.15	1.58	1.88	2.48	3.09	3.77	4.20	5yr	3.34	4.04	4.67	5.69	6.14	5yr
10yr	0.44	0.68	0.84	1.17	1.51	1.84	10yr	1.30	1.80	2.13	2.81	3.48	4.28	5.04	10yr	3.78	4.84	5.26	5.94	6.86	10yr
25yr	0.50	0.76	0.95	1.36	1.78	2.19	25yr	1.54	2.14	2.45	3.28	4.04	5.06	6.08	25yr	4.48	5.85	6.15	6.82	7.92	25yr
50yr	0.55	0.84	1.05	1.51	2.03	2.48	50yr	1.75	2.42	2.68	3.68	4.49	5.77	7.01	50yr	5.11	6.74	6.95	7.54	8.83	50yr
100yr	0.62	0.93	1.17	1.68	2.31	2.80	100yr	1.99	2.74	2.94	4.14	4.99	6.58	8.09	100yr	5.82	7.78	7.82	8.35	9.82	100yr
200yr	0.68	1.02	1.29	1.87	2.61	3.18	200yr	2.25	3.11	3.22	4.62	5.55	7.51	9.33	200yr	6.65	8.97	8.88	9.25	10.99	200yr
500yr	0.78	1.15	1.48	2.16	3.07	3.73	500yr	2.65	3.65	3.58	5.36	6.38	8.99	11.31	500yr	7.96	10.87	10.53	10.65	12.72	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.32	0.50	0.61	0.82	1.00	1.26	1yr	0.87	1.23	1.50	2.00	2.51	3.13	3.35	1yr	2.77	3.22	3.74	4.30	4.91	1yr
2yr	0.38	0.59	0.72	0.98	1.20	1.47	2yr	1.04	1.44	1.71	2.27	2.84	3.49	3.82	2yr	3.09	3.68	4.35	5.11	5.59	2yr
5yr	0.48	0.74	0.92	1.26	1.61	1.94	5yr	1.39	1.90	2.26	2.90	3.57	4.60	5.09	5yr	4.07	4.89	5.45	6.22	7.29	5yr
10yr	0.58	0.90	1.11	1.55	2.01	2.42	10yr	1.73	2.37	2.80	3.54	4.30	5.65	6.12	10yr	5.00	5.88	6.64	8.11	8.86	10yr
25yr	0.76	1.16	1.45	2.06	2.71	3.25	25yr	2.34	3.18	3.79	4.63	5.53	7.43	8.00	25yr	6.57	7.70	8.58	10.58	11.49	25yr
50yr	0.93	1.42	1.77	2.54	3.42	4.08	50yr	2.95	3.99	4.77	5.67	6.71	9.11	9.82	50yr	8.06	9.44	10.42	12.95	13.99	50yr
100yr	1.15	1.74	2.17	3.14	4.31	5.11	100yr	3.72	5.00	6.01	6.94	8.16	11.20	12.05	100yr	9.91	11.59	12.73	15.91	17.06	100yr
200yr	1.41	2.12	2.69	3.89	5.43	6.41	200yr	4.68	6.27	7.59	8.53	9.92	13.75	14.80	200yr	12.16	14.23	15.50	19.49	20.82	200yr
500yr	1.87	2.78	3.57	5.19	7.38	8.65	500yr	6.37	8.46	10.36	11.22	12.88	17.97	19.42	500yr	15.91	18.67	20.06	25.47	27.11	500yr



APPENDIX E

TSS and Recharge Calculations



Project Name: Cape View Way

Project No:	19038
Calculated by:	GK
Checked:	BRK

Silty Clay

Clay

STANDARD 3-RECHARGE REQUIREMENTS

TOTAL DRAINAGE AREA	346,654	sf
	7.96	acres
TOTAL IMPERVIOUS AREA	81,004	sf
	1.86	acres
TOTAL IMPERVIOUS DIRECTED TO RECHARGE	81,004	sf
	1.86	acres
% IMPERVIOUS TO BE RECHARGED	100	%

SOIL TYPE	A	
RECHARGE VOLUME REQUIRED (Rv)	4,050	cft
AVE. INFILTRATION RATE	2.41	in/hr

RECHARGE VOLUMES		
RAINFALL	5.88	in
TREE TRENCH 1	627	cf
TREE TRENCH 2	184	cf
BIO-1	908	cf
BIO-2	288	cf
URC-1	7,623	cf
URC-2	5,303	cf
URC-3	2,652	cf
TOTAL RECHARGE VOLUME PROVIDED	17,585	cf
TOTAL RECHARGE VOLUME REQUIRED	4,050	cf

Soil Type	Target Depth (in)	Target Depth (ft)
A	0.6	0.05
В	0.35	0.029
С	0.25	0.021
D	0.1	0.008
Rawls Table		
Texture Class	NRCS Hydrologic	Infiltration Rate

Date:

Date:

12/10/2021

Soil Group (HSG) Inches/Hour Sand А 8.27 Loamy Sand А 2.41 В 1.02 Sandy Loam В 0.52 Loam С Silt Loam 0.27 С Sandy Clay 0.17 Clay Loam D 0.09 Silty Clay Loam D 0.06 Sandy Clay D 0.05

D

D

0.04

0.02

Calculate *Required Recharge Volume*.⁷ The *Required Recharge Volume* equals a depth of runoff corresponding to the soil type times the impervious areas covering that soil type at the post-development site.

Rv = F x impervious area

Equation (1)

Rv= Required Recharge Volume, expressed in Ft³, cubic yards, or acre-feetF= Target Depth Factor associated with each Hydrologic Soil GroupImpervious Area= pavement and rooftop area on site

To determine whether an infiltration BMP will drain within 72 hours, the following formula must be used²¹:

$$Time_{drawdown} = \frac{Rv}{(K)(Bottom Area)}$$

Where:

Rv = Storage Volume

K = Saturated Hydraulic Conductivity For "Static" and "Simple Dynamic" Methods, use Rawls Rate (see Table 2.3.3). For "Dynamic Field" Method, use 50% of the in-situ saturatec hydraulic conductivity.

Bottom Area = Bottom Area of Recharge Structure²²

²¹ In some cases, the infiltration structure may be designed to treat the Required Water Quality Volume and/or to attenuate peak discharges in addition to infiltrating the Required Recharge Volume. In that event, the storage volume of the structure must be used in the formula for determining drawdown time in place of the Required Recharge Volume.

Practice	(cf))	(sf)	K	Time to drawdown (hrs)*
TREE TRENCH 1 TREE TRENCH 2	627 184	639 150	2.41 2.41	5 6
BIO-1	908	820	2.41	6
BIO-2	288	408	2.41	4
URC-1	7,623	2,090	2.41	18
URC-2	5,303	1,499	2.41	18
URC-3	2,652	823	2.41	16

*MUST BE LESS THAN 72 HOURS

 $^{^{20}\,}$ The drawdown analysis also assumes that the water table does not fluctuate during the draw down period.







INSTRUCTIONS:

1. In BMP Column, click on Blue Cell to Activate Drop Down Menu

2. Select BMP from Drop Down Menu

3. After BMP is selected, TSS Removal and other Columns are automatically completed.

Locati	on:	Subcatchment DA3c to RB1			
	В		D Starting TSS	E	F
	BMP1	Rate1	Load*	Amount Removed (C*D)	Load (D-E)
	Deep Sump Catch Basin with Inlet Filter	0.99	1.00	0.99	0.01
loval	Dry Well	0.80	0.01	0.01	0.00
Remculat		0.00	0.00	0.00	0.00
TSS Cal		0.00	0.00	0.00	0.00
		0.00	0.00	0.00	0.00
		Total T	SS Removal =	100%	Separate Form Needs to be Completed for Each Outlet or BMP Train
	Project: Prepared By: Date:	¹⁹⁰³⁸ GK 9/3/2021		*Equals remaining load from which enters the BMP	n previous BMP (E)



APPENDIX F

Stormwater Operation and Maintenance Plan

Stormwater Management Operation and Maintenance Plan

> Cape View Way Bourne, Massachusetts

> > Prepared for:

PRESERVATION OF AFFORDABLE HOUSING 2 Oliver Street, Suite 500 Boston, MA 02109

Prepared by: Horsley Witten Group, Inc

December 2021



TABLE OF CONTENTS

Page

1.0	OWNER AND RESPONSIBILITY FOR MAINTENANCE	1
2.0	INTRODUCTION	2
3.0	FUNCTION & MAINTENANCE	3
3.2	Dry Well (Recharge Basin)	5
3.3	Tree Trench	6
3.4	Underground Chambers	7
3.5	Landscape Maintenance	8
WE	ED GUIDE	10
4.0	ROUTINE MAINTENANCE	14
5.0	SNOW REMOVAL	14
6.0	LONG-TERM POLLUTION PREVENTION PLAN	14
6.1	Lawn/Landscaping Maintenance	14
6.2	Pet Waste Management	14
6.3	Solid Waste Management	14
6.4	Pavement Sweeping Schedules	14
6.5	Illicit Discharges	14
6.6	Personnel Training	14
7.0	ESTIMATED OPERATION AND MAINTENANCE BUDGET	15

APPENDICES

- Appendix A: Maintenance Checklists
- Appendix B: Maintenance Plans
- Appendix C: Overall Stormwater BMP Locations
- Appendix D: Underground Chambers and FlexStorm Manufacturer's Requirements

1.0 OWNER AND RESPONSIBILITY FOR MAINTENANCE

POAH, LLC is responsible for the financing and continuous operation, maintenance and required emergency repair for the stormwater management system and associated drainage network.

Owner: POAH, LLC 2 Oliver Street Suite 500 Boston, MA 02109

Contact:	TBD	
Name:		
Email:		
Ph:		

Signed:_____

Date:_____

1

2.0 INTRODUCTION

This Guide provides a general description of the function and maintenance requirements for the Stormwater Management System for the Cape View Way Housing Development. Proper maintenance is vital to their long-term success.

The proposed stormwater management includes a green stormwater infrastructure (GSI) approach to filter, infiltrate and store stormwater runoff prior to discharge. Therefore, the maintenance provider is required to familiarize themselves with this Guide and inspect and maintain the following GSI practices, as indicated on the construction drawings, and as outlined in this maintenance guide throughout the year.

PRETREATS AND FILTERS

- Deep Sump Catch Basins
 - An underground retention system designed to remove trash, debris, and coarse sediment from runoff. Typically, deep sump catch basins have a minimum fourfoot sump. The catch basins will have FLEXSTORM® PURE inlet filter installed to provide further pretreatment before discharging to another stormwater management control.
- Bioretention Area
 - A shallow depression in the landscape designed to collect, move, hold, and treat stormwater as it infiltrates through a soil matrix to remove phosphorus and reduce stormwater runoff prior to discharge to the storm drain system.
- <u>Tree Trench</u>
 - A tree pit with underground infiltration trenches. It is a type of bioretention facility.

STORES AND INFILTRATES

- Dry Well (Recharge Basin)
 - An excavated pit used to infiltrate runoff. Pretreatment must occur prior to the runoff discharging to a dry well for runoff sourced from parking lots and other impervious cover with higher potential pollutant loads.
- Underground Recharge Chambers
 - The underground recharge chambers are designed to store and infiltrate runoff. The underground chambers include an overflow structure to slowly release runoff from larger storm events to the drainage system.

3.0 FUNCTION & MAINTENANCE

How Does Green Infrastructure Work?

GSI is nature-based approach to stormwater treatment and management. These stormwater practices or "treatment areas" are designed to mimic nature and use the natural filtration properties of soil and plants to remove pollutants from stormwater runoff prior to discharging to the drainage system. GSI relies on the following five basic design elements, or steps, to function properly.

- 1. Collect (Inlets)
- 2. Capture Sediment (Pretreatment)
- 3. Move Water (Pipes)
- 4. Treat and Manage (Stores, Filters, and Infiltrates)
- 5. Overflows (Structure)

These five steps will be referenced throughout this Guide. If one of these steps does not function properly, the entire system can be compromised and the GSI practice itself could be contributing to maintenance problems. This can lead to a landscape nuisances, more frequent maintenance, and costly repairs/improvement.

What is required for Maintenance?

As these are nature-based systems that often rely on plant care, the maintenance for GSI typically falls under landscape and general site maintenance services. The regularly scheduled maintenance as outlined in this Guide is critical to ensure proper function, maintain infiltration rates and storage capacity and preserve the pollutant removal capabilities as well as the visual appearance. Regularly scheduled maintenance can prevent deficiencies in the effectiveness of the systems, due to sediment build-up, damage, or deterioration.

General maintenance includes the following:

- 1. Removing sediment from the pretreatment practices used to capture sediment.
- 2. Maintaining the proper drainage function and pollutant removal capacity of the systems.
- 3. Maintaining healthy native, tress, plants, and vegetative cover as well as the removal of unwanted weeds.

It is recommended that all practices be maintained regularly as part of the routine landscape maintenance or at a minimum four times per year and after major rain events.

- Early Spring: during spring cleanup
- Summer: during lawn mowing and other routine park maintenance
- Early Fall: when leaves begin to fall
- Late Fall/Early Winter: after all the leaves have fallen during leaf removal
- After major storm events: 2" of rain or greater.

The following sections describes the general function and landscape maintenance of each practice. Included in the appendices is a specific maintenance checklist for each practice type along with a plan showing the location of the items to be inspected and maintained.



FUNCTION:

- 1. <u>COLLECT</u> Inlet flume Stormwater runoff is collected at the inlet flume and diverted to the sediment forebay.
- <u>CAPTURE</u> Sediment Forebay Sediment, trash, and debris is captured and accumulates overtime in the sediment forebay.
- 3. <u>MOVE -</u> The stormwater discharges directly to the bioretention area via a granite check dam weir.
- <u>TREAT AND MANAGE</u> Bioretention Soil (filters) Stormwater overtops the forebay granite check dam and flows through the planted bioretention area, which infiltrates or filters stormwater through the planted sandy soil matric and subsoils.
- <u>OVERFLOW</u> Perforated Underdrain and Structure During larger rain events, soils become saturated and the underdrain drains water to the overflow structure. For extreme events, the water level will rise and overflow into the outlet structure.

<u>SURROUNDING AREA</u> - Parking lots, roadways, sidewalks, and open lawns Problems such as unstabilized soils, erosion, invasive plants and over sanding of the parking lot can contribute to long-term maintenance problems (See Section 6.0).

See Appendix A for Maintenance Checklist

3.2 Dry Well (Recharge Basin)



FUNCTION:

- <u>COLLECT</u> Catch Basin Grate and Inlet Pipes Stormwater runoff is collected from roof drains by pipe and overland flow through the catch basin grate.
- <u>CAPTURE</u>–Dry Well Stormwater runoff is captured in the dry wells and stored during rain events.
- 3. <u>MOVE</u>– NA
- <u>INFILTRATE</u> Gravel and Subsoil Runoff if is infiltrated into the sub soils through the dry well perforations and surrounding gravel.
- 5. OVERFLOW -

When the capacity of the dry well is exceeded an overflow pipe is provided, which is assumed to discharge to the City drainage network in Thatcher Street.

SURROUNDING AREA – Landscape Area

Problems such as unstabilized soils, erosion, and leaf litter can contribute to long-term maintenance problems (See Section 6.0).

See Appendix A for Maintenance Checklist



FUNCTION:

- <u>COLLECT</u> Catch Basin Grate and Sidewalk Area Drain Stormwater runoff is collected along the road gutter and sidewalk surface via overland flow through the catch basin and area drain.
- <u>CAPTURE</u> Catch Basin and Area Drain Sump The catch basin and area capture sediment and debris in the 4' sump prior to discharge.
- <u>MOVE</u> Pipe and Laterals Stormwater is diverted to an 8" pipe from the catch basin to an 8" perforated pipe or lateral which discharges to the gravel storage bed.
- <u>TREAT AND MANAGE</u> Tree, Gravel Bed and Subsoil Runoff off is stored in the gravel bed providing water for the trees and infiltrated into the subsoils.
- <u>OVERFLOW</u> Pipe and chimney connection to the city storm drain When the capacity of the gravel bed is exceeded an overflow pipe in the catch basin diverts overflow runoff to the existing drainage system.

SURROUNDING AREA - Roadway and Sidewalks

Problems such as unstabilized soils, erosion, and over sanding during the winter can contribute to long-term maintenance problems (See Section 6.0).

See Appendix A for Maintenance Checklist



- <u>COLLECT</u> Catch basin or Bioretention/Sand Filter (See Section 3.1 and 3.2) Stormwater runoff is directed overland to the inlet(s) where stormwater enters the system.
- <u>CAPTURE</u> Catch basin/Isolator Row/ Bioretention/Sand Filter) Sediment, trash, and debris is captured and accumulates overtime in the deep sumps, the bioretention area or sand filter forebays, or chamber isolator row (see Appendix C)
- <u>MOVE</u> Pipes and Manifold Runoff is directed to the isolator row and underground chambers via a closed pipe/manifold system.
- 4. <u>STORE AND INFILTRATE</u>- Underground Chambers (infiltrates and/or stores) For recharge chambers, runoff from small rain events infiltrate into the subsurface soils beneath the chambers. Larger storm events are stored and slowly released. For underground storage pipes, stormwater is stored and slowly released.
- <u>OVERFLOW</u> Manhole with Weir During larger rain events and underdrain will supplement the infiltration to release stored water from the chambers and discharge via an overflow structure.

SURROUNDING AREA – Parking Lot/Driveway

Problems such as unstabilized soils, erosion, invasive plants and over sanding can contribute to long-term maintenance problems (See Section 6.0).

See Appendix A for Maintenance Checklist

See Appendix D for additional manufacturer's requirements.

3.5 Landscape Maintenance

By design, plants in the GIS practices are meant to help filter the stormwater and flourish throughout the growing season. The plants do not require fertilizers, watering and/or mowing. Remove and replace vegetation as necessary, using the appropriate species as shown on the recommended plant list below.

Plants				
Task	Frequency	Requirement	Time of Year	
Watering	First three months after planting or drought	• During establishment or drought conditions, plants should be watered a minimum of once every seven to ten days.	• June-Sept.	
Plant Cutting & Pruning	Annually	 Leave dry standing stalks during the dormant months and remove in the spring. Cut back grasses, sedges, and rushes in the spring. Prune trees to remove deadwood and low hanging branches. 	Early Spring	
Plant Thinning	Once every 3 years	 Separation of herbaceous vegetation rootstock should occur when over-crowding is observed 	 Early Spring or Late Fall 	
Plant Replacement	As required	 Replace/replant diseases, unhealthy or dead plans to maintain a healthy plant community 	• Early Spring or Fall	
Mowing, Bioretention	NOT REQUIRED	• NONE, DO NOT MOW	• NA	
Fertilizing	NOT REQUIRED	NONE	• NA	
Mulch	NOT REQUIRED	• NONE	• NA	

To reduce the level of effort, regular weeding should occur quarterly from April thru October.

Weeds					
Task	Frequency	Requirement	Time of Year		
Weeding	Quarterly	 Weeding should be limited to invasive and exotic species, which can overwhelm the desired plant community. Non-chemical methods including hand pulling and hoeing are recommended Chemical herbicides should be avoided. 	 Early Spring Late Spring Late Summer Late Fall 		

REPLACEMENT PLANTS

The plants that thrive in bioretention areas and tree trenches are typically quite drought tolerant due to the filter profile having a top layer of planting soil and sandy soil media or aggregate below. In bioretention areas, they need to be able to withstand periods of inundation after storm events, however; when it doesn't rain, there will be less water held naturally in the sand than in other soil types for the plants to use.

If replacements are needed, use the planting plan as a guide (see proposed species list below). However, if all the plants of a certain species have not done well in the bioretention area, do not replace with that same species. Rather, replant with one or more of the other species that has thrived under the bioretention area conditions or have a plant professional choose a different species based on current photos of the site and the following site-specific considerations.

Plants for bioretention areas should be:

- Preferably native species to help support native wildlife like pollinators
- Drought tolerant
- Tolerant of inundation for 24 hours
- Size constraints: Not so tall that they impact sightlines, as applicable
- Sun and salt tolerant
- A mix of different types of plants that will create a resilient plant community: cold & warm season grasses, perennials, groundcovers in all areas.

Trees

Amelanchier x grandiflora	Serviceberry
Betula nigra	River Birch
Acer Rubrum	Red Maple

Ornamental Grasses / Perennials

Asclepia tuberosa	Butterflyweed	
Carex pensylvanica	Pennsylvania Sedge	
Deschampsia cespitosa	Tufted Hair Grass	
Elymus virginicus	Virginia Wild Rye	
Eupatorium maculatum	Joe-Pye Weed	
Geranium maculatum	Cranesbill	
Iris versicolor	Blue Flag	
Juncus effusus	Soft Rush	
Liatris spicata	Blazing Star	
Monarda fistulosa	Wild bergamot	
Muhlenbergia capillaris	Pink Muhly Grass	
Pycnanthemum muticum	Big Leaf Mountain Mint	
Panicum virgatum	Switch grass	
Schizachyrium scoparium	Little Bluestem	
Sporobolus heterolepis	Prairie Dropseed	

WEED GUIDE



Yellow Toadflax (Linaris vulgaris)



Redroot Pigweed- (Amaranthus retroflexus)









Fireweed (Erechtites hieracifolia)

Spotted Spurge (Euphorbia maculata)



Crabgrass (Digitaria ischaemum)





Japanese Knotweed (Polygonum cuspidatum)






Catalpa Tree Seedling (Catalpa speciosa)



Purple Loosetrife (Lythrum salicaria)



Field Bindweed (Convolvulus arvensis)



Black Swallow-wort (Cynanchum Iouisea)

4.0 ROUTINE MAINTENANCE

Other routine maintenance should include the following:

- Remove of trash and litter from paved and perimeter areas.
- Pavement Sweeping:
 - Minimum of once per year after the spring thaw.
- Check for erosions problems and sediment source(s) along the GSI practice sidewalls if excessive, frequent sediment accumulation occurs in practice area.
- Check for erosions problems and sediment source(s) in the contributing drainage area if excessive, frequent sediment accumulation occurs at inlet flume of sediment forebay.
- Contributing drainage pipes:
 - Inspect annually for proper operation.

5.0 SNOW REMOVAL

Snow removal from the practice is not necessary. Plowed or shoveled snow piles should not block the catch basin grates or inlet flumes.

Excessive salting, sanding or other de-icing practices should be avoided. Use of large amounts of sand should also be avoided to avoid obstructing/clogging the conveyance system.

6.0 LONG-TERM POLLUTION PREVENTION PLAN

Long-term pollution prevention measures implemented throughout the development site will further reduce pollutants in stormwater discharges after construction.

6.1 Lawn/Landscaping Maintenance

Lawn and landscaping maintenance should be conducted with minimal use of fertilizers and pesticides to protect the nearby wetland and water resources. In particular, phosphate-based fertilizers are not to be used. Prior to applying fertilizers to the lawn and landscape, a soil analysis should be completed,

6.2 Pet Waste Management

Residents and visitors will be encouraged to pick up after their pets with signage along lawn areas.

6.3 Solid Waste Management

Enclosed dumpsters with lids will be provided on-site for solid waste management at the site.

6.4 Pavement Sweeping Schedules

The road and parking area will be, at a minimum, swept annually after spring snowmelt.

6.5 Illicit Discharges

No sewer pipes, floor drains or other new pipe connections will be connected to the drainage system. All wastewater will be connected to the municipal sewer.

6.6 **Personnel Training**

All staff/ personnel responsible for maintaining the practices will be given a copy of this Guide and will receive training in the applicable practices and implementation described in herein.

7.0 ESTIMATED OPERATION AND MAINTENANCE BUDGET

The estimated average annual operating and maintenance budget for the proposed system is shown below:

Bioretention (2): \$1,000 per practice Source: Center for Watershed Protection (CWP)	\$ 2,000
Tree Trench (2) \$1,000 per practice Source: Estimate	\$ 2,000
Catch Basin and Recharge Basins (6): \$200/structure Source: Massachusetts Highway Department	\$ 1,200
Underground Chamber (3): \$2,000/field Source: Manufacturer	\$ 6,000
Other Routine Maintenance: Removal of trash and litter Annual parking lot cleaning Drainage network inspections Source: Estimate	\$ 2,000
Total:	\$ 13,200

It should be noted that the maintenance costs provided are estimates only.



Bioretention/Bioswale Maintenance Checklist Cape View Way Development

Date:

Time:

Inspector:

Maintenance Item	Description	Maintenance (Y/N)			
 COLLECT Includes: Catch basin/Inlet Frequency: Inspect four tim rain or greater) 	Structure nes per years during regular park maintenance and after major storm e	events (2" of			
When: March, June, Septer	nber, November				
Surface Debris Cleaning	Remove all trash, leaf litter and inlet clogging.				
Inlets	Check for clogging and sediment accumulation that impacts inflow. If sediment/debris accumulation				
Actions to be taken:					
2. CAPTURE Includes: Sediment Foreba Frequency: Inspect four tim storm events (2" of rain or g When: March, June, Septer	ay nes per year and after major storm events the first year; then annually reater) nber, November	and after major			
Debris Cleanout	Remove all trash and debris.				
Side Slopes Signs of erosion gullies, animal burrowing, overtopping, or slumping are observed. Repair, as necessary.					
Sediment/Organic Debris Removal Removal Remove sediment accumulation and properly dispose when accumulation is greater than or equal to 3 inches or you cannot see stones.*					
Actions to be taken:	•				
3 & 4. MOVES & FILTERS Includes: Planting bed Frequency: Inspect four times per years during regular park maintenance and after major storm events (2" of rain or greater)					
When: March, June, September, November					
Debris Cleanout Remove trash and debris from the surface.					
Sediment/Organic Debris Remove and properly disposed of when build-up is greater than or equal to 3 inches.*					

Maintenance Item	ce Item Description N		
Erosion	Check for areas of erosion/ gullies, particularly along the bottom. Repair/reseed as necessary		
Side Slopes	Signs of erosion gullies, animal burrowing, overtopping, or slumping are observed. Repair, as necessary.		
Vegetation Maintenance Replacement	Cut back twice per year minimum (12" grass height). Over seed bare or thin grass growth areas. See also Landscape Maintenance		
Water Draining properly	event, check for standing water in cleanouts. If standing water observed flush underdrains. If still not draining, rototill or aerate the bottom 6 inches to breakup any hard-packed sediment		
Actions to be taken:			
5. OVERFLOW			
Frequency: Inspect bi-annu When: March and Septemb	ally and after major storm events (2" of rain or greater) er		
	Water level should below underdrain and outlet pipe inverts.		
Overflow Structure	Check for sediment accumulation that impacts outflow.		
	If sediment accumulation. Schedule cleaning.		
Actions to be taken:			
Other Routine Grounds Includes: Surrounding land Frequency: Inspect four tim	Maintenance Iscape beyond the practice. Thes per year during regular park maintenance and after major storm even	/ents	
When: March, June, Septer	nber, November		
	Remove trash from perimeter areas.		
Contributing drainage area	area Look for sediment sources from erosion in the surrounding area.		
Drainage Network	Ensure proper operation.		
Pavement Sweeping	Sweep parking lot minimum once a year after spring thaw.		
Actions to be taken:			

*Sediment shall be disposed of offsite in a pre-approved location.

Underground Chambers - Maintenance Checklist Cape View Way Development

Date:

Time:

Inspector:

Maintenance Item	Description	Maintenance (Y/N)			
 COLLECT Includes: Catch basin/Inlet Frequency: Inspect four tim rain or greater) 	Structure - see also bioretention hes per years during regular park maintenance and after major storm e	events (2" of			
When: March, June, Septer	nber and November				
Inlet Grate Remove all trash, leaf litter and inlet clogging. Inlet Grate Remove sediment regularly or when accumulation impedes proper inflow and/or outflow.					
Surface Debris Cleaning	Remove all trash, leaf litter and inlet clogging. Check for clogging and sediment accumulation that impacts inflow.				
Actions to be taken:					
Includes: Deep Sump/Sedi Frequency: Inspect four tim storm events (2" of rain or gi When: Mar March, June, Se	iment Forebay/Isolator Row les per year and after major storm events the first year; then annually reater) eptember and November	and after major			
Debris Cleanout	Remove all trash and debris from the swale.				
Sediment/Organic Debris RemovalRemove sediment accumulation and properly dispose when accumulation is greater than or equal to 3 inches or you cannot see stones.*					
Actions to be taken:					
3. MOVE					
Drain Manhole and manifold Cleanout	Remove trash and debris from the surface.				
	See Also Manufacturer's Requirements				
Actions to be taken:					

Maintenance Item	Maintenance (Y/N)					
4. STORE AND INFILTE	RATE					
Includes: Chambers						
Frequency: Inspect annuall When: Spring	y – see manufacturer's requirements					
Sediment/Organic Debris Removal	Use inspection ports to check chambers for sediment accumulation in isolator row.					
Water Draining properly	If standing water is observed for more than 48 hours after a storm event, jet vac chambers.					
5. OVERFLOW						
Includes: Drain manholes a	and weir walls					
Frequency: Inspect annual	y and after major storm events (2" of rain or greater)					
When: Spring		r				
Overflow Structure	accumulation. Schedule cleaning.					
	Check for leaf litter, debris, and inlet clogging.					
Actions to be taken:						
Other Routine Grounds	Maintenance					
Includes: Surrounding land Frequency: Inspect four time	lscape beyond the practice. les per year during regular park maintenance and after major storm ev	vents				
When: March, June, Septer	nber and November					
Debris Removal	Remove trash from perimeter areas.					
Contributing drainage area	Look for sediment sources from erosion in the surrounding area.					
Drainage Network	Ensure proper operation.					
Pavement Sweeping Sweep parking lot minimum once a year after spring thaw.						
Actions to be taken:						

*Sediment shall be disposed of offsite in a pre-approved location.

Dry Well - Maintenance Checklist Cape View Way Development

Date:

Time:

Inspector:

	Req'd (Y/N)							
1. COLLECT								
Includes: Catch basin grate	e/Inlet pipes							
Frequency: Inspect four tim rain or greater)	nes per years during regular park maintenance and after major storm e	events (2" of						
When: March, June, Septer	nber, November							
Surface Debris Cleaning	Surface Debris Cleaning Remove all trash, leaf litter and inlet clogging							
Inlet Pipes	Check for clogging and sediment accumulation that impacts inflow. Inlet Pipes If sediment/debris accumulation. Check roof downspouts for clogging.							
Actions to be taken:								
2. CAPTURE								
Includes: Dry Well								
Frequency: Inspect four tim storm events (2" of rain or g	nes per year and after major storm events the first year; then annually reater)	and after major						
When: March, June, Septer	nber, November							
Debris Cleanout	Remove trash and debris							
Sediment/Organic Debris RemovalRemove sediment accumulation and properly dispose when accumulation is greater than or equal to 6 inches or you cannot see stones along the bottom.								
Actions to be taken:	•							
3. MOVE								
Includes: NA								
Frequency: NA								
When: NA								
4. INFILTRATE								
Includes: See # 2 above								
Frequency: See # 2 above								
When: See # 2 above								
Water Dreining property	If standing water is observed for more than 48 hours after a storm event, check for clogging.							
vvater Draining property	If necessary, vactor basin and use a hose to breakup any hard- packed sediment along the bottom.							
Actions to be taken:								

Maintenance Item	Maintenance Req'd (Y/N)	
5. OVERFLOW		
Includes: Outlet pipe		
Frequency: Inspect bi-annu When: March and Novembe	ally and after major storm events (2" of rain or greater) er	
Overflow Pipe	Check for sediment accumulation that impacts inflow. If sediment accumulation. Schedule cleaning. See # 4 above	
Actions to be taken:		
Other Routine Grounds Includes: Surrounding land Frequency: Inspect four time	Maintenance Iscape beyond the practice. les per year during regular park maintenance and after major storm ev	vents
When: March, June, Septer	nber, November	
Debris Removal	Remove trash from perimeter areas.	
Leaf and landscape debris removal	Clean grate regularly during landscape maintenance.	
Surrounding Drainage Network	Ensure proper operation.	
Contributing drainage area	Check to ensure the surrounding area is stabilized. Look for erosion and other sediment sources	
Actions to be taken:		

*Sediment shall be disposed of offsite in a pre-approved location.

<u>Tree Trench - Maintenance Checklist</u> Cape View Way Development

Date:

Time:

Inspector:

Maintenance Item	Maintenance Req'd (Y/N)				
1. Collect					
Includes: Catch basin grate	e/sidewalk area drain				
Frequency: Inspect three till (2" of rain or greater)	mes per years during regular landscape maintenance and after major	storm events			
When: Spring, Summer, Lat	te Fall				
	Remove all trash, leaf litter and inlet clogging.				
Inlet Grate	Remove sediment regularly or when accumulation impedes proper inflow and/or outflow.				
Actions to be taken:					
2. Capture					
Includes: Deep sump					
Frequency: Inspect bi-annu events (2" of rain or greater)	ally and after major storm events the first year; then annually and afte	r major storm			
When: Spring and Fall					
Deep Sump	Remove trash, sediment and debris from the structures and debris from the surface.				
Actions to be taken:					
3. Move					
Includes: Inlet pipe					
Frequency: Inspect bi-annu	ally for sediment, sand, debris accumulation and clogging.				
When: Early Spring, Late Fa	all				
	Check catch basin for standing water above inlet invert (lowest				
Clogging	pipe). Water level should be below pipe invert.				
	If standing water is observed clean pipe.				
Actions to be taken:					
4. Treat and Manag	e (stores and infiltrates)				
Includes: Laterals, subsurfa	ace gravel storage bed and trees				
Frequency: Inspect three til (2" of rain or greater)	mes per years during regular landscape maintenance and after major	storm events			

When: Spring, Summer, Late Fall

Maintenance Item	Maintenance Item Description						
Laterals	If standing water is observed in the catch basin, open cleanouts and check for standing water.						
	If standing water is observed flush laterals.						
Tree	Tree Check tree health and look for evidence of stress, insects, or disease						
Action to be Taken:							
5. Overflow							
Includes: Pipe and chimne	y connection to city storm drain						
Frequency: Inspect annuall When: Spring	ly and after major storm events (2" of rain or greater)						
Water Draining properly If water is observed in the catch basin above the overflow invert (highest pipe), check pipe for clogging and flush, as necessary.							
Actions to be taken:							
Other Poutine Maintona	200						
Includes: Surrounding land	iscape beyond the practice						
Frequency: Inspect three til	mes per year during regular park maintenance and after major storm e	events					
When: Spring, Summer, Lat	te Fall						
Debris Removal	Remove trash from perimeter areas.						
Pavement Sweeping	Sweep contributing paved surfaces minimum once a year after spring thaw.						
Surrounding Drainage Network Ensure proper operation of surrounding catch basins							
Contributing drainage area	Check to ensure the surrounding area is stabilized. Look for erosion and other sediment sources						
Actions to be taken:							

*Sediment shall be disposed of offsite in a pre-approved location.





BIORETENTION AREA 1

____ DMH 400 **BIORETENTION AREA 2** RIM:64.71' BOTTOM EL. = 62.50' FFE: 67.25 12" INV IN:59.15' 408 SF 12" INV OUT:58.00' 12" INV OUT:58.00' 69-TR/ CR 67.08 ST/ RD # 66.92 **#66.71** VD RD 04.78 04.78 65 64 65.02 60 G 66.97 1 TO I 64.75 63 94 5.78 12" HDPE 4 65 €65.62 65. +LP:63.99 5:1 64-# 65.46 64 -631 62-1 -61 85.30 65.14 CHAIN LINK FENCE -PR ∇ -<mark>DI</mark> RI ×EL:57.26 SEDIMENT FOREBAY URC-2 BOTTOM EL. = 62.50' STORMTECH 12 MC-3500 105 SF 12 DB 410 25 CHAMBERS GE OF PAVEMENT 12 RIM:63.00' 4" INV IN:60.00' 12" INV OUT:59.50'

BIORETENTION AREA 2

TREE TRENCH 1



TREE TRENCH 2





UNDERGROUND RECHARGE CHAMBERS 1

UNDERGROUND RECHARGE CHAMBERS 2



UNDERGROUND RECHARGE CHAMBERS 3





Overall Stormwater BMP Locations







Underground Chambers and FlexStorm Manufacturer's Requirements





FLEXSTORM[®] PURE PERMANENT INLET PROTECTION

SPECIFY WITH CONFIDENCE

State DOTs and municipalities across the country now have a universal structural BMP to address the issue of storm sewer inlet protection: FLEXSTORM PURE Inlet Filters.

The FLEXSTORM PURE system is the preferred choice for permanent inlet protection and storm water runoff control. Constructed of versatile stainless steel, FLEXSTORM PURE Inlet Filters will fit any drainage structure and are available with site-specific filter bags providing various levels of filtration. Whether you're the specifier or the user, it's clear to see how FLEXSTORM PURE Inlet Filters outperform the competition.

APPLICATIONS:

Car Washes	Gas Stations
Commercial	Parking Lots
Loading Ramps	Dock Drains
Industrial	Maintenance

FEATURES:

- Stainless Steel filter framing is custom configured to fit perfectly into any drainage structure, whether a standard design or obstructed inlet opening
- Filtered Flow Rates and Ultimate Bypass Rates are designed to meet your specific inlet requirements
- Multiple Filter Bags are available targeting site specific removal of trash, litter, leaves, or small particles, oil and grease
- Filters work below grade with an ultimate bypass allowing inlet area to drain with a full bag
- Units install in seconds and are easily maintained with the FLEXSTORM Universal Removal Tool (no heavy machinery required)

ADS Service: ADS representatives are committed to providing you with the answers to all your questions, including selecting the proper filter, specifications, installation and more. Also try the ADS FLEXSTORM Online Product Configurator at www.inletfilters.com





FEATURES:

- Receive payback on your investment: durable stainless steel framing provides extended service life while replaceable filter bags handle loads with a safety factor of 5
- Meets stringent removal requirements:
 FX filter bags are rated for >80% removal efficiency of street sweep-size particles
 - PC/PC+ filter bags have been tested to 99% TSS removal of OK-110 US Silica Sand and 97% TPH (total petroleum hydrocarbon) removal
- Help prevent fines: FLEXSTORM Inlet Filters comply with EPA NPDES initiatives as a temporary or permanent BMP
- If not in stock, orders up to 100 pieces can ship within 48 hours





FLEXSTORM PURE INLET FILTERS SPECIFICATION

IDENTIFICATION

The installer shall inspect the plans and/or worksite to determine the quantity of each drainage structure casting type. The foundry casting number, exact grate size and clear opening size, or other information will be necessary to finalize the FLEXSTORM part number and dimensions. The units are shipped to the field configured precisely to fit the identified drainage structure.

MATERIAL AND PERFORMANCE

The FLEXSTORM Inlet Filter system is comprised of a corrosion resistant steel frame and a replaceable geotextile filter bag attached to the frame with a stainless steel locking band. The filter bag hangs suspended at a distance below the grate that shall allow full water flow into the drainage structure if the bag is completely filled with sediment. The standard Woven Polypropylene FX filter bags are rated for 200 gpm/sqft with a removal efficiency of 82% when filtering a USDA Sandy Loam sediment load. The Post Construction PC filter bags are rated for 137 gpm/sqft and have been 3rd party tested at 99% TSS removal to 110 micron and 97% TPH removal of used motor oil hydrocarbon mix.

INSTALLATION

Remove the grate from the casting or concrete drainage structure. Clean the ledge (lip) of the casting frame or drainage structure to ensure it is free of stone and dirt. Drop in the FLEXSTORM Inlet Filter through the clear opening and be sure the suspension hangers rest firmly on the inside ledge (lip) of the casting. Replace the grate and confirm it is elevated no more than 1/8", which is the thickness of the steel hangers. For wall mount units, follow instructions for attaching the stainless steel mounting brackets using the provided concrete fasteners.

INSPECTION FREQUENCY

Construction site inspection should occur following each ½" or more rain event. Post Construction inspections should occur three times per year (every four months) in areas with mild year round rainfall and four times per year (every three months Feb-Nov) in areas with summer rains and before and after the winter snowfall season. Industrial application site inspections (loading ramps, wash racks, maintenance facilities) should occur on a regularly scheduled basis no less than three times per year.

MAINTENANCE GUIDELINES

Empty the filter bag if more than half filled with sediment and debris, or as directed by the engineer. Remove the grate, engage the lifting bars or handles with the FLEXSTORM Removal Tool, and lift from the drainage structure. Dispose of the sediment or debris as directed by the engineer or maintenance contract in accordance with EPA guidelines.

As an alternative, an industrial vacuum may be used to collect the accumulated sediment. Remove any caked-on silt from the sediment bag and reverse flush the bag with medium spray for optimal filtration. Replace the bag if torn or punctured to ½" diameter or greater on the lower half of the bag. Post Construction PC/PC+ Bags should be maintained prior to 50% oil saturation. The average 2' x 2' PC filter bag will retain approx. 96 oz (5.4 lbs) of oil at which time it should be serviced or replaced. It can be centrifuged or passed through a wringer to recover the oils, and the fabric reused with 85% to 90% efficacy. It may also be recycled for its fuel value through waste to energy incineration. When utilizing the Cleartec Rubberizer Pouches in the + bags, note that these oil skimmers will gradually turn brown and solidify as they become saturated, indicating time for replacement. Each pouch will absorb approximately 62 oz (4 lbs) of oil before requiring replacement. The spent media may also be recycled for its fuel value through waste to energy incineration. Dispose of all oil contaminated products in accordance with EPA guidelines.

FILTER BAG REPLACEMENT

Remove the bag by loosening or cutting off the clamping band. Take the new filter bag, which is equipped with a stainless steel worm drive clamping band, and use a screw driver to tighten the bag around the frame channel. Ensure the bag is secure and that there is no slack around the perimeter of the band.

For more information on FLEXSTORM Inlet Filters and other ADS products, please contact our Customer Service Representatives at 1-800-821-6710 Try the ADS FLEXSTORM Online Product Configurator at www.inletfilters.com.

ADS "Terms and Conditions of Sale" are available on the ADS website, www.ads-pipe.com The ADS logo and the Green Stripe are registered trademarks of Advanced Drainage Systems, Inc. FLEXSTORM is a registered trademark of Inlet & Pipe Protection, Inc. © 2019 Advanced Drainage Systems, Inc. BRO 10892 06/19 MH

THE MOST ADVANCED NAME IN WATER MANAGEMENT SOLUTIONS®

Lift Handles ease installation and maintenance



Replaceable Sediment Bag

1/8" thick steel hangers & channels; precision stampings **configured to fit each individual casting**



CAD drawings, work instructions and test reports on website: **www.inletfilters.com**



Advanced Drainage Systems, Inc. 1-800-821-6710 www.ads-pipe.com

	StormTech Maintenance Log						
Project Name:							
Location:							
	_		-	StormTec www.stormtech.co	h		
	Stadia Rod	Readings					
Date	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)	Sediment Depth (1) - (2)	Observations / Actions	Inspector		



STANDARD LIMITED WARRANTY OF STORMTECH LLC ("STORMTECH"): PRODUCTS

- (A) This Limited Warranty applies solely to the StormTech chambers and end plates manufactured by StormTech and sold to the original purchaser (the "Purchaser"). The chambers and end plates are collectively referred to as the "Products."
- (B) The structural integrity of the Products, when installed strictly in accordance with StormTech's written installation instructions at the time of installation, are warranted to the Purchaser against defective materials and workmanship for one (1) year from the date of purchase. Should a defect appear in the Limited Warranty period, the Purchaser shall provide StormTech with written notice of the alleged defect at StormTech's corporate headquarters within ten (10) days of the discovery of the defect. The notice shall describe the alleged defect in reasonable detail. StormTech agrees to supply replacements for those Products determined by StormTech to be defective and covered by this Limited Warranty. The supply of replacement products is the sole remedy of the Purchaser for breaches of this Limited Warranty. StormTech's liability specifically excludes the cost of removal and/or installation of the Products.
- (C) THIS LIMITED WARRANTY IS EXCLUSIVE. THERE ARE NO OTHER WARRANTIES WITH RESPECT TO THE PRODUCTS, INCLUDING NO IMPLIED WARRANTIES OF MERCHANTABILITY OR OF FITNESS FOR A PARTICULAR PURPOSE.
- (D) This Limited Warranty only applies to the Products when the Products are installed in a single layer. UNDER NO CIRCUMSTANCES, SHALL THE PRODUCTS BE INSTALLED IN A MULTI-LAYER CONFIGURATION.
- (E) No representative of StormTech has the authority to change this Limited Warranty in any manner or to extend this Limited Warranty. This Limited Warranty does not apply to any person other than to the Purchaser.

- (F) Under no circumstances shall StormTech be liable to the Purchaser or to any third party for product liability claims; claims arising from the design, shipment, or installation of the Products, or the cost of other goods or services related to the purchase and installation of the Products. For this Limited Warranty to apply, the Products must be installed in accordance with all site conditions required by state and local codes; all other applicable laws; and StormTech's written installation instructions.
- (G) THE LIMITED WARRANTY DOES NOT EXTEND TO INCIDENTAL, CONSEQUENTIAL, SPECIAL OR INDIRECT DAMAGES. STORMTECH SHALL NOT BE LIABLE FOR PENALTIES OR LIQUIDATED DAMAGES, INCLUDING LOSS OF PRODUCTION AND PROFITS; LABOR AND MATERIALS; OVERHEAD COSTS; OR OTHER LOSS OR EXPENSE INCURRED BY THE PURCHASER OR ANY THIRD PARTY. SPECIFICALLY EXCLUDED FROM LIMITED WARRANTY COVERAGE ARE DAMAGE TO THE PROD-UCTS ARISING FROM ORDINARY WEAR AND TEAR: ALTERATION, ACCIDENT, MISUSE, ABUSE OR NEGLECT; THE PRODUCTS BEING SUBJECTED TO VEHICLE TRAFFIC OR OTHER CONDITIONS WHICH ARE NOT PERMITTED BY STORMTECH'S WRITTEN SPECIFICA-TIONS OR INSTALLATION INSTRUCTIONS; FAILURE TO MAINTAIN THE MINIMUM GROUND COVERS SET FORTH IN THE INSTALLATION INSTRUCTIONS; THE PLACEMENT OF IMPROPER MATERIALS INTO THE PRODUCTS; FAIL-URE OF THE PRODUCTS DUE TO IMPROPER SITING OR IMPROPER SIZING; OR ANY OTHER EVENT NOT CAUSED BY STORMTECH. THIS LIMITED WARRANTY REPRESENTS STORMTECH'S SOLE LIABILITY TO THE PURCHASER FOR CLAIMS RELATED TO THE PROD-UCTS, WHETHER THE CLAIM IS BASED UPON CON-TRACT, TORT, OR OTHER LEGAL THEORY.



70 Inwood Road Suite 3 Rocky Hill Connecticut 06067 888-892-2694

www.stormtech.com



Isolator[®] Row O&M Manual





THE MOST ADVANCED NAME IN WATER MANAGEMENT SOLUTIONS[™]

THE ISOLATOR® ROW

INTRODUCTION

An important component of any Stormwater Pollution Prevention Plan is inspection and maintenance. The StormTech Isolator Row is a technique to inexpensively enhance Total Suspended Solids (TSS) and Total Phosphorus (TP) removal with easy access for inspection and maintenance.

THE ISOLATOR ROW

The Isolator Row is a row of StormTech chambers, either SC-160, SC-310, SC-310-3, SC-740, DC-780, MC-3500 or MC-4500 models, that is surrounded with filter fabric and connected to a closely located manhole for easy access. The fabric-wrapped chambers provide for settling and filtration of sediment as storm water rises in the Isolator Row and ultimately passes through the filter fabric. The open bottom chambers and perforated sidewalls (SC-310, SC- 310-3 and SC-740 models) allow storm water to flow both vertically and horizontally out of the chambers. Sediments are captured in the Isolator Row protecting the storage areas of the adjacent stone and chambers from sediment accumulation.

A woven geotextile fabric is placed between the stone and the Isolator Row chambers. The woven geotextile provides a media for stormwater filtration, a durable surface for maintenance, prevents scour of the underlying stone and remains intact during high pressure jetting. A nonwoven fabric is placed over the chambers to provide a filter media for flows passing through the perforations in the sidewall of the chamber. The non-woven fabric is not required over the SC-160, DC-780, MC-3500 or MC-4500 models as these chambers do not have perforated side walls.

The Isolator Row is typically designed to capture the "first flush" and offers the versatility to be sized on a volume basis or flow rate basis. An upstream manhole provides access to the Isolator Row and typically includes a high flow weir. When flow rates or volumes exceed the Isolator Row weir capacity the water will flow over the weir and discharge through a manifold to the other chambers.

Another acceptable design uses one open grate inlet structure. Using a "high/low" design (low invert elevation on the Isolator Row and a higher invert elevation on the manifold) an open grate structure can provide the advantages of the Isolator Row by creating a differential between the Isolator Row and manifold thus allowing for settlement in the Isolator Row.

The Isolator Row may be part of a treatment train system. The design of the treatment train and selection of pretreatment devices by the design engineer is often driven by regulatory requirements. Whether pretreatment is used or not, the Isolator Row is recommended by StormTech as an effective means to minimize maintenance requirements and maintenance costs.

Note: See the StormTech Design Manual for detailed information on designing inlets for a StormTech system, including the Isolator Row.



Looking down the Isolator Row from the manhole opening, woven geotextile is shown between the chamber and stone base.



StormTech Isolator Row with Overflow Spillway (not to scale)





ISOLATOR ROW INSPECTION/MAINTENANCE

INSPECTION

The frequency of inspection and maintenance varies by location. A routine inspection schedule needs to be established for each individual location based upon site specific variables. The type of land use (i.e. industrial, commercial, residential), anticipated pollutant load, percent imperviousness, climate, etc. all play a critical role in determining the actual frequency of inspection and maintenance practices.

At a minimum, StormTech recommends annual inspections. Initially, the Isolator Row should be inspected every 6 months for the first year of operation. For subsequent years, the inspection should be adjusted based upon previous observation of sediment deposition.

The Isolator Row incorporates a combination of standard manhole(s) and strategically located inspection ports (as needed). The inspection ports allow for easy access to the system from the surface, eliminating the need to perform a confined space entry for inspection purposes.

If upon visual inspection it is found that sediment has accumulated, a stadia rod should be inserted to determine the depth of sediment. When the average depth of sediment exceeds 3 inches throughout the length of the Isolator Row, clean-out should be performed.

MAINTENANCE

The Isolator Row was designed to reduce the cost of periodic maintenance. By "isolating" sediments to just one row, costs are dramatically reduced by eliminating the need to clean out each row of the entire storage bed. If inspection indicates the potential need for maintenance, access is provided via a manhole(s) located on the end(s) of the row for cleanout. If entry into the manhole is required, please follow local and OSHA rules for a confined space entries.

Maintenance is accomplished with the JetVac process. The JetVac process utilizes a high pressure water nozzle to propel itself down the Isolator Row while scouring and suspending sediments. As the nozzle is retrieved, the captured pollutants are flushed back into the manhole for vacuuming. Most sewer and pipe maintenance companies have vacuum/JetVac combination vehicles. Selection of an appropriate JetVac nozzle will improve maintenance efficiency. Fixed nozzles designed for culverts or large diameter pipe cleaning are preferable. Rear facing jets with an effective spread of at least 45" are best. Most JetVac reels have 400 feet of hose allowing maintenance of an Isolator Row up to 50 chambers long. The JetVac process shall only be performed on StormTech Isolator Rows that have AASHTO class 1 woven geotextile (as specified by StormTech) over their angular base stone.

StormTech Isolator Row (not to scale)

Note: Non-woven fabric is only required over the inlet pipe connection into the end cap for SC-160LP, DC-780, MC-3500 and MC-4500 chamber models and is not required over the entire Isolator Row.





ISOLATOR ROW STEP BY STEP MAINTENANCE PROCEDURES

STEP 1

Inspect Isolator Row for sediment.

A) Inspection ports (if present)

- i. Remove lid from floor box frame
- ii. Remove cap from inspection riser
- iii. Using a flashlight and stadia rod, measure depth of sediment and record results on maintenance log.
- iv. If sediment is at or above 3 inch depth, proceed to Step 2. If not, proceed to Step 3.
- **B) All Isolator Rows**
 - i. Remove cover from manhole at upstream end of Isolator Row
 - ii. Using a flashlight, inspect down Isolator Row through outlet pipe
 - 1. Mirrors on poles or cameras may be used to avoid a confined space entry
 - 2. Follow OSHA regulations for confined space entry if entering manhole
 - iii. If sediment is at or above the lower row of sidewall holes (approximately 3 inches), proceed to Step 2. If not, proceed to Step 3.

STEP 2

Clean out Isolator Row using the JetVac process.

- A) A fixed floor cleaning nozzle with rear facing nozzle spread of 45 inches or more is preferable
- B) Apply multiple passes of JetVac until backflush water is clean
- C) Vacuum manhole sump as required

STEP 3

Replace all caps, lids and covers, record observations and actions.

STEP 4

Inspect & clean catch basins and manholes upstream of the StormTech system.



SAMPLE MAINTENANCE LOG

	Stadia Rod Readings		Sodimont Donth		
Date	Fixed point to chamber bottom (1)	Fixed point to top of sediment (2)	(1)–(2)	Observations/Actions	Inspector
3/15/11	6.3 ft	none		New installation. Fixed point is CI frame at grade	MCG
9/24/11		6.2	0.1 ft	some grit felt	SM
6/20/13		5.8	0.5 ft	Mucky feel, debris visible in manhole and in Isolator Row, maintenance due	NV
7/7/13	6.3 ft		0	System jetted and vacuumed	DJM

ADS "Terms and Conditions of Sale" are available on the ADS website, www.ads-pipe.com The ADS logo and the Green Stripe are registered trademarks of Advanced Drainage Systems, Inc. Stormtech^{*} and the Isolator[#] Row are registered trademarks of StormTech, Inc. © 2018 Advanced Drainage Systems, Inc. #11011 08/18 CS





Advanced Drainage Systems, Inc. 4640 Trueman Blvd., Hilliard, OH 43026 1-800-821-6710 www.ads-pipe.com